APPROVED
Florence Planning Commission Design Review Board

BURGER KING TRAFFIC IMPACT ANALYSIS

Florence, Oregon
BEI Project 20-113

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# BURGER KING TRAFFIC IMPACT ANALYSIS Florence, Oregon 



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### 1.0 EXECUTIVE SUMMARY

### 1.1 DESCRIPTION

The development site is located at the northeast quadrant of HWY 101 at $35^{\text {th }}$ Street in Florence, Oregon. The site is located on tax lot 06800 of assessor's map 18-12-23-22. The site is currently undeveloped and vacant. The site is planned to be developed with a 2,992 SF fast food restaurant with drive-through window. The subject site is zoned and designated for highway commercial land uses that supports the proposed use. Construction is expected to begin late in 2020 and the site is expected to be open for the second quarter in 2021.

### 1.2 TRAFFIC

The proposed development is projected to generate 120 AM and 98 PM peak hour automobile trips accessing the site at the street frontages at HWY 101 and at $35^{\text {th }}$ Street. On average, 58 of the AM trips and 48 of the PM trips are expected to be pass-by trips already on the public roadway system. Traffic impacts are evaluated for full development build-out for the years 2021 and 2026 during the AM and PM peak hours.

### 1.3 VEHICLE SAFETY, ACCESS

Crash history is evaluated for the intersection of $35^{\mathrm{th}}$ Street and HWY 101. The crash analysis did not identify a significantly high intersection crash frequency or unusual crash pattern occurring at the intersection of $35^{\mathrm{th}} \mathrm{St}$. and HWY 101. Driveway queuing and onsite vehicle and pedestrian circulation are evaluated to determine the operational safety of the site's proposed access configuration.

The site currently has a grant of access for one driveway approach on HWY 101 that includes provisions for restricting the operation of the access to right-in and right-out only turning movements. As planned, development and operation at the site will include utilizing the access entitlement to HWY 101, constructing a pork chop to comply with the permitted operation of the approach, and constructing an unrestricted access connection at the $35^{\text {th }}$ Street frontage. The access at the $35^{\mathrm{th}}$ Street frontage will provide primary access via the traffic signal at $35^{\mathrm{th}}$ Street and HWY 101.

### 1.4 RESULTS

In summary, the proposed development will not result in any identifiable significant impacts to the performance or operation of background traffic conditions with the added traffic from the proposed development during the anticipated year of opening 2021 peak hour conditions nor the projected year 2026 horizon year peak hour conditions.

### 2.0 INTRODUCTION

Branch Engineering Inc. has been retained to evaluate potential traffic impacts associated with the proposed site development. This report strives to meet the City of Florence Traffic Impact Study requirements as presented in a scoping memorandum from March 2020, which is included as Appendix A. Included in the analysis is a summary of existing and proposed transportation infrastructure, documentation of observed traffic volumes, projected post development 'Build' traffic volumes, an analysis of crashes at study area street intersections, an analysis of City of Florence access management code, an intersection performance evaluation for the studied intersections, and a queuing and blocking analysis at site driveways and the studied public street intersection at HWY 101 and $35^{\mathrm{th}}$ Street. A site plan is included in Appendix B.

The following is a list of analysis year scenarios analyzed in this report:

* Year 2020 ‘Existing’ Traffic Volumes (AM and PM Design Peak Hours);
* Year 2021 ‘Background’ Traffic Volumes (AM and PM Design Peak Hours);
* Year 2021 ‘Build’ Traffic Volumes (AM and PM Design Peak Hours);
* Year 2026 ‘Background’ Traffic Volumes (AM and PM Design Peak Hours), and;
* Year 2026 ‘Build’ Traffic Volumes (AM and PM Design Peak Hours).

The aerial below illustrates the location of the proposed development site.


Figure 1: Site Location
(Aerial Photography from Lane County Maps)

### 3.0 EXISTING CONDITIONS

### 3.1 ROADWAY NETWORK

The relevant roadways for this study are:

* $35^{\mathrm{th}}$ Street;
* Redwood Street, and;
* HWY 101.

The following table shows characteristics of the study area roadways in the vicinity of the site:

| Table 1: Roadway Network |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Roadway | Classifica <br> tion | Travel <br> Lanes | Sidewalks | Bike <br> Lanes | On-Street <br> parking | Posted <br> Speed |
| HWY 101 | Major/ <br> Principal <br> Arterial | $4^{1}$ | Yes | Yes | No | 40 MPH |
| $35^{\text {th }}$ Street | Minor <br> Arterial ${ }^{2}$ | 2 | Yes | Yes | No | 25 MPH |
| Redwood <br> Street | Local | 2 | Yes ${ }^{3}$ | Yes | No | 25 MPH |
| 1HWY 101 is developed with a continuous center two-way left-turn lane (CTWLTL) <br> 2The 2012 City of Florence TSP identifies 35" Street as a Minor Arterial, while ODOT maps identify it as a <br> Major/Urban Collector <br> ${ }^{3}$ Sidewalk present on east side only |  |  |  |  |  |  |

HWY 101 is within ODOT's jurisdiction and is maintained by ODOT. $35^{\text {th }}$ street is within the City of Florence's jurisdiction and is maintained by the City. The intersection of $35^{\text {th }}$ Street and HWY 101 is fully controlled by a traffic signal that is maintained by ODOT. The traffic signal features protected and permissive left-turn phasing with dedicated left turn lane approaches developed from a center two-way left-turn lane that is present on northbound and southbound approaches to $35^{\mathrm{th}}$ Street. The eastbound and westbound side street approaches on $35^{\text {th }}$ Street to HWY 101 feature permissive left turn phasing and are each developed with dedicated left-turn lanes for storage.

The intersection at Redwood and $35{ }^{\text {th }}$ Streets is a three-way stopped controlled ('tee") intersection, with the northbound Redwood Street approach to $35^{\mathrm{th}}$ Street stopped and the approaches on $35^{\text {th }}$ Street free movements.

Figure 2 shows the lane configurations and intersection controls of the study area intersections and roadways.
Z: \2020\20-113 Florence Burger King \Traffic \Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG2 6/30/2020 $11: 28$ AM DANH 23.1 s (LMS Tech)
EXISTING LANE CONFIGURATIONS \& INTERSECTION CONTROLS


### 3.2 NON-MOTORIZED FACILITIES AND TRANSIT

The site frontages include adjacent curbside sidewalks to accommodate pedestrian traffic and striped bike lanes to accommodate bicycles. Rhody Express is the local transit service and is a subsidiary of Lane Transit District. Rhody Express makes two route loops in Florence, with one north loop and one south loop that meet along $21^{\text {st }}$ Street. The north loop travels north and south on Spruce Street between $21^{\text {st }}$ and $42^{\text {nd }}$ Streets with a slight detour to Redwood Street between $32^{\text {nd }}$ and $35^{\text {th }}$ Streets. At $42^{\text {nd }}$ Street, the north loop travels to HWY 101, then via HWY 101 to Bi Mart and then to Fred Meyer before turning around and traveling south back to $21^{\text {st }}$ Street. The Rhody express operates on a 60-minute circuit around the loop Tuesday through Friday from 10:00AM through 6:00PM. The routes (north and south loops) provide public transportation to key parts of Florence, including retail centers, schools, food share, the Old Town district, the hospital and several city parks. The availability of transit, pedestrian and bicycle facilities can help to reduce the reliance on single occupant motorized vehicles. A Rhody Express route schedule is included as Appendix C.

### 3.3 INTERSECTIONS

With City concurrence, the following intersections are evaluated with background and proposed post-development traffic scenarios:

* $35^{\text {th }}$ Street at HWY 101;
* HWY 101 at proposed site right-in right-out only driveway, and;
* $35^{\text {th }}$ Street at proposed driveway access and Redwood Street (treated as one intersection).

The intersection of $35^{\text {th }}$ Street at Redwood Street is currently a three-approach ("tee") intersection. The proposed driveway approach on $35^{\text {th }}$ Street would add a southbound approach and a northbound receiving lane to the site, and would operate effectively as a fourth leg to the intersection at $35^{\text {th }}$ and Redwood Street. The southbound site driveway approach alignment to $35^{\text {th }}$ Street would be offset from the northbound Redwood Street approach alignment by approximately 35 feet at the proposed centerline of the site.

### 3.4 CRASHES

An analysis of crash history is performed for the public street intersections at HWY 101 and $35^{\text {th }}$ Street and $35^{\text {th }}$ Street at Redwood Street. Detailed crash data reports were downloaded from the Oregon Department of Transportation's (ODOT's) crash data system online for the most recent five-years of available data, which at the time of this report is the period from January 1, 2014 through December 31, 2018. Crash data reports are available from ODOT's Crash Analysis and Reporting Unit web tool at: https://www.oregon.gov/odot/Data/Pages/Crash.aspx. The current crash reporting methodology utilized by ODOT accounts for crashes only if an injury occurred or if a loss in property value of $\$ 1,500$ or more occurred as a result of the crash. There were no crashes reported to have occurred at the intersection of $35^{\text {th }}$ Street and Redwood Street. The following table illustrates the overall five-year crash history and frequency for the intersection at $35^{\text {th }}$ Street and HWY 101.

| Year | AADT <br> Volume | $\begin{aligned} & \text { Head- } \\ & \text { On } \end{aligned}$ | Ped | $\begin{aligned} & \text { Angle/ } \\ & \text { SS } \end{aligned}$ | Turn <br> Mvmts | Rear End | Fixed Object/Other | $\begin{gathered} \text { Total } \\ \text { Crashes } \end{gathered}$ | $\begin{aligned} & \text { Crash } \\ & \text { Rate }^{1} \end{aligned}$ | Critical <br> Rate ${ }^{2}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2014 | 16,000 | 0 | 0 | 0 | 2 | 0 | 0 | 2 | 0.07 | 0.62 |
| 2015 | 16,000 | 0 | 0 | 0 | 0 | 0 | $1^{3}$ | 1 | 0.17 | 0.62 |
| 2016 | 16,000 | 0 | 0 | 1 | 2 | 0 | 0 | 3 | 0.51 | 0.62 |
| 2017 | 16,000 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 0.07 | 0.62 |
| 2018 | 16,000 | 0 | 0 | 2 | 2 | 0 | 0 | 4 | 0.68 | 0.62 |
| Totals | 16,000 | 0 | 0 | 3 | 7 | 0 | 2 | 12 | 0.41 | 0.62 |
| 1. Crash Rate (Crashes per Million Entering Vehicles $=$ CPMEV) $=($ total 5 -year crashes $\times 1,000,000) /(5 \times$ ADT $\times 365)$ <br> 2. Critical rate per Highway Safety Manual (HSM) methodology <br> 3. One reported crash was the result of a backing maneuver |  |  |  |  |  |  |  |  |  |  |

While the year 2018 annual crash rate was greater than the referenced critical five-year average crash rate of 0.68 crashes per million entering vehicles for comparable signalized four-way urban intersections, the cumulative average of 0.41 crashes per million entering vehicles over the fiveyear analysis period was below the critical rate of 0.62 CPMEV, which is the standard applicable criteria. The crash frequency and types of crashes that are reported to have occurred are not considered abnormal and no additional crash reducing measures are necessary.

ODOT Crash Data System intersection crash data reports are included as Appendix D.

### 3.5 EXISTING TRAFFIC VOLUMES

AM and PM peak hour traffic volumes were collected by manual traffic counts conducted by Gary's Traffic Data and Branch Engineering in March 2020, and are included as Appendix E. The turning movement traffic counts at HWY 101 and $35^{\mathrm{th}}$ Street were collected between 7:00AM and 9:00AM and between $4: 00 \mathrm{PM}$ and $6: 00 \mathrm{PM}$ on a typical weekday (Tuesday-Thursday, non-holiday week), which was Thursday March 19, 2020. Peak hours extrapolated from the collected traffic count volumes were determined to occur between 8:00 AM and 9:00 AM and between 4:30 PM and 5:30 PM. Using the peak hour collected traffic count data for HWY 101 at $35^{\text {th }}$ Street are balanced through for design hour conditions to the proposed driveway intersection on HWY 101. The observed traffic volumes are provided on Figure 3 on page 9.

## $3.6 \quad$ SPECIAL TRANSPORTATION TREND ADJUSTMENT

Because traffic count data was collected during the week prior to the Oregon Governor's Stay Home Stay Safe (SHSS) guidelines that were implemented on March $23^{\text {rd }}$, 2020 due to concerns about spreading the Corona Virus/COVID-19, an adjustment factor is necessary to adjust collected traffic count data to account for reduced approaching traffic volume and trip making trends on the nearby transportation system. During the weeks leading to the SHSS guidelines, traffic had been declining, and public school was first temporary closed starting the week of March $16^{\text {th }}$, which was the week traffic count data was collected for the subject site and this traffic impact analysis. The Oregon Department of Transportation has been keeping track of traffic trends on statewide highways where there are Automated Traffic Recorder (ATR) Stations installed that continuously collect and record traffic volume data in real time. ODOT Weekly COVID-19 Traffic Trend Reports are available online at: https://www.oregon.gov/odot/Data/Pages/Traffic-Counting.aspx. For the
week immediately prior to the Governor's SHSS order, the data is available by clicking the link for April 10, 2020 - Observed Statewide Traffic Volume Patterns, which will take the user to collected data for the years 2019 and 2020 corresponding to the week that that traffic count data was collected for the site. Per the statewide data, the overall average daily traffic trend for the week of March $16^{\text {th }}$, 2020 was approximately 19 percent lower than 2019 traffic volume data for the ATR stations, therefore; an adjustment factor of 1.19 is assumed appropriate for adjusting collected traffic count data to simulate the effects of COVID-19 on transportation trends that were already occurring the week prior to the Governor's SHSS guidelines. The Oregon Department of Transportation's April 10, 2020 Traffic Report comparing year 2019 and 2020 traffic data collected at ATR stations during the week of March $16^{\mathrm{th}}, 2020$ that data was collected for the site, is included in Appendix E with the collected traffic count data.

### 3.7 SEASONAL ADJUSTMENT

Typically, peak hour traffic data collected for interior City streets within urbanized areas do not require seasonal adjustments to represent design hourly approaching traffic volumes (i.e. $30^{\text {th }}$ highest hour conditions), however, since the application of the Special Transportation Trend Adjustment Factor discussed and developed in the previous section is based on Statewide highways where there is seasonal fluctuation, and for the purpose of providing a conservative analysis herein, the collected year 2020 turning movement traffic count data with adjustments for the effects of COVID-19 are factored again by a calculated seasonal adjustment factor. The seasonal adjustment factor is calculated by using the most recent available ODOT Seasonal Trend Table (2018) available online at the ODOT Analysis Procedures Manual website (https://www.oregon.gov/odot/Planning/Pages/APM.aspx) with the coastal destination traffic trend assumed applicable for each approach/movement and by applying linear interpolation to the count date from tabulated values for the corresponding dates of those values. The adjustment methodology is consistent with the methodology outlined in the current edition of the Oregon Department of Transportation's (ODOT's) Analysis Procedures Manual (APM) at the link above. Tabulated seasonal trend values contained within the ODOT Seasonal Trend Table are available for the $1^{\text {st }}$ and the $15^{\text {th }}$ days of each month throughout the year. Per the 2018 Seasonal Trend Table, the coastal destination trend's peak seasonal period is represented during the period from July $1^{\text {st }}$ to July $15^{\mathrm{th}}$ with the Seasonal Trend Peak Period Factor of 0.8074 . For approaching traffic data collected on the March 19 ${ }^{\text {th }}$, 2020 count date, the seasonal trend is linearly interpolated for four days for tabulated values of between 1.04732 and 1.0504 corresponding to dates of March $15^{\text {th }}$ and April $1^{\text {st }}$. The interpolated seasonal trend value is 1.048 for March $19^{\text {th }}$, and the calculated seasonal adjustment factors applied to the turning movement traffic count data is 1.298.

Design Hour year 2020 traffic volumes with the product of the Special Transportation Trend and the calculated Seasonal Adjustment Factors applied are provided on Figure 4 on page 10.

### 3.8 PIPELINE TRAFFIC VOLUMES

In addition to existing roadway traffic volumes, pending local area land use development projects in construction that have been approved by the City of Florence but not yet issued occupancy may add traffic to the study area intersections and travel routes that would not be accounted for when traffic count data was collected. Motor vehicle trips that are included in a traffic impact analysis that has been approved, but not yet subject to post-development occupancy level day-to-day design hour trip generation conditions are known as pipeline trips. At the time of this traffic study, one approved traffic impact analyses for a residential subdivision located north of $35^{\text {th }}$ Street on

Rhododendron Drive northwest of the site with 31 single family residential detached housing units and 101 apartment units was approved but not built-out yet. The City of Florence furnished the approved traffic study for said development, which will add left-turns at the northbound and eastbound approaches to HWY 101 at $35^{\text {th }}$ Street and eastbound right-turns during design hour traffic conditions by the end of the year 2021. Figure 5 on page 11 shows the pipeline trips in the study area.

## $3.9 \quad$ TRAFFIC GROWTH AND FUTURE TRAFFIC

The City of Florence codified Traffic Impact Analysis requirements do not contain analysis details pertaining to analysis years that need to be included in a TIA. Per the Oregon Department of Transportation's (ODOT's) development review guidelines, Table 3.3, proposed developments that that generate between 1,000 and 2,999 average daily trips (ADT) and include a single-phase year of opening are suggested to include the year of opening and a five-year post-development planning horizon year in the analysis scenarios. Although ODOT does not require an analysis for this proposed development, the analysis required by the City of Florence is proposed to include the year of opening (anticipated to be 2021), and a five-year post-development horizon year, 2026, consistent with the suggested analysis year scenarios by ODOT.

To grow year 2020 design hour traffic volumes displayed on Figure 4 to represent year 2021 and year 2026 background design hour conditions, an average annual growth rate (AAGR) is calculated from the ODOT Future Volume Tables (FVT). The ODOT FVT provides base year 2018 and future year 2038 traffic volume data based on collected annual average daily traffic volumes for the base year and statewide traffic growth models that account for land use, transportation and employment trends associated with projected population growth and planned transportation improvements. The FVT for HWY 101 near the study area includes three sites on HWY 101 (ODOT HWY \#9). The following table provides the local area FVT tabulated values for three sites near the subject development site.

| Table 3: ODOT Future Volumes Table Data (AADT) |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Site ID | HwY | Description | 2018 | 2038 | RSQ |  |
| 1170 | 9 | 0.03 Mile South Munsel Lake Rd | 9500 | 9600 | 0.2347 |  |
| 1171 | 9 | 0.02 Mile South of 36th Street | 12500 | 12600 | 0.4298 |  |
| 1172 | 9 | 0.02 Mile South of 29th Street | 14100 | 14200 | 0.805 |  |

As shown in the table, one site has an RSQ $\left(\mathrm{R}^{2}\right)$ value greater than 0.75 that generally indicates that the data is reliable and usable. The usable data was collected at site 1172, which is located approximately 0.375 mile south of the intersection at $35^{\text {th }}$ Street and HWY 101. Using the data for site 1172 , the average annual growth rate (AAGR) calculation over the 20-year period between the 2018 base year and the 2038 future year is as follows:

$$
\text { AAGR }=(14,200-14,100) /(14,100 \times 20)=0.0355 \% / \text { year }
$$

The AAGR is then factored for one year and six years to project year 2021 year of opening background traffic volumes and year 2026 five-year post development horizon design hour background traffic volumes. The AAGR and growth rate factors are calculated and applied using
the linear growth methodology discussed in the current ODOT Analysis Procedures Manual (APM), Section 6.4.

Pipeline trips discussed in section 3.8, previously, are expected to be added to projected background growth calculated in the above steps by the year 2021 analysis year, therefore, background traffic volumes for the year 2021 and 2026 design hours will include growth of 2020 design hour traffic and pipeline trips. Background traffic volumes for the 2021 design hours are provided on Figure 6 on page 13. Year 2026 background traffic volumes are provided on Figure 7 on page 14 .
Z: \2020\20-113 Florence Burger King \Traffic \Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG3 6/30/2020 $11: 28$ AM DANH 23.1 s (LMS Tech)

SCALE: NTS
FIGURE 3
Z: \2020\20-113 Florence Burger King \Traffic \Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG4 6/30/2020 11:28 AM DANH 23.1s (LMS Tech)

SCALE: NTS
FIGURE 4
Branch
Z: \2020\20-113 Florence Burger King \Traffic $\backslash$ Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG5 6/30/2020 11:28 AM DANH 23.1 Is (LMS Tech)

Z: \2020\20-113 Florence Burger King \Traffic \Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG6 6/30/2020 $11: 28$ AM DANH 23.1 s (LMS Tech)

LEGEND
$x x=A M$ PEAK HOUR
$(x x)=$ PM PEAK HOUR
*BALANCED TO 35TH AND HWY IOI
IN \& OUT FLOWS
SCALE: NTS
FIGURE 6
Z: \2020\20-113 Florence Burger King \Traffic \Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG7 6/30/2020 $11: 28$ AM DANH 23.1 s (LMS Tech)

LEGEND
$x x=A M$ PEAK HOUR
$(x x)=$ PM PEAK HOUR
*BALANCED TO 35TH AND HWY IOI
IN \& OUT FLOWS
SCALE: NTS
FIGURE 7
Branch

### 4.0 DEVELOPMENT CONDITIONS

### 4.1 LAND USE

The potential developer entity is seeking land use approval for development of a 2.992 KSF GFA single occupant fast food restaurant with a drive-through window on the site. As planned and shown on the current site plan, the remaining land area in the area of disturbance abutting $35^{\text {th }}$ Street and approximately mid-block between $35^{\text {th }}$ Street and the extension of the right-of-way of $36^{\text {th }}$ Street will be utilized for parking and drive-through facilities to serve the proposed use. The drive through facilities will feature two parallel ordering kiosks that will allow two entrance lanes that will merge downstream into a single drive-through lane. The dual kiosks will allow approximately six vehicles to queue in three vehicle lengths from the entrance to the merge point in the drive-through lane downstream of the kiosks. As discussed in the introduction, a site plan is included as Appendix B.

### 4.2 SITE ACCESS

The site has frontage on HWY 101 between $35^{\text {th }}$ Street and the projected extension of the centerline of $36^{\mathrm{th}}$ Street across the north subject property line. The HWY 101 frontage has an existing curb cut at approximately mid-block between $35^{\mathrm{th}}$ and $36^{\mathrm{th}}$ Streets, where there was previously a right-of-way provided in the 1891 Frasier and Berry's Subdivision Plat between lots 1-10 and lots 11-20 of Block 10. This right-of-way was vacated within the last couple of decades, and the approximate location of the existing curb cut is planned to be improved and utilized with right-in right-out access controls between the site and HWY 101. Early correspondence with ODOT indicated that an access permit to occupy at this location will be supported by ODOT, given the proposed configuration and that all the applicant will need to do is submit for a driveway approach permit or for a construction permit for work within the right-of-way of HWY 101.

The site entitlements currently include a crossover access easement to the existing driveway approach at the adjacent site to the north, which is currently an unrestricted approach. The site plan does not currently show a connection to this existing driveway, which is aligned with the centerline of $36^{\text {th }}$ Street.

## $4.3 \quad 35^{\text {TH }}$ STREET ACCESS

The site plan includes a proposed 25 -foot wide unrestricted access connection on the site's $35^{\text {th }}$ Street frontage that is shown as a fourth leg (dog-leg) to the existing three-way intersection at Redwood Street. The proposed driveway centerline shown on the site plan is located approximately 35 feet west of the existing centerline of Redwood Street. Redwood Street's centerline is aligned approximately with the existing eastern property line at the site. The placement of this access is shown on the site plan to maximize the spacing from the stop bar at the westbound $35^{\text {th }}$ Street approach to HWY 101, where the designer presumably is concerned with impacting the existing striped queue storage and driveway operation within close proximity to the intersection of HWY 101 at $35^{\mathrm{th}}$ Street. The City of Florence Code requires a minimum of 25 feet of spacing distance between access and intersections (Title 8, section 8-2-3-4(D)), but it is not clear from this section of the code if the code is applicable to the centerlines or the nearside of the edges of the approach throats when offsetting driveway and public street driveway approaches are across the street from
one another. The subdivision code, Title 10, Chapter 35, also discusses approach spacing in section 10-35-2-7, where it says the minimum separation of accesses and public street intersections on the same side of the street shall be 30 feet between the near side surfacing of the approach and the near right-of way of collector streets. Title 10-35-2-7 also includes a minimum offset of 50 feet of separation between near side edge of surfacing of the approach and the right-of way of areterial streets. The proposed access location offset from the intersection at Redwood Street shown on the site plan does not involve conflicting left turns, since left-turns are allowed to be executed in front of one another approaching the driveway from the west and approaching Redwood Street from the east. The approach also leads to an access easement, which is presumably for future development of parcels to the north that are not currently proposed to be developed.

### 4.4 TRIP GENERATION

To project trip generation for the proposed post development traffic conditions, references were made to the Trip Generation Manual, $10^{\text {th }}$ Edition, and the Trip Generation Handbook, $3^{\text {rd }}$ Edition, both published by the Institute of Transportation Engineers (ITE). The table on the following page summarizes the projected peak hour site generated traffic based on the available published data.

### 4.5 PASS-BY TRIPS

The development site's type of land use is expected generate a significant amount of pass-by and diverted linked traffic during AM and PM peak hour trip generation periods, since most motorists do not make exclusive or long trips across town, passing similar competing markets on the way to make fast food restaurant purchases, but rather visit these types of establishments on the way to or from a primary origination/destination in the vicinity. The presence of competing markets nearby will have a significant effect on the distance motorists are willing to travel to make a trip to the site, since the HWY 101 corridor is developed with a significant density of competing uses similar to the site's proposed use.

According to the ITE Trip Generation Handbook, $3^{\text {rd }}$ Edition, "Pass-by trips are trips that are made as intermediate stops on the way from an origin to a destination without a route diversion. Pass-by trips are attracted from traffic passing the site on an adjacent street or roadway that offers direct access to the trip generator. Pass-by trips are not diverted from another roadway." The Trip Generation Handbook contains the following average pass-by trip rate percentages for the proposed use as a fast food restaurant with drive through window: 49 percent of AM and 50 percent of PM trip generation during the peak hours of adjacent street traffic.

Based on the site's proposed access configuration, most northbound pass-by trips by familiar drivers on HWY 101 would be expected to enter the site via a right-turn at $35^{\text {th }}$ Street to access the site driveway, traverse the drive-through, then exit the site at the proposed RIRO driveway, which will be similar to a diverted linked trip. Similarly, southbound pass-by trips are likely to be disguised as diverted linked trips, that will enter the site via a left-turn from HWY 101 at $35^{\text {th }}$ Street to access the driveway, traverse the drive-through and re-enter southbound traffic on HWY 101 as left-turns from $35^{\text {th }}$ Street's westbound approach.

The trip generation potential for the development of the site is illustrated in the following table.

| Table 4: Site Trip Generation |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use/ ITE Land Use Code | Ind. Variable | Units (QTY) | TG Rate Trips/Unit | Trips | $\begin{aligned} & \hline \text { Trips } \\ & \text { IN } \\ & (\% / \#) \\ & \hline \end{aligned}$ | Trips OUT (\%/\#) |
| AM Peak Hour |  |  |  |  |  |  |
| Fast Food Restaurant with Drive-Through Window/934 | KSF GFA | 2.992 | 40.19 | 120 | 51\%/61 | 49\%/59 |
| Pass-By Trips: |  |  | 49\% | 58* | 29 | 29 |
| PM Peak Hour |  |  |  |  |  |  |
| Fast Food Restaurant with Drive-Through Window/934 | KSF GFA | 2.992 | 32.67 | 98 | 52\%/51 | 48\%/47 |
| Pass-By Trips: |  |  | 50\% | 48* | 24 | 24 |
| ADT |  |  |  |  |  |  |
| Fast Food Restaurant with Drive-Through Window/934 | KSF GFA | 2.992 | 470.95 | 1,409 | 50\%/705 | 50\%/704 |
| *AM and PM Pass-by trips (59 and 49) are rounded down to nearest even number for $50 \%$ even directional split in/out |  |  |  |  |  |  |

As indicated above, the proposed post-development traffic conditions could result in an average of 120 AM peak hour vehicle trips and 98 PM peak hour vehicle trips accessing the site. Out of the total peak hour vehicle trips, 49 percent of the AM and 50 percent of the PM peak hour trips are expected to be pass-by trips, which are trips already on the roadway that would be expected to make a route diversion at the site driveway to enter and exit the site, but will otherwise continue in the same direction they were traveling upon leaving the site between primary trip origins and destinations. Per City of Florence Title 10, Chapter 1, Section 10-1-1-4(E), any development that generates 250 or more average daily trips is required to provide a traffic impact analysis. Because the proposed development will generate 1,409 average daily trips during typical weekday, the proposed development requires a traffic impact analysis.

### 4.6 TRIP DISTRIBUTION AND ASSIGNMENT

To project the proposed development's trip distribution, references were made to the ODOT Future Volumes Table (FVT) for HWY 101 locations, projected design hour traffic volumes, recent traffic impact analyses and the location of the site with respect to local land use patterns (trip origin and destination pairs). The ODOT FVT includes daily traffic volumes that were most recently collected in 2018 at a location 0.02 miles south of the intersection at $36^{\text {th }}$ Street that is north of the site and has the highest AADT reported $(12,500)$ between the site and the north UGB on HWY 101. A second location south of the site located approximately 0.02 mile south of the intersection at $17^{\text {th }}$ and HWY 101 had an AADT of 16,400 , and was the highest ADT reported for the corridor between the south UGB and the site. Using these data and the approaching traffic volumes on east and west approaches to the intersection at HWY 101 and $35^{\text {th }}$ Street, and the relative distribution of residential land uses near the site, the expected primary non-pass-by trip assignment is anticipated to be split with the following assignments:
$\div 35 \%$ of primary trip origins and destinations north via HWY 101;
$\div \quad 45 \%$ of primary trip origins and destinations south (east and west) via HWY 101;

* $15 \%$ of primary trip origins and destinations west via $35^{\text {th }}$ Street, and;
* 5\% of primary trip origins and destinations south and east via Redwood Street and Spruce Streets.

Anticipated pass-by (non-primary) trip assignments are based on AM and PM design hour approaching traffic volumes, which are generally evenly split between northbound and southbound directions at the site frontage during AM and PM design hour conditions. The distribution of pass-by trips is therefore evenly split, with approximately 50 percent of pass-by trips coming from and destined to each the northbound and southbound direction at the site frontage. As described previously, the site's access configuration will result in a significant number of would-be pass-by trips at the site's driveway on HWY 101 being redistributed from through movements to turning movements at the intersection at $35^{\text {th }}$ Street and HWY 101 because the site driveway is limited to right-in and right-out only turning movements and will be located at the northern extent of the site. The arrangement of access on the site places the RIRO approach on HWY 101 north of the drive-through window entrance and parking area, with the site located on the east side of the highway. The approaching traffic volumes on $35^{\text {th }}$ Street are not likely to contribute significantly to the pass-by trip making phenomenon, and it is assumed that there will not be a significant number of pass-by trips out of $35^{\text {th }}$ Street's approaching traffic volume.

An illustration of the site's peak hour site generated traffic distribution is provided on the following page as Figure 8.

To analyze the post-development build scenario traffic conditions in the design hours of the analysis years, the site generated traffic was added to the year 2021 and 2026 background traffic volumes. These "build" total traffic volumes are provided on Figures 9 and 10 on pages 20 and 21.
Z: \2020\20-113 Florence Burger King\Traffic \Figures \20-113 TRAFFIC FIGURES.dwg FIG8 6/30/2020 11:28 AM DANH 23.1s (LMS Tech)
$\begin{array}{cl}\text { PEAK HOUR SITE GENERATED } \\ \text { TRAFFIC } & \text { DISTRIBUTION }\end{array}$


| PEAK <br> HOUR | PRIMARY <br> TRIPS |  | PASS-BY <br> TRIPS |  | TOTAL |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | IN | OUT | IN | OUT | IN | OUT |
| AM | 32 | 30 | 29 | 29 | 61 | 59 |
| PM | 27 | 23 | 24 | 24 | 51 | 47 |

SCALE: NTS
FIGURE 8
Z: \2020\20-113 Florence Burger King \Traffic \Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIG9 6/30/2020 11:28 AM DANH 23.1 Is (LMS Tech)

Z: \2020 \20-113 Florence Burger King \Traffic $\backslash$ Figures $\backslash 20-113$ TRAFFIC FIGURES.dwg FIGIO 6/30/2020 11:28 AM DANH 23.1 s (LMS Tech)


### 4.7 INTERSECTION PERFORMANCE

Study area intersections are evaluated to determine Level of Service (LOS) based on average vehicle delay. Calculations for the signalized and unsignalized intersections are performed with the computer program SYNCHRO 10, by Trafficware, which utilizes selectable Highway Capacity Manual (HCM) methodologies to analyze intersection and approach lane performance characteristics. Per the Oregon Department of Transportation Analysis Procedures Manual's (APM's) recommended analysis procedure, the HCM 2000 methodology is utilized to analyze signalized intersections and the HCM 2010 methodology is utilized to analyze unsignalized intersection and approach performances.

Level of service is classified by a letter scale from 'A' to ' F '. LOS ' A ' represents optimum operating conditions and minimal delay. LOS ' F ' indicates over capacity conditions causing unacceptable delay. The City of Florence's current transportation system plan (TSP) identifies with LOS ‘D' at all signalized and all-way stop controlled intersections if the V/C ratio is not higher than 1.0 for the facility. The City of Florence standard for unsignalized two-way stop-controlled intersections is LOS ' $E$ ' at the critical movement (poorest performing approach). Mitigation measures may be necessary to remedy performance level conditions when level(s) of service fall(s) below the codified City standard or when the ODOT V/C performance standard is not met and the condition is identified to be the result of development impacts.

The LOS determined by average delay per vehicle as established in the Highway Capacity Manual, 2000 and HCM, 2010 are as follows:

| Table 5: HCM Level of Service Criteria |  |  |
| :---: | :---: | :---: |
| Level of Service | Unsignalized $-\mathbf{H C M} 2010$ | Signalized - HCM 2000 |
| A | $<10 \mathrm{sec}$ | $<10 \mathrm{sec}$ |
| B | $>10$ and $\leq 15 \mathrm{sec}$ | $>10$ and $\leq 20 \mathrm{sec}$ |
| C | $>15$ and $\leq 25 \mathrm{sec}$ | $>20 \mathrm{and} \leq 35 \mathrm{sec}$ |
| D | $>25$ and $\leq 35 \mathrm{sec}$ | $>35 \mathrm{and} \leq 55 \mathrm{sec}$ |
| E | $>35$ and $\leq 50 \mathrm{sec}$ | $>55 \mathrm{and} \leq 80 \mathrm{sec}$ |
| F | $>50 \mathrm{sec}$ | $>80 \mathrm{sec}$ |
|  |  |  |

Another measure of performance and congestion is volume to capacity ratio (v/c), which can indicate the level of demand as a proportion of the roadway's theoretical capacity. As described previously, ODOT does not require a formal traffic impact analysis for the proposed development, but HWY 101 is an ODOT facility and ODOT performance standards apply to their facilities. The ODOT mobility performance standard "target" that would be applicable to a statewide highway with a speed limit of 40 MPH , not designated a freight route and within an urban growth boundary of a non MPO area, is to maintain a not-to exceed $\mathrm{v} / \mathrm{c}$ of 0.85 for the entire intersection at signalized intersections or state highway approaches to unsignalized intersections. In order to maintain safe operation, non-state highway approaches at unsignalized intersections are expected to meet or not to exceed the $v / \mathrm{c}$ of 0.90 , based on the same area and speed characteristics, applicable for District/Local interest roads. The v/c of 0.90 is applicable to stop controlled driveway approaches to the state highway. The basic peak hour performance standard for evaluation of intersection performance at HWY 101 at $35^{\text {th }}$ Street would be the not-to-exceed v/c ratio of 0.85 based on the highway designation.

To analyze the study area intersections, the intersection peak hour factors calculated from collected traffic count data are utilized for all existing year design hour conditions and for future build and background/no-build traffic conditions, as shown on the figures representing traffic volumes. For driveway approaches that do not currently serve any inflow or outflow traffic (background/no-build conditions) peak hour factors from the approaches at the intersection of HWY 101 and $35^{\text {th }}$ Street are calculated from balancing the approaching traffic stream and utilized in the build conditions, with appropriate adjustments made consistent with the Oregon Department of Transportation's (ODOT's) Analysis Procedures Manual for future conditions (where applicable).

Per the Analysis Procedures Manual, the default saturation flow rate of 1750 pchpl is utilized in the SYNCHRO analysis software program models for all background and build traffic analysis scenarios, since the site is within the Florence Urban Growth Boundary, which is not identified as an MPO area. Saturation flow rate is defined by the ODOT Analysis Procedures Manual (APM) as the maximum departure (queue discharge) flow rate achieved by vehicles departing from the queue during the green period at traffic signals.

Results of the intersection performance analysis calculations are documented in Appendices G and H and are summarized in the following table:

| Table 6: Intersection Performance Analysis |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | Performance Standard ${ }^{(1)}$ | 2020 Existing Conditions | $\begin{gathered} \hline 2021 \\ \text { No- } \\ \text { Build } \end{gathered}$ | $\begin{aligned} & 2021 \\ & \text { Build } \end{aligned}$ | $\begin{gathered} \hline 2026 \\ \text { No- } \\ \text { Build } \end{gathered}$ | $\begin{aligned} & 2026 \\ & \text { Build } \end{aligned}$ |
| AM Peak Hour |  |  |  |  |  |  |
| HWY 101 at $35{ }^{\text {th }}$ Street | $\begin{gathered} \hline \text { LOS D/ } \\ \text { v/c } 0.85 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \hline \text { LOS A/ } \\ \text { v/c } 0.41 \end{gathered}$ | $\begin{gathered} \hline \text { LOS A/ } \\ \text { v/c } 0.43 \end{gathered}$ | $\begin{array}{c\|} \hline \hline \text { LOS A/ } \\ \text { v/c } 0.43 \end{array}$ | $\begin{gathered} \hline \hline \text { LOS A/ } \\ \text { v/c } 0.43 \end{gathered}$ | $\begin{gathered} \hline \text { LOS A/ } \\ \text { v/c } 0.43 \end{gathered}$ |
| $35^{\mathrm{th}}$ Street at Redwood Street/Site Dwy ${ }^{(2)}$ | LOS E | LOS A | LOS A | LOS B | LOS A | LOS B |
| HWY 101 at Site Dwy ${ }^{(2)}$ | $\begin{gathered} \hline \text { LOS E/ } \\ \text { v/c } 0.85 \\ \hline \end{gathered}$ | LOS A ${ }^{(3)}$ | LOS A ${ }^{(3)}$ | $\begin{array}{\|c\|} \hline \text { LOS B/ } \\ \text { v/c } 0.04 \\ \hline \end{array}$ | LOS A ${ }^{(3)}$ | $\begin{gathered} \hline \text { LOS B/ } \\ \text { v/c } 0.04 \\ \hline \end{gathered}$ |
| PM Peak Hour |  |  |  |  |  |  |
| HWY 101 at $35{ }^{\text {th }}$ Street | LOS D/ <br> $\mathrm{v} / \mathrm{c} 0.85$ | LOS B/ <br> v/c 0.50 | $\begin{gathered} \hline \text { LOS B/ } \\ \text { v/c } 0.52 \\ \hline \end{gathered}$ | $\begin{array}{\|c\|} \hline \text { LOS B/ } \\ \text { v/c } 0.52 \\ \hline \end{array}$ | $\begin{array}{\|c\|} \hline \text { LOS B/ } \\ \text { v/c } 0.52 \\ \hline \end{array}$ | $\begin{gathered} \hline \text { LOS B/ } \\ \text { v/c } 0.52 \end{gathered}$ |
| $35^{\text {th }}$ Street at Redwood Street/Site Dwy ${ }^{(2)}$ | LOS E | LOS B | LOS B | LOS B | LOS B | LOS B |
| HWY 101 at Site Dwy ${ }^{(2)}$ | $\begin{gathered} \hline \text { LOS E/ } \\ \text { v/c } 0.85 \end{gathered}$ | LOS A ${ }^{(3)}$ | LOS A ${ }^{(3)}$ | $\begin{gathered} \hline \text { LOS A/ } \\ \text { v/c } 0.04 \end{gathered}$ | LOS A ${ }^{(3)}$ | $\begin{gathered} \hline \text { LOS B/ } \\ \text { v/c } 0.04 \\ \hline \end{gathered}$ |
| ${ }^{(1)}$ LOS $=$ Level Of Service <br> ${ }^{(2)}$ LOS \& $\mathrm{v} / \mathrm{c}$ at stop controlled approach <br> ${ }^{(3)}$ HCM 2010 analysis methodology of unsignalized intersections (driveways) that are not serving traffic do not report a $\mathrm{v} / \mathrm{c}$ for free movements or stop controlled approaches |  |  |  |  |  |  |

As shown in the table, under the build scenario traffic conditions, the study area street and site driveway intersections are projected to continue to operate within the identified City of Florence and Oregon Department of Transportation mobility standards for level of service and v/c. Background/No-Build and Build HCM 2000 performance reports for the signalized intersection at HWY 101 and $35^{\text {th }}$ Street and HCM 2010 performance reports for the unsignalized driveway intersections are included as Appendices G and H .

### 4.8 VEHICLE QUEUING ANALYSIS

In addition to mobility performance calculations, vehicle queue length calculations are performed for the public street intersections at HWY 101 and $35^{\text {th }}$ Street and $35^{\text {th }}$ Street at Redwood Street, as well as the proposed site driveway approach on the HWY 101 site frontage. To simulate and review approximate vehicle queue lengths, the analysis software program SimTraffic 10, by TrafficWare is utilized to model development scenario traffic conditions to determine if vehicle queue lengths are increased significantly with traffic added from the proposed development, or if blocking conditions exist or are worsened with the development traffic. SimTraffic utilizes random number seeding to generate approaching traffic scenarios and to simulate resulting traffic conditions. For queuing reports, a minimum five runs are simulated with the vehicle queue lengths calculated from the average of the five runs.

To accurately model the signalized intersection approach queue lengths at the site driveway on HWY 101 the driveway approach intersection node representing the driveway location was relocated out of the potential traffic signal queue area at the street intersection at HWY 101 and $35^{\mathrm{th}}$ Street. The iteration was performed because SimTraffic does not report overlapping queues appropriately when an approach is located within an intersection approach queue, and it is unknown if the intersection's southbound approach queue is as long or longer than the link length.

The SimTraffic report splits the queue length between the two intersection nodes and reports through movement queues for the uncontrolled approaches at the upstream intersection and the downstream intersection, rather than reporting the queue for the intersection where vehicles are required to stop downstream. In some instances, SimTraffic also reports through movement queues at uncontrolled approaches that are not influenced by downstream metering at partially controlled (unsignalized) intersections. For these reasons, ODOT allows alternate manual calculation methods for calculating vehicle queue lengths at stopped controlled approaches, such as the AASHTO two-minute rule.

Background/No-Build and Build output files from SimTraffic documenting calculated vehicle queue lengths are included as Appendices I and J. The following table summarizes the SimTraffic queuing microsimulation results:

| Table 7: Design Hour Intersection Queuing |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection: | Approach Movement | Queue Storage Length (ft) | $\begin{array}{\|c\|} \hline \text { Current } \\ \text { Year } \\ 2020 \\ \text { Ave/95 } \\ \hline \end{array}$ | $\begin{gathered} 2021 \\ \text { 'No-Build' } \\ \text { Ave/95 } \end{gathered}$ | $\begin{gathered} 2021 \\ { }^{2021}{ }^{\text {Build }} \text { ' } \\ \text { Ave } / 955^{\text {ah }} \end{gathered}$ | $\begin{gathered} 2026 \\ \text { 'No- } \\ \text { Build' } \\ \text { Ave/95 }{ }^{\text {th }} \\ \hline \end{gathered}$ | $\begin{gathered} 2026 \\ \text { 'Build' } \\ \text { Ave/95 } \end{gathered}$ |
| AM Peak Hour |  |  |  |  |  |  |  |
| HWY 101 at 35 ${ }^{\text {th }}$ Street | EBL | $110^{\prime}$ | 50/75 | 50/75 | 50/75 | 50/75 | 50/75 |
|  | EBTR | 250+ | 50/75 | 50/75 | 50/75 | 50/75 | 50/75 |
|  | WBL | 140 ' | 25/50 | 25/50 | 25/75 | 25/50 | 25/50 |
|  | WBTR | 275' | 25/50 | 25/50 | 25/50 | 25/50 | 25/50 |
|  | NBL | 150' | 25/50 | 25/50 | 25/50 | 25/50 | 25/50 |
|  | NBT | 250' | 50/100 | 50/100 | 50/125 | 50/100 | 25/50 |
|  | NBTR | 250 ' | 25/50 | 25/75 | 50/75 | 25/75 | 50/75 |
|  | SBL | 93 ' | 25/50 | 25/50 | 25/75 | 25/50 | 25/75 |
|  | SBT | 140' | 50/100 | 50/100 | 50/100 | 50/100 | 50/100 |
|  | SBTR | 245' | 50/100 | 50/100 | 50/100 | 50/100 | 50/100 |
| 35 $\begin{array}{r}\text { th } \\ \text { Street at } \\ \text { Redwood }\end{array}$ | NBLTR | 150'+ | 25/50 | 25/50 | 25/50 | 25/50 | 25/50 |
|  | SBLTR | $55^{\prime}$ | 0/0 | 0/0 | 25/50 | 0/0 | 25/50 |
| HWY 101 at Site Dwy | WBR | 50' | 0/0 | 0/0 | 25/50 | 0/0 | 25/50 |
| PM Peak Hour |  |  |  |  |  |  |  |
| HWY 101 at $35^{\text {th }}$ Street | EBL | $110^{\prime}$ | 50/75 | 50/75 | 50/75 | 50/75 | 50/100 |
|  | EBTR | 250+ | 50/100 | 50/100 | 50/100 | 50/100 | 50/100 |
|  | WBL | 140' | 25/50 | 25/50 | 25/50 | 25/50 | 50/75 |
|  | WBTR | 275' | 25/50 | 25/75 | 25/75 | 25/75 | 25/75 |
|  | NBL | 150' | 50/75 | 50/100 | 50/75 | 50/75 | 50/100 |
|  | NBT | $250^{\prime}$ | 75/150 | 75/150 | 75/150 | 75/150 | 100/150 |
|  | NBTR | 250' | 75/125 | 75/125 | 75/125 | 75/150 | 75/150 |
|  | SBL | $93^{\prime}$ | 25/75 | 25/75 | 50/75 | 25/75 | 50/75 |
|  | SBT | 140' | 100/150 | 100/150 | 100/150 | 100/175 | 100/175 |
|  | SBTR | 245 | 75/150 | 75/150 | 100/150 | 100/150 | 100/175 |
| $35^{\mathrm{th}}$ Street at Redwood | NBLTR | 150'+ | 50/50 | 25/50 | 25/50 | 25/50 | 50/75 |
|  | SBLTR | $55^{\prime}$ | 0/0 | 0/0 | 25/50 | 0/0 | 25/50 |
| HWY 101 at Site Dwy | WBR | 50' | 0/0 | 0/0 | 25/50 | 0/0 | 25/50 |

As shown in the table, the projected vehicle queuing on the study area roadways is not significantly increased with the addition post-development traffic.

As discussed previously, approaches that are unsignalized are not modeled as well as signalized approaches in the SimTraffic microsimulation analysis software. Although ODOT allows alternate manual calculations for unsignalized intersections, such as the AASHTO 2-minute rule, the results reported above are from SimTraffic for both signalized and unsignalized intersection approaches within the study area. The SimTraffic reports also show a westbound queue at the $35^{\text {th }}$ Street approach to Redwood Street for no-build conditions and east- and westbound queues for the build traffic scenarios. The eastbound and westbound approaches are not likely to result in tangible queues, since the approaches are free movements, and most reported queues are less than one standard car length which is generally considered 25 feet. The ODOT Analysis Procedures Manual
discusses the use of the AASHTO 2-minute rule for approximating unsignalized intersection vehicle queues, specifically at stop-controlled approaches. The AASHTO 2-minute rule does not calculate through street non-stopped movement traffic queues, which are also not shown here.

### 4.9 VEHICLE CIRCULATION

The site plan includes a proposed unrestricted driveway approach on $35^{\text {th }}$ street that is currently shown offset west from the centerline of Redwood Street. Redwood Street approaches $35^{\text {th }}$ street from the south with the site driveway approach at the north side of $35^{\text {th }}$ Street approximately 35 feet west of Redwood Street. During the scoping process, the City of Florence mentioned the proposed offset of the driveway to the intersection of Redwood Street and requested a discussion about the operation of offsetting left turns from $35^{\text {th }}$ Street to the site and from $35^{\text {th }}$ Street to Redwood Street. Optimally, the site driveway would be aligned with the Redwood Street approach centerline, except the site's eastern property boundary is aligned with the centerline of Redwood Street and does not allow such a configuration to be constructed. To add to the discussion regarding the offset intersection and driveway approach on $35^{\text {th }}$ Street that was provided in section 4.3 previously, a driveway approach movement simulation is provided as Figure 11 on the following page that simulates vehicle maneuvers from the site directly to Redwood Street and from Redwood Street directly into the site. The turning movement simulations are prepared based on AUTOTURN ${ }^{\circledR}$ version 10.2.3.70, and demonstrate that a full-size crew cab pickup truck with a 168inch (14.0 foot) wheel base is capable of maneuvering to the site driveway from Redwood Street and the reverse, simultaneously, without any noted physical conflicts. There are not likely to be a large number of vehicles making these maneuvers, as was demonstrated in section 4,6 and on Figures 8 through 10 (trip distribution and build traffic volumes).

Vehicular traffic destined to the drive-through can either enter the site at the RIRO (right-in rightout only) approach on HWY 101, which requires circulation through the parking area to the drivethrough, or enter via the unrestricted approach on $35^{\text {th }}$ Street which is generally aligned with the drive-through entrance. The site plan shows stacking for up to 13 vehicles total with approximately 20 feet of linear space for each vehicle. The drive-through features two ordering kiosks (two) that will be located in the southeast quadrant of the parcel that allow simultaneous ordering for two vehicles. Downstream of the ordering kiosks (between the kiosks and the pick-up window), there is a merge point where the two lines from the two kiosks merge to form one single lane through the drive-through pick-up window. Including vehicles at the ordering kiosks, there is stacking available for approximately 7 vehicles at the ordering kiosks and upstream to the drive-through entrance in the parking area. The drive-through window is shown in the northwest portion of the building where drive-through egress vehicles can circulate counter clockwise back through the site parking area to the unrestricted driveway approach on $35^{\text {th }}$ Street for destinations to the south or to the west, or make a right turn followed by a left turn to get to HWY 101 and travel northbound.

Although the drive-through appears to work as it is shown with available space for up to 13 vehicles, and 7 of those vehicles at the ordering kiosks and upstream within the designated aisle, if/when the number of vehicles waiting to order is greater than 8 vehicles, there is a propensity for traffic entering the site to be blocked from accessing the parking area from the driveway. It is recommended that the driveway approach be relocated 70 to 75 feet west to avoid a direct alignment with the drive-through queue and to allow more vehicles to be stored on site leading to the drive-through facilities, and to improve access to the parking area when the drive-through is experiencing peak usage. A scenario showing a possible access location on $35^{\text {th }}$ Street is shown
on Figure 12. The relocated proposed access shown on the site plan would be accommodated with parking adjustments that would replace the area currently shown on the site plan with the proposed driveway. There may be issues with crossover access as is shown on the site plan for future use if the parcels to the north are developed, however the proposed driveway access stub shown at the RIRO is assumed to provide access to the property north of the Burger King Site as well for future considerations. If there is currently a recorded deed covenant regarding an access easement, the currently shown location of the approach will meet the City's codified requirements, but is otherwise recommended to be widened to 35 feet to allow two ingress lanes, one of which will allow the drive-through to be by-passed during peak usage to access parking on the site.

In addition to the recommended access relocation, some additional motor vehicle traffic related circulation improvements are recommended to keep onsite circulation flowing at optimal flow conditions and to maintain a high level of safety and situational awareness for the traveling public. The recommended improvements include signage for egress vehicles reminding drivers not to block the driveway entrance on HWY 101 when departing the site (turn left against a queue when queue is present to turn right onto HWY 101 northbound - mostly for future consideration when parcels to the north are developed and the driveway is extended); do not enter signage for vehicles entering the site designating the operation of the drive-through egress; do-not enter signage for southbound traffic on HWY 101 to remind drivers not to enter the RIRO approach via left-turn, and; signage showing the channelization of the approach at HWY 101, which will be restricted to right-in right-out only. These recommended improvements are shown on Figure 12 on page 30.

### 4.10 PEDESTRIAN ACCESS AND CIRCULATION

Florence Development Code Title 10, Chapter 35, Section 3-2 (10-35-3-2B) provides direction on pedestrian access and circulation from public rights-of-way at development site street frontages to building entrances on site. A few recommended improvements to the site plan are included on Figure 12 on Page 30. The recommended improvements include the following:

* An ADA compliant raised walkway adjacent to the parking stalls at the north side of the western parking area that connects to the HWY 101 sidewalk at the frontage. The walkway should lead to the building entrance area near the door facing HWY 101 and should include accessible ramps as needed as well as crosswalk markings across the area exposed to vehicular traffic;
* An ADA compliant walkway from the $35^{\text {th }}$ street frontage to the main building entrance facing $35^{\text {th }}$ street. There is currently an ADA accessway between two ADA parking stalls shown that would be a suitable location. The walkway should connect to the existing sidewalk on $35^{\text {th }}$ Street and there may be a need for a ramp in the parking area. The crossing is already shown with suitable markings

The site plan layout provides pedestrian access and circulation around the perimeter of the building where applicable (not including adjacent to the drive-through lane, where it would be between the drive-through and the building).



### 5.0 RECOMMENDATIONS AND CONCLUSION

Crash Analysis

No crash reducing measures are necessary to accommodate the proposed project. The accident histories show that the intersection at $35^{\mathrm{th}}$ Street and HWY 101 has been relatively safe for the five-year period of available crash data received from ODOT. HWY 101 at the site frontage is not identified as a $95^{\text {th }}$ percentile ODOT Safety Priority Index System (SPIS) location on the most recent SPIS maps.

## Access

Based on access locations and geometries, there are no unusual safety related concerns associated with the existing and proposed access locations or conditions. The site's proposed driveway approach on the HWY 101 is proposed as a right-in right-out only restricted access. The site plan shows a second unrestricted access on $35^{\text {th }}$ Street that may have an access easement associated with it for future development of the parcels on the site that are not proposed to be developed at this time. It is recommended that the site driveway approach on $35^{\mathrm{th}}$ Street be considered for relocation to approximately 70 to 75 feet to the west to improve onsite circulation during peak drive-through usage. There appears to be adequate available queue storage at the westbound approach to HWY 101 with the recommended relocation of the driveway and the recommended driveway location meets the spacing criteria from the intersection at HWY 101 required by City of Florence code. All accesses are necessary to accommodate existing on-site circulation of the expected site generated levels of vehicle traffic and for access and circulation of delivery vehicles.

## Trip Generation

The proposed development is projected to generate an average of 120 AM and 98 PM new vehicle trips and 1,409 new average daily trips accessing the site split between the approach at the site driveway at the frontage on HWY 101 and the unrestricted access on $35^{\text {th }}$ Street .

## Intersection Performance

The studied public street intersection at $35^{\text {th }}$ Street and HWY 101 and the site driveway approaches are projected to operate at acceptable levels of service under all analyzed scenarios. The LOS are currently within the mobility standards of the City of Florence for LOS E or better and are not projected to exceed that standard under the analyzed conditions for the anticipated year 2021 year of opening or the future year 2026 post development analysis scenarios. As modeled, the vehicle queue lengths are accommodated within available storage and no modifications are necessary to accommodate incremental traffic increases from the proposed development on the public street frontages and approaches.

## Site Traffic Control

The site plan was reviewed and recommendations were made to improve access and circulation on the site and to and from the public streets. The improvements are mostly related to providing inexpensive signage and pavement markings to enhance situational awareness of pedestrians on the site and provide logical wayfinding for the general traveling public to ensure traffic flows on and off the site efficiently and safely.

## Conclusion

In summary, the proposed development project will not cause significant adverse impact on the performance of the public transportation system.

## APPENDIX A

## SCOPE OF WORK MEMO AND E-MAILS

## Dan H

To:
Mike Miller
Subject:
RE: 35th St BK Site TIA

From: Mike Miller [mike.miller@ci.florence.or.us](mailto:mike.miller@ci.florence.or.us)
Sent: Thursday, March 19, 2020 1:46 PM
To: Dan H [danh@branchengineering.com](mailto:danh@branchengineering.com)
Cc: Wendy Farley-Campbell [wendy.farleycampbell@ci.florence.or.us](mailto:wendy.farleycampbell@ci.florence.or.us); planningdepartment [planningdepartment@ci.florence.or.us](mailto:planningdepartment@ci.florence.or.us)
Subject: RE: 35th St BK Site TIA

Hi Dan,

Myself and one of our engineers reviewed the scoping memo (attached for the benefit of our planning staff) for the Burger King TIA. We do not have any concerns, but wanted you to be aware of a couple of items:

- The $35^{\text {th }}$ and Hwy 101 intersection will have some impact due to a new proposed residential development at Rhody and $35^{\text {th }}$. You might want to include the information in your analysis (I have attached their TIA)
- If a traffic count is used for the study, we recommend if possible, to complete a count in July/August instead of just relying on a seasonal adjustment.

Thank you!

Mike

From: Dan H [danh@branchengineering.com](mailto:danh@branchengineering.com)
Sent: Monday, March 16, 2020 9:33 AM
To: Mike Miller [mike.miller@ci.florence.or.us](mailto:mike.miller@ci.florence.or.us)
Subject: [Suspected SPAM] 35th St BK Site TIA
Importance: Low
Hi Mike,
I got your voice mail last week, and have prepared the attached memo with the proposed scope of work for the forthcoming TIA. Per your request, we will look at the driveway on $35^{\text {th }}$ with respect to offsetting left turns with the intersection of Redwood Street and discuss the access configuration as well as looking at the intersections of $35^{\text {th }}$ and Redwood and $35^{\text {th }}$ and HWY 101 for operations and safety.

Please let me know if this sounds reasonable.

Thanks,

DAN HAGA, P.E.
Project Engineer

## BRANCH ENGINEERING, INC.

310 5th Street, Springfield, OR 97477
p_ 541.746.0637

## TECHNICAL MEMORANDUM

DATE: March 16, 2020

# civil - transportation structural - geotechnica S URVEYING 

PROJECT: HWY 101/35 ${ }^{\text {th }}$ St. BK Site, BEI Project 20-113

TO: Mike Miller
City of Florence Public Works

FROM: Dan Haga, P.E.
Branch Engineering

RE: Burger King Development Site, Map lot: 18-12-23-22-06800


EXPIRES: JUNE 30, 2021

In an effort to assist the City of Florence in the process of concurring on the scope of traffic impact analysis required to address applicable approval criteria for the proposed development of the subject site referred to in the subject line, identified as taxlot 06800 of tax map 18-12-23-22, I am supplying this memorandum summarizing the land use assumptions and associated site generated traffic, the proposed study area and details to be included in the forthcoming analysis. The proposed uses of the site are in compliance with the City of Florence's existing Highway Commercial zone and Comprehensive Plan Map designations for the site. The result of this memorandum is intended to be the basis for concurring on the scope of analysis required to analyze development impacts in compliance with City of Florence Development Code, Title 10, Chapter 1, Section 10-1-1-4: E and Title 10, Chapter 35, Section 10-35-2-5.

## Existing Conditions

The subject site is located in the northeast quadrant of the intersection of HWY 101 at $35^{\text {th }}$ Street. The site is within the urban growth boundary and city limits and is within an urbanized area of Florence, Oregon. The property is approximately 1.85 acres in size and is currently vacant and undeveloped. The aerial photograph on the following page shows the approximate site location.


Site Location (Aerial by Lane County Maps)
The currently proposed development of the site includes a 2.992 KSF fast-food restaurant with a drive-through window and improvements for parking on the site. Primary access to the proposed development would be at a right-in right-out approach to be constructed at the site's frontage on the east side of HWY 101, and at an unrestricted access at the site's frontage on the north side of $35^{\text {th }}$ Street west of the intersection at Redwood Street. There is a possibility for secondary access to the vacated right-of-way of $36^{\mathrm{th}}$ Street that serves as the primary access to the adjacent property to the north. The vacated $36^{\text {th }}$ Street right-of-way to the north borders the subject site's north property boundary. The adjacent property to the north is developed as a full-service restaurant.

## Trip Generation

To project trip generation for the proposed development's use, a reference was made to the Trip Generation Manual, $10^{\text {th }}$ Edition, and the Trip Generation Handbook, $3^{\text {rd }}$ Edition, both published by the Institute of Transportation Engineers (ITE). The table on the following page summarizes the projected peak hour site generated traffic based on the available published data:

| Florence Burger King Site Trip Generation |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use/ ITE Land Use Code | Ind. Variable | Units (QTY) | TG Rate <br> Trips/Unit | Trips | $\begin{aligned} & \text { Trips } \\ & \text { IN } \\ & (\% / \#) \\ & \hline \end{aligned}$ | Trips OUT (\%/\#) |
| AM Peak Hour |  |  |  |  |  |  |
| Fast Food Restaurant with DriveThrough Window/934 | KSF GFA | 2.992 | 40.19 | 120 | 51\%/61 | 49\%/59 |
| Pass-By Trips: |  |  | 49\% | 59 | 30 | 29 |
| PM Peak Hour |  |  |  |  |  |  |
| Fast Food Restaurant with DriveThrough Window/934 | KSF GFA | 2.992 | 32.67 | 98 | 52\%/51 | 48\%/47 |
| Pass-By Trips: |  |  | 50\% | 49 | 25 | 24 |
| ADT |  |  |  |  |  |  |
| Fast Food Restaurant with DriveThrough Window/934 | KSF GFA | 2.992 | 470.95 | 1,409 | 50\%/705 | 50\%/704 |

As indicated above, the proposed post-development traffic conditions could result in up to 120 new AM peak hour vehicle trips and up to 98 new PM peak hour vehicle trips accessing the site. Out of the total peak hour vehicle trip totals, 49 percent of the AM and 50 percent of the PM peak hour ingress and egress trips would be expected to be pass-by trips, which are trips already on the roadway that would be expected to make a route diversion at the driveway to enter and exit the site, but will otherwise continue in the same direction they were traveling upon leaving the site. Pass-by trip occur between primary origins and destinations and do not add traffic to intersections or facilities, other than the site driveway approach. Per City of Florence Zoning Administration, any development that generates 250 or more vehicle trips per day is required to provide a traffic impact analysis. Because the proposed development will generate a potential 1,409 ADT (daily) trips during typical weekday traffic conditions, the proposed development requires a traffic impact analysis.

## Study Area and Analysis Scoping

The site plan is proposing a primary access at the site's frontage on HWY 101, that will be restricted to right-in and right-out only turning movements. HWY 101 at the frontage is within the jurisdiction of the Oregon Department of Transportation. A second unrestricted primary access is proposed to be located on the site's $35^{\text {th }}$ Street frontage, which is within the City of Florence's jurisdiction. A secondary access connection may be provided with a crossover access agreement with the adjacent neighbor to the north, where there is an existing unrestricted access that serves the existing restaurant at that site. For the scope of this traffic impact analysis, all post-development site generated traffic will be assumed to utilize the site's driveways for access. The potential crossover access connection to the adjacent unrestricted access is unlikely to detract a lot of additional turning movements, since it is out of direction for left-turns from the site to HWY 101 southbound. Once developed and a connection is established, there may be a few left-turns into the shared access from the north, but there does not appear to be a particular benefit to turning left at the shared approach over turning left at the traffic signal at HWY 101 and $35^{\text {th }}$ Street, since the public roads have a higher rate of speed than a route through the parking area, and the site's parking area and drive-through are aligned the accommodate arrivals from the site's frontage on $35^{\mathrm{th}}$ Street. Per City of Florence
comments, offsetting left -turns at the proposed site driveway approach on $35^{\text {th }}$ Street with respect to the public street intersection at $35^{\text {th }}$ St. and Redwood St. will be evaluated and discussed.

The Oregon Department of Transportation (ODOT) has indicated that ODOT will not require a traffic impact analysis with this development as the proposed access on HWY 101 remains proposed as a right-in right-out restricted access. The City of Florence requires a traffic impact analysis and has indicated that the TIA should include both of the site's driveway approaches, the intersection at HWY 101 and $35^{\text {th }}$ St. and at the intersection of $35^{\text {th }}$ Street and Redwood St., therefore; these intersections will be analyzed for operational performance, queuing and safety.

## Analysis Year(s)

The City of Florence Traffic Impact Analysis requirements do not contain analysis details that include a definition of the study area that the TIA needs to analyze, nor what analysis years need to be included in a TIA. Per the Oregon Department of Transportation's development review guidelines, Table 3.3, proposed developments that require a traffic impact analysis from ODOT that generate between 1,000 and 2,999 ADT and include a single-phase development horizon year are suggested to include the year of opening and a five-year post-development planning horizon year in the analysis scenarios. Although ODOT does not require an analysis for this proposed development, the analysis required by the City of Florence is proposed to include the year of opening (assumed to be the current year 2020/year of application), and a five-year post-development horizon year, 2025, consistent with the suggested analysis year scenarios by ODOT. The forthcoming analysis will include AM and PM peak hour scenarios with and without additional traffic from the proposed development during those years.

## Pipeline Traffic

Branch Engineering is not aware of any nearby pending land use applications or traffic impact analyses that may add a significant amount of incremental traffic to the study area that is in the approval process at the time of scoping for this application. If there are any known in-process developments that have pending approved TIA applications in with the City and have not been constructed and opened yet at the time of this scoping exercise, please provide any information on those if they include pipeline trips at the site's frontages or at the intersection of $35^{\text {th }}$ Street and HWY 101.

## Conclusion

In summary, based on the City of Florence Traffic Impact Analysis trip generation threshold of 250 ADT trips provided in Title 10, Chapter 1, Section 10-1-1-4: E, the proposed Burger King development site will require a traffic impact analysis to determine if there is an identifiable impact to existing and background future traffic performance conditions during the design hour of the year of application (year 2020) and at the end of a five (5) year post-development planning horizon (year 2025). The forthcoming analysis will determine traffic impacts based on any updates to the preliminary site plan. We look forward to working with you to get concurrence on the scope of work to be provided through this traffic impact analyses and land development approval process. Please do not hesitate to contact me if I can provide any additional information.

Dan H

From:
Sent:
To:
Cc:
Subject:

NELSON Brian S * Scott [Brian.S.NELSON@odot.state.or.us](mailto:Brian.S.NELSON@odot.state.or.us)
Tuesday, March 10, 2020 1:52 PM
Dan H; BLAIR Keith P
(mike.miller@ci.florence.or.us); BAUMGARTNER Douglas G
RE: Florence BK TIA

Hi Dan, our concerns related to the new private approach would be addressed with a restriction to right-in/right-out. If the developer agrees to this no TIA will be required with the approach permit process. If the developer is not in agreement with the right-in/right-out approach they should expect a TIA addressing our operational and safety concerns to be required.

If the City's code requires a TIA that looks at highway intersections we are always happy to assist with that review. Keith is handling a few positions right now as he was promoted to the Region 2 Traffic Manager, but we will find resources to take a look at it if the City wishes to include us.

Below is a design for a curb tight sidewalk right-in/right-out porkchop that was done about 5 years ago in McMinnville. It operates pretty effectively.


B Scott Nelson, P.E.<br>Region 2 Access Management Engineer

455 Airport Rd SE, Bldg. B
Salem, OR 97301
Office 503.986.2882
Cell 503.602.0703

From: Dan H [danh@branchengineering.com](mailto:danh@branchengineering.com)
Sent: Tuesday, March 10, 2020 12:04 PM
To: NELSON Brian S * Scott [Brian.S.NELSON@odot.state.or.us](mailto:Brian.S.NELSON@odot.state.or.us); BLAIR Keith P [Keith.P.BLAIR@odot.state.or.us](mailto:Keith.P.BLAIR@odot.state.or.us) Cc: (mike.miller@ci.florence.or.us) [mike.miller@ci.florence.or.us](mailto:mike.miller@ci.florence.or.us)
Subject: Florence BK TIA
Hi Scott and Keith,
I spoke with Scott a while back regarding a site in Florence at the NE corner of HWY 101 and $35^{\text {th }}$ Street regarding access and TIA applicability. It sounded like a TIA may not necessarily be required by ODOT. I have attached a preliminary site plan for the proposed development. It looks like the plan includes approximately 2.992 KSF GFA, which equates to 98 PM peak hour trips, with $50 \%$ of those as pass-by trips per the $10^{\text {th }}$ Edition ITE and Trip Gen Handbook, $3^{\text {rd }}$ Edition. The AM trip gen comes to 120 trips, with $48 \%$ of those as pass-by. The ADT based on the ITE rate would be 1409 .

Can we get confirmation if ODOT will or will not require a TIA for this site? If a study is required, please let me know if you need anything for scoping.

Also, FYI, the City's Trip Generation threshold is 250 ADT trips, so it looks like we will at least need to provide a TIA to meet the City's requirements. I contacted Mike Miller (Public Works Director) regarding scoping for that, and am waiting for a response to see what the City requires, or whom I should contact as engineer of record for the City to negotiate scoping for the City. Will ODOT want to review the TIA if it isn't required by ODOT? I suspect that most of the potential for impact would be on HWY 101, with primary trip origins and destinations filtering out fairly evenly to side streets in the vicinity. It looks like they are proposing access control on HWY 101 at the proposed driveway approach.

Thanks,

DAN HAGA, P.E.
Project Engineer

## BRANCH ENGINEERING, INC.

310 5th Street, Springfield, OR 97477
p_ 541.746.0637
www.branchengineering.com

## APPENDIX B

## SITE PLAN



## APPENDIX C

## RHODY EXPRESS ROUTE SCHEDULES




RHODY EXPRESS IS ACCESSIBLE The Rhody Express vehicle is accessible to people who use mobility devices. The driver will assist with lift boarding and securement.

Anyone can request the lift to help board the bus. BUS BUDDIES ARE HERE TO HELP If you need help learning how to use the bus or would like a buddy to ride with you the first few times, call the Rhody Express and ask for a Bus Buddy. These experienced riders are available and ready to help. 541-902-2067 or 7-1-1 (TTY - Oregon Relay).

WHAT IF I CANNOT ACCESS THE BUS? If you experience a disabling condition that prevents you from using the Rhody Express, you may qualify for the Rhody Dial-a-Ride, a
service that can take you from point $A$ to point $B$. You must live within three-quarters of a mile from the bus route and participate in an
in-person eligibility assessment to access
 eligibility questions, call the RideSource Call Center toll free at 1-877-800-9899 or 7-1-1 (TTY - Oregon Relay).


| NORTH LOOP |  |  |  |
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| Leave <br> Florence Food Share | Bi-Mart | Fred Meyer | Arrive <br> Grocery Outlet |
| 8 | 9 | 10 | (1) |
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| SOUTH LOOP |  |  |  |  |  |  |  |
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| Leave Grocery Outlet | Dollar <br> Tree/ <br> Kozy <br> Kitchen | Siuslaw <br> Public <br> Library | Peace Harbor Hospital | Old Town Park | Safeway | Three Rivers Casino | Arrive <br> Florence Food <br> Share |
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## RIDING FROM

 SOUTH TO NORTHThe Rhody Express operates a 60-minute circuit by first traveling the South Loop and then the North Loop.

If your destination is not on the loop on which you are boarding, remain on the bus as it switches to the next loop, and you will reach your destination.


## APPENDIX D

## CRASH DATA

| General \& Site Information |  |
| :--- | :--- |
| Analyst: | DNH |
| Agency/Company: | Branch Engineering, Inc |
| Date: | (15/2020 |
| Project Name: | Burger King TIA |


| Intersection Crash Data |  |  |  |  |  |  |  |
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|  | Intersection | Year |  |  |  |  |  |
| Intersection | Type | 2014 | 2015 | 2016 | 2017 | 2018 | Total |
| 35th at HWY 101 | Urban 4SG | 2 | 1 | 3 | 2 | 4 | 12 |
| 35th at Redwood | Urban 3ST | 0 | 0 | 0 | 0 | 0 | 0 |
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|  |  |  |  |  |  |  | 0 |
|  | Total | 2 | 1 | 3 | 2 | 4 | 12 |


| Antersection Population Type Crash Rate Crash Rate per intersection type |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Sum of <br> Crashes | Sum of 5- <br> year MEV | Avg Crash <br> Rate for Ref <br> Pop. | INT in Pop |
| Intersection Pop. Type | 0 | 0 |  |  |
| Rural 3SG | 0 | 0 |  |  |
| Rural 3ST | 0 | 0 |  |  |
| Rural 4SG | 0 | 0 |  |  |
| Rural 4ST | 0 | 2 | 0.0000 | 1 |
| Urban 3ST | 0 | 0 |  |  |
| Urban 3SG | 0 | 0 |  |  |
| Urban 4ST | 12 | 29 | 0.4110 | 1 |
| Urban 4SG |  |  |  |  |


| Critical Rate Calculation |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | AADT Entering Intersection | 5-year MEV | Crash Total | Intersection Population Type | Intersection Crash Rate | Reference Population Crash Rate | Critical Rate | Over Critical |
| 35th at HWY 101 | 16,000 | 29.2 | 12 | Urban 4SG | 0.41 | 0.41 | 0.62 | Under |
| 35th at Redwood | 1,200 | 2.2 | 0 | Urban 3ST | 0.00 | 0.00 | 0.23 | Under |
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OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
CRASH SUMMARIES BY YEAR BY COLLISION TYPE
$\mathbf{3 5 T H ~ S T ~ a t ~ R E D W O O D ~ S T , ~ C i t y ~ o f ~ F l o r e n c e , ~ L a n e ~ C o u n t y , ~ 0 1 / 0 1 / 2 0 1 4 ~ t o ~ 1 2 / 3 1 / 2 0 1 8 ~}$
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35TH ST at REDWOOD ST, City of Florence, Lane County, 01/01/2014 to 12/31/2018
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$\underset{\text { CRASHES }}{\text { FATAL }}$
CDS150
$06 / 15 / 2020$
COLLISION TYPE
FINAL TOTAL
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$01 / 01 / 2004$, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

oregon.. department of transportation - transportation development diviston Ransportation data section - Crash anayiysis and reporting unit
 of Florence, Lane
of 12 Crash records shown. cords shown.

| SER\# | P R | J s w | w date | CLASS |  | City street |  | int-TYPE |  |  |  |  |  | SPCL USE |  |  |  |  |  |  |  |  |  |  |  |
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| invest | Eau | I co | O DAY | DIST |  | FIRST STREET | RD CHAR | (MEdian) | InT-rel | OFFRD | WTHR | CRASH |  | trid ety | move |  |  | A | s |  |  |  |  |  |  |
| RD DPT | E L G | N H R | R time | FROM |  | second street | direct | Legs | traf- | RNDBT | SURF | CoLl |  | OWNER | from | PRTC | INJ | G | E | Licns | ped |  |  |  |  |
| UNLOC? | D c S | V L K | K LAT | Long |  | LRS | LOCTN | (\#LANES) | Contl | DRVWY | LIGHT | SVRTY Vid |  | TYPE | To | P. Type | SVRTY | E | x | Res | Loc | ERROR | ACT | Event | cause |
|  |  |  |  |  |  |  |  |  |  |  |  |  | 02 N | NONE 0 | TURN-L |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | prvie | s -w |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | pSngr car |  | 02 PSNG | inje | 95 | F |  |  | 000 | 000 |  | 00 |
| 01865 | N N N | N N | N 05/26/2016 |  | 14 | OREGON COASt hy | inter | Cross | N | N | CLR | 0-1 L-TURN 0 | 01 | none 0 | turn-L |  |  |  |  |  |  |  |  |  | 02 |
| No RPT |  |  | тн |  |  | 35 TH ST | cN |  | tre Signal | N | DRY | turn |  | prvie | w - |  |  |  |  |  |  |  | 000 |  | 00 |
| N |  |  | 10A |  |  |  | 02 | 0 |  | N | DAY | INJ |  | psngr car |  | 01 DrVr | none | 25 | F | OR-Y |  | 004,028 | 000 |  | 02 |
| N |  |  | 435948.99 | $\begin{aligned} & -124 \\ & 5.19 \end{aligned}$ |  | 000900100500 |  |  |  |  |  |  |  |  |  |  |  |  |  | OR<25 |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | none 0 | strght |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | prvie | e -w |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | psngr car |  | 01 Drvr | InJC | 69 | F | OR-Y |  | 000 | 000 |  | 00 |
| 02544 | N N N |  | 07/13/2016 |  | 14 | OREGON COASt hy | inter | cross | N | N | CLR | ANGL-OTH | 01 | none 0 | strght |  |  |  |  |  |  |  |  |  | 04 |
| State |  |  | wE |  |  | 35TH ST | $\mathrm{cN}^{\text {a }}$ |  | Tre SIGNAL | N | DRY | angl |  | prvie | N -s |  |  |  |  |  |  |  | 000 |  | 00 |
| N |  |  | 2P |  |  |  | 01 | 0 |  | N | DAY | InJ |  | pSNGR Car |  | 01 DRVR | InJC | 71 | M | OR-Y |  | 020 | 000 |  | 04 |
| N |  |  | 435948.99 | $\begin{aligned} & -124 \\ & 5.19 \end{aligned}$ |  | 000900100s00 |  |  |  |  |  |  |  |  |  |  |  |  |  | OR<25 |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | none 0 | Strght |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | prvie | e -w |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | fsngr car |  | 01 Drvr | none | 62 | F | $\begin{aligned} & \text { OR-Y } \\ & \text { OR }<25 \end{aligned}$ |  | 000 | 000 |  | 00 |
| 01567 | N N N | N N | n 05/07/2017 |  | 14 | OREGON COASt hy | inter | cross | N | N | CLR | --1 L-TURN 0 | 01 | none 0 | tURN-L |  |  |  |  |  |  |  |  |  | 02 |
| county |  |  | su |  |  | 35TH ST | $\mathrm{cN}^{\text {a }}$ |  | tre Signal | N | DRY | turn |  | prvie | s-w |  |  |  |  |  |  |  | 000 |  | 00 |
| N |  |  | 2P |  |  |  | 01 | 0 |  | N | DAY | InJ |  | fsngr car |  | 01 DRVR | InJC | 46 | M | suse |  | 004,028 | 000 |  | 02 |
| N |  |  | 435948.99 | $\begin{aligned} & -124 \\ & 5.19 \end{aligned}$ |  | 000900100s00 |  |  |  |  |  |  |  |  |  |  |  |  |  | N-res |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | 01 | none 0 | turn-L |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | Prvie | s-w |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | f ${ }^{\text {SNGR C Car }}$ |  | 02 pSNG | injc | 63 | M |  |  | 000 | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  | 01 | none 0 | turn-L |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | prvie | s -w |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | fsngr car |  | 03 PSNG | inje | 03 | F |  |  | 000 | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  | 02 | none 0 | STRGHT |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | prvte | N-S |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | psngr car |  | 01 DRVR | injb | 24 | F | $\begin{aligned} & \text { OR-Y } \\ & \text { OR>25 } \end{aligned}$ |  | 000 | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | none O | Strght |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | prvte | N-s |  |  |  |  |  |  |  | 000 |  | 00 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | ${ }^{\text {f SNGR }}$ car |  | 02 PSNG | InJB | 01 | F |  |  | 000 | 000 |  | 00 |


ACTION CODE TRANSLATION LIST

| 000 | NONE | NO ACTION OR NON-WARRANTED |
| :---: | :---: | :---: |
| 001 | SKIDDED | SKIDDED |
| 002 | ON/OFF V | GETTING ON OR OFF STOPPED OR PARKED VEHICLE |
| 003 | LOAD OVR | OVERHANGING LOAD STRUCK ANOTHER VEHICLE, ETC. |
| 006 | SLOW DN | SLOWED DOWN |
| 007 | AVoiding | AVOIDING MANEUVER |
| 008 | PAR PARK | PARALLEL PARKING |
| 009 | Ang Park | ANGLE PARKING |
| 010 | INTERFERE | PASSENGER INTERFERING WITH DRIVER |
| 011 | Stopped | Stopped in trafeic not Waiting to make a left turn |
| 012 | STP/L TRN | Stopped because of Left turn signal or waiting, etc. |
| 013 | STP TURN | Stopped while executing a turn |
| 014 | EMR V PKD | EMERGENCY VEHICLE LEGALLY PARKED IN THE ROADWAY |
| 015 | GO A/STOP | PROCEED AFTER STOPPING FOR A STOP SIGN/FLASHING RED. |
| 016 | TRN A/RED | TURNED ON RED AFTER STOPPING |
| 017 | LOSTCTRL | LOST CONTROL OF VEHICLE |
| 018 | EXIT DWY | ENTERING STREET OR HIGHWAY FROM ALLEY OR DRIVEWAY |
| 019 | ENTR DWY | ENTERING ALLEY OR DRIVEWAY FROM STREET OR HIGHWAY |
| 020 | Str Entr | BEFORE ENTERING ROADWAY, STRUCK PEDESTRIAN, ETC. ON SIDEWALK OR SHOULDER |
| 021 | NO DRVR | CAR RAN AWAY - NO DRIVER |
| 022 | PREV COL | STRUCK, OR WAS STRUCK BY, VEHICLE OR PEDESTRIAN IN PRIOR COLLISION BEFORE ACC. STABILIZED |
| 023 | STALLED | VEHICLE STALLED OR DISABLED |
| 024 | DRVR DEAD | DEAD BY UNASSOCIATED CAUSE |
| 025 | FATIGUE | fatigued, Sleepy, ASLEEP |
| 026 | SUN | DRIVER BLINDED BY SUN |
| 027 | HDLGHTS | DRIVER BLINDED BY HEADLIGHTS |
| 028 | ILLNESS | PHYSICALLY ILL |
| 029 | THRU MED | VEHICLE CROSSED, PLUNGED OVER, OR THROUGH MEDIAN BARRIER |
| 030 | PURSUIT | PURSUING OR ATTEMPTING TO STOP A VEHICLE |
| 031 | PASSING | PASSING SITUATION |
| 032 | PRKOFFRD | VEHICLE PARKED BEYOND CURB OR SHOULDER |
| 033 | CROS MED | VEHICLE CROSSED EARTH OR GRASS MEDIAN |
| 034 | X N/SGNL | CROSSING AT INTERSECTION - NO TRAFEIC SIGNAL PRESENT |
| 035 | X W/ SGNL | CROSSING AT INTERSECTION - TRAFFIC SIGNAL PRESENT |
| 036 | DIAGONAL | CROSSING AT INTERSECTION - DIAGONALLY |
| 037 | BTWN INT | CROSSING BETWEEN INTERSECTIONS |
| 038 | DISTRACT | DRIVER'S Attention distracted |
| 039 | W/TRAF-S | WALKING, RUNNING, RIDING, ETC., ON SHOULDER WITH TRAFFIC |
| 040 | A/TRAF-S | WALKING, RUNNING, RIDING, ETC., ON SHOULDER FACING TRAFFIC |
| 041 | W/TRAF-P | WALKING, RUNNING, RIDING, ETC., ON PAVEMENT WITH TRAFFIC |
| 042 | A/TRAF-P | WALKING, RUNNING, RIDING, ETC., ON PAVEMENT FACING TRAFFIC |
| 043 | PLAYINRD | PLAYING IN STREET OR ROAD |
| 044 | PUSH MV | PUSHING OR WORKING ON VEHICLE IN ROAD OR ON SHOULDER |
| 045 | WORK ON | WORKING IN ROADWAY OR ALONG SHOULDER |
| 046 | W/ TRAFIC | NON-MOTORIST WALKING, RUNNING, RIDING, ETC. WITH TRAFFIC |
| 047 | A/ TRAFIC | NON-MOTORIST WALKING, RUNNING, RIDING, ETC. FACING TRAFFIC |
| 050 | LAY ON RD | STANDING OR LYING IN ROADWAY |
| 051 | ENT OFFRD | ENTERING / STARTING IN TRAFFIC LANE FROM OFF ROAD |
| 052 | MERGING | MERGING |
| 055 | SPRAY | BLINDED BY WATER SPRAY |


|  | ACTION CODE |  |
| :---: | :--- | :--- |
| ARANSLATION LIST |  |  |
| ACTION | SHORT |  |
| CODE | DESCRIPTION | LONG DESCRIPTION |
| 088 | OTHER | OTHER ACTION |
| 099 | UNK | UNKNOWN ACTION |



| CODE | DESCRIPTION | LONG DESCRIPTION |
| :---: | :--- | :--- |
| 00 | NO CODE | NO CAUSE ASSOCIATED AT THIS LEVEL |
| 01 | TOO-FAST | TOO FAST FOR CONDITIONS (NOT EXCEED POSTED SPEED |
| 02 | NO-YIELD | DID NOT YIELD RIGHT-OF-WAY |
| 03 | PAS-STOP | PASSED STOP SIGN OR RED FLASHER |
| 04 | DIS SIG | DISREGARDED TRAFFIC SIGNAL |
| 05 | LEFT-CTR | DROVE LEFT OF CENTER ON TWO-WAY ROAD; STRADDLING |
| 06 | IMP-OVER | IMPROPER OVERTAKING |
| 07 | TOO-CLOS | FOLLOWED TOO CLOSELY |
| 08 | IMP-TURN | MADE IMPROPER TURN |
| 09 | DRINKING | ALCOHOL OR DRUG INVOLVED |
| 10 | OTHR-IMP | OTHER IMPROPER DRIVING |
| 11 | MECH-DEF | MECHANICAL DEFECT |
| 12 | OTHER | OTHER (NOT IMPROPER DRIVING) |
| 13 | IMP LN C | IMPROPER CHANGE OF TRAFFIC LANES |
| 14 | DIS TCD | DISREGARDED OTHER TRAFFIC CONTROL DEVICE |
| 15 | WRNG WAY | WRONG WAY ON ONE-WAY ROAD; WRONG SIDE DIVIDED RO; |
| 16 | FATIGUE | DRIVER DROWSY/FATIGUED/SLEEPY |
| 17 | ILLNESS | PHYSICAL ILLNESS |
| 18 | IN RDWY | NON-MOTORIST ILLEGALLY IN ROADWAY |
| 19 | NT VISBL | NON-MOTORIST NOT VISIBLE; NON-REFLECTIVE CLOTHIN |
| 20 | IMP PKNG | VEHICLE IMPROPERLY PARKED |
| 21 | DEF STER | DEFECTIVE STEERING MECHANISM |
| 22 | DEF BRKE | INADEQUATE OR NO BRAKES |
| 24 | LOADSHFT | VEHICLE LOST LOAD OR LOAD SHIFTED |
| 25 | TIREFAIL | TIRE FAILURE |
| 26 | PHANTOM | PHANTOM / NON-CONTACT VEHICLE |
| 27 | INATTENT | INATTENTION |
| 28 | NM INATT | NON-MOTORIST INATTENTION |
| 29 | F AVOID | FAILED TO AVOID VEHICLE AHEAD |
| 30 | SPEED | DRIVING IN EXCESS OF POSTED SPEED |
| 31 | RACING | SPEED RACING (PER PAR) |
| 32 | CARELESS | CARELESS DRIVING (PER PAR) |
| 33 | RECKLESS | RECKLESS DRIVING (PER PAR) |
| 34 | AGGRESV | AGGRESSIVE DRIVING (PER PAR) |
| 35 | RD RAGE | ROAD RAGE (PER PAR) |
| 40 | VIEW OBS | VIEW OBSCURED |
| 50 | USED MDN | IMPROPER USE OF MEDIAN OR SHOULDER |
| 51 | FAIL LN | FAILED TO MAINTAIN LANE |
| 52 | OFF RD | RAN OFF ROAD |



|  | DRIVER LICENSE CODE TRANSLATION LIST |  |
| :---: | :--- | :--- |
|  |  |  |
| LIC | SHORT |  |
| CODE | DESC | LONG DESCRIPTION |
| 0 | NONE | NOT LICENSED (HAD NEVER BE |
| 1 | OR-Y | VALID OREGON LICENSE |
| 2 | OTH-Y | VALID LICENSE, OTHER STATE |
| 3 | SUSP | SUSPENDED/REVOKED |
| 4 | EXP | EXPIRED |
| 8 | N-VAL | OTHER NON-VALID LICENSE |
| 9 | UNK | UNKNOWN IF DRIVER WAS LICE |

ERROR CODE TRANSLATION LIST
WIDE TURN
$\begin{array}{ll}\text { RROR } & \text { SHORT } \\ \text { CODE } & \text { DESCRIPTION FULL DESCRIPTION }\end{array}$

PASSING IN "NO PASSING" ZONE
PASSING IN FRONT OF ONCOMING TRAFFIC
PASSING IN FRONT OF ONCOMING TRAFFIC
 DRIVING THROUGH SAFETY ZONE OR OVER ISLAND
FAILED TO STOP FOR SCHOOL BUS
IMPROPER OR NO LIGHTS (VEHICLE IN TRAFFIC)
INATTENTION (FAILURE TO DIM LIGHTS PRIOR TO 4/1/97)
DRIVING UNSAFE VEHICLE (NO OTHER ERROR APPARENT)
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLE
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
DISREGARDED OTHER DRIVER'S SIGNAL
DISREGARDED TRAFFIC SIGNAL
DISREGARDED STOP SIGN OR FLASHING RED
DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
DISREGARDED POLICE OFFICER OR FLAGMAN
DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
DID NOT HAVE RIGHT-OF-WAY
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
DISREGARDED OTHER DRIVER'S SIGNAL
DISREGARDED TRAFFIC SIGNAL
DISREGARDED STOP SIGN OR FLASHING RED
DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
DISREGARDED POLICE OFFICER OR FLAGMAN
DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
DID NOT HAVE RIGHT-OF-WAY
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
DISREGARDED OTHER DRIVER'S SIGNAL
DISREGARDED TRAFFIC SIGNAL
DISREGARDED STOP SIGN OR FLASHING RED
DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
DISREGARDED POLICE OFFICER OR FLAGMAN
DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
DID NOT HAVE RIGHT-OF-WAY
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
DISREGARDED OTHER DRIVER'S SIGNAL
DISREGARDED TRAFFIC SIGNAL
DISREGARDED STOP SIGN OR FLASHING RED
DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
DISREGARDED POLICE OFFICER OR FLAGMAN
DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
DID NOT HAVE RIGHT-OF-WAY
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
DISREGARDED OTHER DRIVER'S SIGNAL
DISREGARDED TRAFFIC SIGNAL
DISREGARDED STOP SIGN OR FLASHING RED
DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
DISREGARDED POLICE OFFICER OR FLAGMAN
DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
DID NOT HAVE RIGHT-OF-WAY
ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
DISREGARDED OTHER DRIVER'S SIGNAL
DISREGARDED TRAFFIC SIGNAL
DISREGARDED STOP SIGN OR FLASHING RED
DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
DISREGARDED POLICE OFFICER OR FLAGMAN
DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
DID NOT HAVE RIGHT-OF-WAY
DID NOT HAVE RIGHT-OF-WAY
FAILED TO YIELD RIGHT-OF-WAY TO PEDESTRIAN
PASSING ON A CURVE
PASSING ON THE WRONG SIDE
PASSING ON STRAIGHT ROAD UNDER UNSAFE CONDITIONS
PASSED VEHICLE STOPPED AT CROSSWALK FOR PEDESTRIAN
PASSING AT INTERSECTION
PASSING ON CREST OF HILL
FAILED TO OBEY MANDATORY TRAFFIC TURN SIGNAL, SIGN OR LANE MARKINGS
LEFT TURN IN FRONT OF ONCOMING TRAFFIC LEFT TURN WHERE PROHIBITED
TURNED INTO WRONG LANE
U-TURNED ILLEGALLY
IMPROPERLY STOPPED IN TRAFFIC LANE
IMPROPER SIGNAL OR FAILURE TO SIGNAL
BACKING IMPROPERLY (NOT PARKING
IMPROPERLY PARKED
IMPROPER START LEAVING PARKED POSITION

ERROR CODE TRANSLATION LIST

[^0]event code translation list

| 001 | FEL/JUMP | OCCUPANT FELL, JUMPED OR WAS EJECTED FROM MOVING VEHICLE |
| :---: | :---: | :---: |
| 002 | INTERFER | PASSENGER INTERFERED WITH DRIVER |
| 003 | BUG INTF | ANIMAL OR INSECT IN VEHICLE INTERFERED WITH DRIVER |
| 004 | INDRCT PED | PEDESTRIAN INDIRECTLY INVOLVED (NOT STRUCK) |
| 005 | SUB-PED | "SUB-PED": PEDESTRIAN INJURED SUBSEQUENT TO COLLISION, ETC. |
| 006 | INDRCT BIK | PEDALCYCLIST INDIRECTLY INVOLVED (NOT STRUCK) |
| 007 | HITCHIKR | HITCHHIKER (SOLICITING A RIDE) |
| 008 | PSNGR TOW | PASSENGER OR NON-MOTORIST BEING TOWED OR PUSHED ON CONVEYANCE |
| 009 | ON/OFF V | GETTING ON/OFF STOPPED/PARKED VEHICLE (OCCUPANTS ONLY; MUST HAVE PHYSICAL CONTACT W/ VEhI |
| 010 | SUB OtRN | OVERTURNED AFTER FIRST HARMFUL EVENT |
| 011 | MV PUSHD | VEHICLE BEING PUSHED |
| 012 | MV TOWED | VEHICLE TOWED OR HAD BEEN TOWING ANOTHER VEHICLE |
| 013 | FORCED | VEHICLE FORCED BY IMPACT INTO ANOTHER VEHICLE, PEDALCYCLIST OR PEDESTRIAN |
| 014 | SET MOTN | VEHICLE SET IN MOTION BY NON-DRIVER (ChILD RELEASED BRAKES, ETC.) |
| 015 | RR ROW | AT OR ON RAILROAD RIGHT-OF-WAY (NOT LIGHT RAIL) |
| 016 | LT RL ROW | AT OR ON LIGHT-RAIL RIGHT-OF-WAY |
| 017 | RR HIT V | TRAIN STRUCK VEHICLE |
| 018 | V HIT RR | VEHICLE STRUCK TRAIN |
| 019 | HIT RR CAR | VEHICLE STRUCK RAILROAD CAR ON ROADWAY |
| 020 | JACKNIFE | JACKKNIFE; TRAILER OR TOWED VEHICLE STRUCK TOWING VEHICLE |
| 021 | trl Otrn | TRAILER OR TOWED VEHICLE OVERTURNED |
| 022 | CN BROKE | TRAILER CONNECTION BROKE |
| 023 | DETACH TRL | DETACHED TRAILING OBJECT STRUCK OTHER VEHICLE, NON-MOTORIST, OR OBJECT |
| 024 | V DOOR OPN | VEHICLE DOOR OPENED INTO ADJACENT TRAFFIC LANE |
| 025 | WHEELOFF | WHEEL CAME OFF |
| 026 | HOOD UP | HOOD FLEW UP |
| 028 | LOAD SHIFT | LOST LOAD, LOAD MOVED OR SHIFTED |
| 029 | TIREFAIL | TIRE FAILURE |
| 030 | PET | PET: CAT, DOG And SIMILAR |
| 031 | LVSTOCK | STOCK: COW, CALF, BULL, STEER, SHEEP, ETC. |
| 032 | HORSE | HORSE, MULE, OR DONKEY |
| 033 | HRSE\&RID | HORSE AND RIDER |
| 034 | GAME | WILD ANIMAL, GAME (INCLUDES BIRDS; NOT DEER OR ELK) |
| 035 | DEER ELK | DEER OR ELK, WAPITI |
| 036 | ANML VEH | ANIMAL-DRAWN VEHICLE |
| 037 | CULVERT | CULVERT, OPEN LOW OR HIGH MANHOLE |
| 038 | Atenuatn | IMPACT ATTENUATOR |
| 039 | PK METER | PARKING METER |
| 040 | CURB | CURB (ALSO NARROW SIDEWALKS ON BRIDGES) |
| 041 | JIGGLE | JIGGLE BAR OR TRAFFIC SNAKE FOR CHANNELIZATION |
| 042 | GDRL END | LEADING EDGE OF GUARDRAIL |
| 043 | GARDRAIL | GUARD RAIL (NOT METAL MEDIAN BARRIER) |
| 044 | BARRIER | MEDIAN BARRIER (RAISED OR METAL) |
| 045 | WALL | RETAINING WALL OR TUNNEL WALL |
| 046 | BR RAIL | BRIDGE RAILING OR PARAPET (ON BRIDGE OR APPROACH) |
| 047 | BR ABUTMnT | BRIDGE ABUTMENT (INCLUDED "APPROACH END" THRU 2013) |
| 048 | BR COLMN | BRIDGE PILLAR OR COLUMN |
| 049 | BR GIRDR | BRIDGE GIRDER (HORIZONTAL BRIDGE STRUCTURE OVERHEAD) |
| 050 | ISLAND | TRAFFIC RAISED ISLAND |
| 051 | GORE | GORE |
| 052 | POLE UNK | POLE - TYPE UNKNOWN |
| 053 | POLE UTL | POLE - POWER OR TELEPHONE |
| 054 | ST LIGHT | POLE - STREET LIGHT ONLY |
| 055 | TRF SGNL | POLE - TRAFFIC SIGNAL AND PED SIGNAL ONLY |
| 056 | SGN BRDG | POLE - SIGN BRIDGE |
| 057 | STOPSIGN | STOP OR YIELD SIGN |
| 058 | OTH SIGN | OTHER SIGN, INCLUDING Street Signs |
| 059 | HYDRANT | HYDRANT |

TREE, STUMP OR SHRUBS
TREE BRANCH OR OTHER VEGETATION OVERHEAD, ETC.
WIRE OR CABLE ACROSS OR OVER THE ROAD
TEMPORARY SIGN OR BARRICADE IN ROAD, ETC.
PERMANENT SIGN OR BARRICADE IN/OFF ROAD
SLIDES, FALLEN OR FALLING ROCKS
FOREIGN OBSTRUCTION/DEBRIS IN ROAD (NOT GRAVEL)
EQUIPMENT WORKING IN/OFF ROAD
OTHER EQUIPMENT IN OR OFF ROAD (INCLUDES PARKED TRAILER, BOAT)
WRECKER, STREET SWEEPER, SNOW PLOW OR SANDING EQUIPMENT
ROCK, BRICK OR OTHER SOLID WALL
OTHER BUMP (NOT SPEED BUMP), POTHOLE OR PAVEMENT IRREGULARITY (PER PAR)
OTHER OVERHEAD OBJECT (HIGHWAY SIGN, SIGNAL HEAD, ETC.); NOT BRIDGE
BRIDGE OR ROAD CAVE IN
HIGH WATER
SNOW BANK
LOW OR HIGH SHOULDER AT PAVEMENT EDGE
SUT SLOPE OR DITCH EMBANKMENT
STRUCK BY ROCK OR OTHER OBJECT SET IN MOTION BY OTHER VEHICLE (INCL. LOST LOADS)
STRUCK BY ROCK OR OTHER OBJECT SET IN MOTION BY OTHER VEHICLE (INCL. LOST LOADS)
STRUCK BY ROCK OR OTHER MOVING OR FLYING OBJECT (NOT SET IN MOTION BY VEHICLE) VEHICLE OBSCURED VIEW
VIEW OBSCURED BY FENCE, SIGN, PHONE BOOTH, ETC.
VEHICLE IMMERSED IN BODY OF WATER
FIRE OR EXPLOSION
FENCE OR BUILDING, ETC.
CRASH RELATED TO ANOTHER SEPARATE CRASH
TWO-WAY TRAFFIC ON DIVIDED ROADWAY ALL ROU
BUILDING OR OTHER STRUCTURE
TWO-WAY TRAFFIC ON DIVIDED ROADWAY ALL ROUTED TO ONE SIDE
BUILDING OR OTHER STRUCTURE
OTHER (PHANTOM) NON-CONTACT VEHICLE
TEENAGE DRIVER IN VIOLATION OF GRADUATED LICENSE PGM
GUY WIRE
BERM (EARTHEN OR GRAVEL MOUND)
GRAVEL IN ROADWAY

CELL PHONE USE WITNESSED BY OTHER PARTICIPANT
FIXED OBJECT, UNKNOWN TYPE.
FIXED OBJECT, UNKNOWN TYPE.
TEXTING
WORK ZONE WORKER PASSENGER RIDING ON VEHICLE EXTERIOR
PASSENGER RIDING ON PEDALCYCLE
PEDESTRIAN IN NON-MOTORIZED WHEELCHAIR
PEDESTRIAN IN MOTORIZED WHEELCHAIR
LAW ENFORCEMENT / POLICE OFFICER
"SUB-BIKE": PEDALCYCLIST INJURED SUBSEQUENT TO COLLISION, ETC.
NON-MOTORIST STRUCK VEHICLE
STREET CAR/TROLLEY (ON RAILS
STREET CAR/TROLLEY (ON RAILS OR OVERHEAD WIRE SYSTEM) STRUCK VEHICLE
VEHICLE STRUCK STREET CAR/TROLLEY (ON RAILS OR OVERHEAD WIRE SYSTEM)
AT OR ON STREET CAR OR TROLLEY RIGHT-OF-WAY
VEHICLE STRUCK RAILROAD EQUIPMENT (NOT TRAIN)
DISTRACTED BY NAVIGATION SYSTEM OR GPS DEVICE
DISTRACTED BY OTHER ELECTRONIC DEVICE
DISTRACTED BY OTHER ELECTRON
RAIL CROSSING DROP-ARM GATE


EVENT CODE TRANSLATION LIST

| CODE | DESCRIPTION | LONG DESCRIPTION |
| :--- | :--- | :--- |
| 118 | EXPNSN JNT | EXPANSION JOINT |
| 119 | JERSEY BAR | JERSEY BARRIER |
| 120 | WIRE BAR | WIRE OR CABLE MEDIAN BARRIER |
| 121 | FENCE | FENCE |
| 123 | OBJ IN VEH | LOOSE OBJECT IN VEHICLE STRUCK OCCUPANT |
| 124 | SLIPPERY | SLIDING OR SWERVING DUE TO WET, ICY, SLIPPERY OR LOOSE SURFACE (NOT GRAVEL) |
| 125 | SHLDR | SHOULDER GAVE WAY |
| 126 | BOULDER | ROCK (S), BOULDER (NOT GRAVEL; NOT ROCK SLIDE) |
| 127 | LAND SLIDE | ROCK SLIDE OR LAND SLIDE |
| 128 | CURVE INV | CURVE PRESENT AT CRASH LOCATION |
| 129 | HILL INV | VERTICAL GRADE / HILL PRESENT AT CRASH LOCATION |
| 130 | CURVE HID | VIEW OBSCURED BY CURVE |
| 131 | HILL HID | VIEW OBSCURED BY VERTICAL GRADE / HILL |
| 132 | WINDOW HID | VIEW OBSCURED BY VEHICLE WINDOW CONDITIONS |
| 133 | SPRAY HID | VIEW OBSCURED BY WATER SPRAY |
| 134 | TORRENTIAL | TORRENTIAL RAIN (EXCEPTIONALLY HEAVY RAIN) |

\[

\]



MILEAGE TYPE CODE TRANSLATION LIST


| MEDIAN TYPE CODE TRANSLATION LIST |  |  |  |
| :---: | :--- | :--- | :---: |
|  | SHORT |  |  |
| CODE | DESC | LONG DESCRIPTION |  |
| 0 | NONE | NO MEDIAN |  |
| 1 | RSDMD | SOLID MEDIAN BARRIER |  |
| 2 | DIVMD | EARTH, GRASS OR PAVED MEDIAN |  |



\[

\]

WEATHER CONDITION CODE TRANSLATION LIST


## APPENDIX E

## TRAFFIC COUNT DATA,

 ADJUSTMENTS AND GROWTH CALCULATIONS
# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> J unction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Sunny, clear
34 degrees F. AM, 60 PM
Collected By: G.Mc.

File Name : FLORENCE 35th St. @ Hwy. 101
Site Code : US 101
Start Date : 3/19/2020
Page No : 1

Groups Printed- Unshifted

|  | HWY 101 From North |  |  |  |  | $35 \mathrm{TH}$ <br> From East |  |  |  |  | HWY 101 From South |  |  |  |  | $\begin{gathered} 35 \mathrm{TH} \\ \text { From West } \end{gathered}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| 07:00 AM | 1 | 37 | 2 | 0 | 40 | 4 | 3 | 5 | 0 | 12 | 5 | 40 | 1 | 0 | 46 | 2 | 0 | 5 | 0 | 7 | 105 |
| 07:15 AM | 1 | 37 | 5 | 0 | 43 | 1 | 2 | 1 | 0 | 4 | 6 | 41 | 1 | 0 | 48 | 4 | 0 | 10 | 0 | 14 | 109 |
| 07:30 AM | 1 | 68 | 9 | 0 | 78 | 1 | 4 | 4 | 0 | 9 | 6 | 43 | 1 | 0 | 50 | 0 | 1 | 10 | 0 | 11 | 148 |
| 07:45 AM | 2 | 74 | 8 | 0 | 84 | 6 | 4 | 2 | 0 | 12 | 5 | 61 | 3 | 0 | 69 | 6 | 1 | 10 | 0 | 17 | 182 |
| Total | 5 | 216 | 24 | 0 | 245 | 12 | 13 | 12 | 0 | 37 | 22 | 185 | 6 | 0 | 213 | 12 | 2 | 35 | 0 | 49 | 544 |
| 08:00 AM | 3 | 56 | 7 | 0 | 66 | 7 | 3 | 1 | 0 | 11 | 3 | 53 | 2 | 0 | 58 | 5 | 0 | 17 | 0 | 22 | 157 |
| 08:15 AM | 3 | 49 | 6 | 0 | 58 | 3 | 6 | 6 | 0 | 15 | 5 | 53 | 1 | 0 | 59 | 11 | 3 | 7 | 0 | 21 | 153 |
| 08:30 AM | 2 | 65 | 8 | 0 | 75 | 2 | 3 | 6 | 0 | 11 | 6 | 52 | 0 | 0 | 58 | 10 | 1 | 8 | 0 | 19 | 163 |
| 08:45 AM | 6 | 71 | 7 | 0 | 84 | 6 | 4 | 7 | 0 | 17 | 11 | 66 | 0 | 0 | 77 | 17 | 4 | 25 | 0 | 46 | 224 |
| Total | 14 | 241 | 28 | 0 | 283 | 18 | 16 | 20 | 0 | 54 | 25 | 224 | 3 | 0 | 252 | 43 | 8 | 57 | 0 | 108 | 697 |

*** BREAK ***

| 04:00 PM | 3 | 106 | 9 | 0 | 118 | 6 | 3 | 5 | 0 | 14 | 15 | 129 | 10 | 0 | 154 | 16 | 5 | 24 | 0 | 45 | 331 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 3 | 95 | 10 | 0 | 108 | 5 | 5 | 7 | 0 | 17 | 14 | 88 | 6 | 0 | 108 | 15 | 5 | 10 | 0 | 30 | 263 |
| 04:30 PM | 9 | 117 | 11 | 0 | 137 | 2 | 1 | 5 | 0 | 8 | 17 | 116 | 3 | 0 | 136 | 15 | 3 | 16 | 0 | 34 | 315 |
| 04:45 PM | 8 | 94 | 11 | 1 | 114 | 10 | 1 | 5 | 0 | 16 | 15 | 98 | 5 | 0 | 118 | 10 | 3 | 19 | 0 | 32 | 280 |
| Total | 23 | 412 | 41 | 1 | 477 | 23 | 10 | 22 | 0 | 55 | 61 | 431 | 24 | 0 | 516 | 56 | 16 | 69 | 0 | 141 | 1189 |
| 05:00 PM | 4 | 114 | 6 | 0 | 124 | 1 | 2 | 9 | 0 | 12 | 22 | 112 | 14 | 0 | 148 | 21 | 6 | 18 | 1 | 46 | 330 |
| 05:15 PM | 4 | 106 | 12 | 0 | 122 | 5 | 6 | 9 | 0 | 20 | 17 | 120 | 5 | 0 | 142 | 9 | 7 | 19 | 0 | 35 | 319 |
| 05:30 PM | 4 | 99 | 5 | 2 | 110 | 3 | 3 | 2 | 0 | 8 | 16 | 98 |  | 2 | 120 | 19 | 6 | 14 | 1 | 40 | 278 |
| 05:45 PM | 4 | 97 | 6 | 0 | 107 | 1 | 2 | 6 | 0 | 9 | 13 | 72 | 1 | 0 | 86 | 12 | 7 | 16 | 0 | 35 | 237 |
| Total | 16 | 416 | 29 | 2 | 463 | 10 | 13 | 26 | 0 | 49 | 68 | 402 | 24 | 2 | 496 | 61 | 26 | 67 | 2 | 156 | 1164 |
| Grand Total | 58 | 1285 | 122 | 3 | 1468 | 63 | 52 | 80 | 0 | 195 | 176 | 1242 | 57 | 2 | 1477 | 172 | 52 | 228 | 2 | 454 | 3594 |
| Apprch \% | 4 | 87.5 | 8.3 | 0.2 |  | 32.3 | 26.7 | 41 | 0 |  | 11.9 | 84.1 | 3.9 | 0.1 |  | 37.9 | 11.5 | 50.2 | 0.4 |  |  |
| Total \% | 1.6 | 35.8 | 3.4 | 0.1 | 40.8 | 1.8 | 1.4 | 2.2 | 0 | 5.4 | 4.9 | 34.6 | 1.6 | 0.1 | 41.1 | 4.8 | 1.4 | 6.3 | 0.1 | 12.6 |  |

# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> Junction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Sunny, clear
34 degrees F. AM, 60 PM
Collected By: G.Mc.

File Name : FLORENCE 35th St. @ Hwy. 101
Site Code : US 101
Start Date : 3/19/2020
Page No : 4

|  | HWY 101 From North |  |  |  |  | 35TH <br> From East |  |  |  |  | HWY 101 From South |  |  |  |  | $\begin{gathered} 35 \mathrm{TH} \\ \text { From West } \end{gathered}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Left | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:30 PM | 9 | 117 | 11 | 0 | 137 | 2 | 1 | 5 | 0 | 8 | 17 | 116 | 3 | 0 | 136 | 15 | 3 | 16 | 0 | 34 | 315 |
| 04:45 PM | 8 | 94 | 11 | 1 | 114 | 10 | 1 | 5 | 0 | 16 | 15 | 98 | 5 | 0 | 118 | 10 | 3 | 19 | 0 | 32 | 280 |
| 05:00 PM | 4 | 114 | 6 | 0 | 124 | 1 | 2 | 9 | 0 | 12 | 22 | 112 | 14 | 0 | 148 | 21 | 6 | 18 | 1 | 46 | 330 |
| 05:15 PM | 4 | 106 | 12 | 0 | 122 | 5 | 6 | 9 | 0 | 20 | 17 | 120 | 5 | 0 | 142 | 9 | 7 | 19 | 0 | 35 | 319 |
| Total Volume | 25 | 431 | 40 | 1 | 497 | 18 | 10 | 28 | 0 | 56 | 71 | 446 | 27 | 0 | 544 | 55 | 19 | 72 | 1 | 147 | 1244 |
| \% App. Total | 5 | 86.7 | 8 | 0.2 |  | 32.1 | 17.9 | 50 | 0 |  | 13.1 | 82 | 5 | 0 |  | 37.4 | 12.9 | 49 | 0.7 |  |  |
| PHF | . 694 | . 921 | . 833 | . 250 | . 907 | 450 | . 417 | . 778 | . 000 | . 700 | . 807 | . 929 | . 482 | . 000 | . 919 | . 655 | . 679 | . 947 | . 250 | . 799 | . 942 |

Peak Hour Analysis From 07:00 AM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:30 PM |  |  |  |  | ${ }^{04.30} \mathrm{PM}$ |  |  |  |  | 04:30 PM |  |  |  |  | 05:00 PM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 9 | 117 | 11 | 0 | 137 | 2 | 1 | 5 | 0 | 8 | 17 | 116 | 3 | 0 | 136 | 21 | 6 | 18 | 1 | 46 |
| +15 mins. | 8 | 94 | 11 | 1 | 114 | 10 | 1 | 5 | 0 | 16 | 15 | 98 | 5 | 0 | 118 | 9 | 7 | 19 | 0 | 35 |
| +30 mins. | 4 | 114 | 6 | 0 | 124 | 1 | 2 | 9 | 0 | 12 | 22 | 112 | 14 | 0 | 148 | 19 | 6 | 14 | 1 | 40 |
| +45 mins. | 4 | 106 | 12 | 0 | 122 | 5 | 6 | 9 | 0 | 20 | 17 | 120 | 5 | 0 | 142 | 12 | 7 | 16 | 0 | 35 |
| Total Volume | 25 | 431 | 40 | 1 | 497 | 18 | 10 | 28 | 0 | 56 | 71 | 446 | 27 | 0 | 544 | 61 | 26 | 67 | 2 | 156 |
| \% App. Total | 5 | 86.7 | 8 | 0.2 |  | 32.1 | 17.9 | 50 | 0 |  | 13.1 | 82 | 5 | 0 |  | 39.1 | 16.7 | 42.9 | 1.3 |  |
| PHF | . 694 | . 921 | 833 | . 250 | . 907 | 450 | . 417 | 778 | 000 | 700 | . 807 | . 929 | 482 | 000 | . 919 | 726 | . 929 | . 882 | . 500 | . 848 |

## Florence, Oregon US 101 @ 35 ${ }^{\text {th }}$ Street <br> Manual Truck Counts

## Thursday 3/19/2020 7:00 to 9:00 AM and 4:00 to 6:00 PM

$\left.\begin{array}{lcccc}\text { Truck } & & & \\ \text { Axles } & \mathbf{7 : 0 0 - 7 : 1 5} & \mathbf{7 : 1 5 - 7 : 3 0} & \mathbf{7 : 3 0 - 7 : 4 5} & \mathbf{7 : 4 5 - 8 : 0 0} \\ \hline 4 & 0 & 2(2-11) & 0 & 0 \\ 5 & 1(1-11) & 0 & 3(1-3,2-11) & 1(1-3) \\ 6 & 0 & 0 & 1(1-14) & 2(2-11) \\ 7 & 0 & 2(1-3,1-11) & 0 & 0 \\ \text { Totals } & 1 & 4 & 4 & 3\end{array}\right] . . .12$

Truck

| Axles | 4:00-4:15 | 4:15-4:30 | 4:30-4:45 | 4:45-5:00 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4 | 0 | 0 | 0 | 1 (1-3) |  |
| 5 | 1 (1-14) | 0 | 0 | 0 |  |
| 6 | 0 | 0 | 0 | 0 |  |
| 7 | 1 (1-11) | 0 | 0 | 0 |  |
| Totals | 2 | 0 | 0 | 1 | ... 3 |
| Truck |  |  |  |  |  |
| Axles | 5:00-5:15 | 5:15-5:30 | 5:30-5:45 | 5:45-6:00 |  |
| 4 | 0 | 0 | 0 | 0 |  |
| 5 | 0 | 1 (1-10) | 0 | 0 |  |
| 6 | 0 | 0 | 2 (2-11) | 0 |  |
| Totals | 0 | 1 | 2 | 0 | ... 3 |
|  |  |  | Total of 6 trucks in $\mathbf{2}$ hrs. |  |  |

## Grand Total of 16 heavy trucks during four peak hours

## LEGEND of MOVEMENTS

2 = Southbound on 101, turning right onto $35^{\text {th }}$ St.
3 = Southbound Through on 101
$10=$ Northbound on 101, turning right onto $35^{\text {th }}$ St.
11 = Northbound Through on 101
$14=$ Eastbound on $35^{\text {th }}$, turning right onto 101
G.Mc. 3/20/2020

## Florence, Oregon

## Hwy. 101 @ 35 ${ }^{\text {th }}$ Street

Manual Bicycle Counts

| Thursday 3/19/2020 7:00 to 9:00 AM, 4:00 to 6:00 PM |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 7:00-7:15 | 7:15-7:30 |  |  |

Grand Total of 8 bicycles observed during two AM and two PM peak hours riding on both sides of Hwy. 101 or $35^{\text {th }}$ St. and executing noted movements at the intersection.

## LEGEND of MOVEMENTS

3 = Southbound Through on 101
7 = Westbound Through on 35 th
$15=$ Eastbound Through on $35^{\text {th }}$
$16=$ Eastbound on $35^{\text {th }}$, turning left onto 101

# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> J unction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Clear, sunny
34 degrees F. AM, 60 PM
Collected By: MH

File Name : FLRNC 35th @ Redwood
Site Code : Florence
Start Date : 3/19/2020
Page No : 1

|  | 35TH ST <br> From East |  |  |  | REDWOOD LOOP <br> From South |  |  |  | 35TH ST From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | App. Total | Left | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Int. Total |
| 07:00 AM | 0 | 8 | 0 | 8 | 5 | 0 | 0 | 5 | 0 | , | 0 | 1 | 14 |
| 07:15 AM | 2 | 1 | 0 | 3 | 2 | 0 | 0 | 2 | 1 | 1 | 0 | 2 | 7 |
| 07:30 AM | 0 | 6 | 0 | 6 | 3 | 1 | 0 | 4 | 2 | 1 | 0 | 3 | 13 |
| 07:45 AM | 3 | 8 | 0 | 11 | 4 | 0 | 0 | 4 | 3 | 3 | 0 | 6 | 21 |
| Total | 5 | 23 | 0 | 28 | 14 | 1 | 0 | 15 | 6 | 6 | 0 | 12 | 55 |
| 08:00 AM | 1 | 7 | 0 | 8 | 5 | 0 | 0 | 5 | 4 | 1 | 0 | 5 | 18 |
| 08:15 AM | 4 | 8 | 2 | 14 | 7 | 1 | 0 | 8 | 0 | 7 | 0 | 7 | 29 |
| 08:30 AM | 1 | 6 | 0 | 7 | 5 | 4 | 0 | 9 | 3 | 0 | 0 | 3 | 19 |
| 08:45 AM | 4 | 10 | 0 | 14 | 7 | 1 | 0 | 8 | 5 | 4 | 0 | 9 | 31 |
| Total | 10 | 31 | 2 | 43 | 24 | 6 | 0 | 30 | 12 | 12 | 0 | 24 | 97 |

## *** BREAK ***

| 04:00 PM | 0 | 7 | 0 | 7 | 10 | 2 | 0 | 12 | 11 | 4 | 0 | 15 | 34 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 5 | 9 | 0 | 14 | 7 | 5 | 0 | 12 | 10 | 6 | 0 | 16 | 42 |
| 04:30 PM | 0 | 6 | 0 | 6 | 4 | 2 | 0 | 6 | 6 | 12 | 0 | 18 | 30 |
| 04:45 PM | 8 | 9 | 0 | 17 | 5 | 2 | 2 | 9 | 7 | 7 | 0 | 14 | 40 |
| Total | 13 | 31 | 0 | 44 | 26 | 11 | 2 | 39 | 34 | 29 | 0 | 63 | 146 |
| 05:00 PM | 6 | 6 | 0 | 12 | 7 | 4 | 0 | 11 | 16 | 10 | 0 | 26 | 49 |
| 05:15 PM | 5 | 8 | 0 | 13 | 10 | 3 | 0 | 13 | 9 | 8 | 0 | 17 | 43 |
| 05:30 PM | 1 | 2 | 0 | 3 | 4 | 3 | 1 | 8 | 6 | 9 | 0 | 15 | 26 |
| 05:45 PM | 2 | 2 | 0 | 4 | 7 | 1 | 0 | 8 | 5 | 6 | 0 | 11 | 23 |
| Total | 14 | 18 | 0 | 32 | 28 | 11 | 1 | 40 | 36 | 33 | 0 | 69 | 141 |
| Grand Total | 42 | 103 | 2 | 147 | 92 | 29 | 3 | 124 | 88 | 80 | 0 | 168 | 439 |
| Apprch \% | 28.6 | 70.1 | 1.4 |  | 74.2 | 23.4 | 2.4 |  | 52.4 | 47.6 | 0 |  |  |
| Total \% | 9.6 | 23.5 | 0.5 | 33.5 | 21 | 6.6 | 0.7 | 28.2 | 20 | 18.2 | 0 | 38.3 |  |

# Gary's Traffic Data <br> 310 Pitney Lane, Unit 39 <br> J unction City, OR 97448 <br> Fast, Accurate, High Quality Counts 

Weather: Clear, sunny
34 degrees F. AM, 60 PM
Collected By: MH

File Name : FLRNC 35th @ Redwood
Site Code : Florence
Start Date : 3/19/2020
Page No : 4

|  | 35TH ST <br> From East |  |  |  | REDWOOD LOOP From South |  |  |  | 35TH ST <br> From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Peds | App. Total | Left | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 05:45 PM - Peak 1 of 1 <br> Peak Hour for Entire Intersection Begins at 04:30 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:30 PM | 0 | 6 | 0 | 6 | 4 | 2 | 0 | 6 | 6 | 12 | 0 | 18 | 30 |
| 04:45 PM | 8 | 9 | 0 | 17 | 5 | 2 | 2 | 9 | 7 | 7 | 0 | 14 | 40 |
| 05:00 PM | 6 | 6 | 0 | 12 | 7 | 4 | 0 | 11 | 16 | 10 | 0 | 26 | 49 |
| 05:15 PM | 5 | 8 | 0 | 13 | 10 | 3 | 0 | 13 | 9 | 8 | 0 | 17 | 43 |
| Total Volume | 19 | 29 | 0 | 48 | 26 | 11 | 2 | 39 | 38 | 37 | 0 | 75 | 162 |
| \% App. Total | 39.6 | 60.4 | 0 |  | 66.7 | 28.2 | 5.1 |  | 50.7 | 49.3 | 0 |  |  |
| PHF | . 594 | . 806 | . 000 | . 706 | . 650 | . 688 | . 250 | . 750 | . 594 | . 771 | . 000 | . 721 | . 827 |

Peak Hour Analysis From 07:00 AM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:15 PM |  |  |  | 04:45 PM |  |  |  | 04:30 PM |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 5 | 9 | 0 | 14 | 5 | 2 | 2 | 9 | 6 | 12 | 0 | 18 |
| +15 mins. | 0 | 6 | 0 | 6 | 7 | 4 | 0 | 11 | 7 | 7 | 0 | 14 |
| +30 mins. | 8 | 9 | 0 | 17 | 10 | 3 | 0 | 13 | 16 | 10 | 0 | 26 |
| +45 mins. | 6 | 6 | 0 | 12 | 4 | 3 | 1 | 8 | 9 | 8 | 0 | 17 |
| Total Volume | 19 | 30 | 0 | 49 | 26 | 12 | 3 | 41 | 38 | 37 | 0 | 75 |
| \% App. Total | 38.8 | 61.2 | 0 |  | 63.4 | 29.3 | 7.3 |  | 50.7 | 49.3 | 0 |  |
| PHF | . 594 | . 833 | . 000 | . 721 | . 650 | . 750 | . 375 | . 788 | . 594 | . 771 | . 000 | . 721 |




ODOT FVT Values:
ODOT FVT Values:

| Site ID | HWY | DIR | Description | $\mathbf{2 0 1 8}$ | $\mathbf{2 0 3 8}$ | RSQ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1170 | 9 | 1 | 0.03 Mile South Munsel Lake Rd | 9500 | 9600 | 0.2347 |
| 1171 | 9 | 1 | 0.02 Mile South of 36 th Street | 12500 | 12600 | 0.4298 |
| 1172 | 9 | 1 | 0.02 Mile South of 29 th Street | 14100 | 14200 | 0.805 |

> EUGENE-SPRINGFIELD SALEM-KEIZER



## APPENDIX F

## ITE TRIP GENERATION

# Land Use: 934 <br> Fast-Food Restaurant with Drive-Through Window 

## Description

This category includes fast-food restaurants with drive-through windows. This type of restaurant is characterized by a large drive-through clientele, long hours of service (some are open for breakfast, all are open for lunch and dinner, some are open late at night or 24 hours a day) and high turnover rates for eat-in customers. These limited-service eating establishments do not provide table service. Non-drive-through patrons generally order at a cash register and pay before they eat. Fast casual restaurant (Land Use 930), high-turnover (sit-down) restaurant (Land Use 932), fast-food restaurant without drive-through window (Land Use 933), and fast-food restaurant with drive-through window and no indoor seating (Land Use 935) are related uses.

## Additional Data

Users should exercise caution when applying statistics during the AM peak periods, as the sites contained in the database for this land use may or may not be open for breakfast. In cases where it was confirmed that the sites were not open for breakfast, data for the AM peak hour of the adjacent street traffic were removed from the database.

The outdoor seating area is not included in the overall gross floor area. Therefore, the number of seats may be a more reliable independent variable on which to establish trip generation rates for facilities having significant outdoor seating.

Time-of-day distribution data for this land use for a weekday, Saturday, and Sunday are presented in Appendix A. For the 46 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 11:45 a.m. and 12:45 p.m. and 12:00 and 1:00 p.m., respectively. For the one dense multi-use urban site with data, the same AM and PM peak hours were observed.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alaska, Alberta (CAN), California, Colorado, Florida, Indiana, Kentucky, Maryland, Massachusetts, Minnesota, Montana, New Hampshire, New Jersey, New York, North Carolina, Ohio, Pennsylvania, South Dakota, Texas, Vermont, Virginia, Washington, and Wisconsin.

## Source Numbers

163, 164, 168, 180, 181, 241, 245, 278, 294, 300, 301, 319, 338, 340, 342, 358, 389, 438, 502, 552,
$577,583,584,617,640,641,704,715,728,810,866,867,869,885,886,927,935,962,977$

## Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.
Setting/Location: General Urban/Suburban
Number of Studies: 111
Avg. 1000 Sq. Ft. GFA: 4
Directional Distribution: 51\% entering, 49\% exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 40.19 | $0.38-164.25$ | 28.78 |

## Data Plot and Equation



## Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
Number of Studies: 185
Avg. 1000 Sq. Ft. GFA: 3
Directional Distribution: 52\% entering, 48\% exiting
Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 32.67 | $8.17-117.22$ | 17.87 |

## Data Plot and Equation



## Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

| Setting/Location: | General Urban/Suburban |
| ---: | :--- |
| Number of Studies: | 67 |
| Avg. 1000 Sq. Ft. GFA: | 3 |
| Directional Distribution: | $50 \%$ entering, $50 \%$ exiting |

Vehicle Trip Generation per 1000 Sq. Ft. GFA

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 470.95 | $98.89-1137.66$ | 244.44 |

## Data Plot and Equation



## APPENDIX G

## Existing and No-Build SYNCHRO Performance Calculations

|  | 4 | $\rightarrow$ | \% |  |  |  | 4 | 4 | \% | ( | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | * ${ }^{\text {a }}$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  |
| Traffic Volume (vph) | 66 | 12 | 88 | 28 | 26 | 31 | 39 | 346 | 5 | 22 | 372 | 43 |
| Future Volume (vph) | 66 | 12 | 88 | 28 | 26 | 31 | 39 | 346 | 5 | 22 | 372 | 43 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.868 |  |  | 0.918 |  |  | 0.998 |  |  | 0.984 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1506 | 0 | 1662 | 1606 | 0 | 1662 | 3223 | 0 | 1662 | 3208 | 0 |
| Flt Permitted | 0.709 |  |  | 0.675 |  |  | 0.424 |  |  | 0.492 |  |  |
| Satd. Flow (perm) | 1241 | 1506 | 0 | 1181 | 1606 | 0 | 742 | 3223 | 0 | 861 | 3208 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 113 |  |  | 40 |  |  | 2 |  |  | 16 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Adj. Flow (vph) | 85 | 15 | 113 | 36 | 33 | 40 | 50 | 444 | 6 | 28 | 477 | 55 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 85 | 128 | 0 | 36 | 73 | 0 | 50 | 450 | 0 | 28 | 532 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


## 2020 AM DESIGN HOUR CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/12/2020


|  | 4 | $\rightarrow$ |  | 7 | - | 4 |  | $\dagger$ |  | $\downarrow$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ${ }_{\$}$ |  |  | ${ }_{\$}$ |  |  | \$ |  |
| Traffic Volume (vph) | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.934 |  |  |  |  |  | 0.973 |  |  |  |  |
| Flt Protected |  |  |  |  | 0.988 |  |  | 0.962 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1634 | 0 | 0 | 1729 | 0 | 0 | 1638 | 0 | 0 | 1750 | 0 |
| Flt Permitted |  |  |  |  | 0.988 |  |  | 0.962 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1634 | 0 | 0 | 1729 | 0 | 0 | 1638 | 0 | 0 | 1750 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 493 |  |  | 457 |  |  | 274 |  |
| Travel Time (s) |  | 8.6 |  |  | 13.4 |  |  | 12.5 |  |  | 6.2 |  |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Adj. Flow (vph) | 0 | 26 | 25 | 19 | 62 | 0 | 48 | 0 | 12 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 51 | 0 | 0 | 81 | 0 | 0 | 60 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(tt) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 20.3\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street



|  | 7 | 4 | $\dagger$ | $>$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 中t |  |  | 个 $\uparrow$ |
| Traffic Volume（vph） | 0 | 0 | 443 | 0 | 0 | 437 |
| Future Volume（vph） | 0 | 0 | 443 | 0 | 0 | 437 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util．Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  |  |  |  |  |  |
| FIt Protected |  |  |  |  |  |  |
| Satd．Flow（prot） | 0 | 1750 | 3260 | 0 | 0 | 3260 |
| Flt Permitted |  |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 1750 | 3260 | 0 | 0 | 3260 |
| Link Speed（mph） | 30 |  | 40 |  |  | 40 |
| Link Distance（ t ） | 243 |  | 716 |  |  | 923 |
| Travel Time（s） | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Heavy Vehicles（\％） | 0\％ | 0\％ | 2\％ | 0\％ | 0\％ | 2\％ |
| Adj．Flow（vph） | 0 | 0 | 540 | 0 | 0 | 533 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 0 | 540 | 0 | 0 | 533 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width（ft） | 0 |  | 12 |  |  | 12 |
| Link Offset（ft） | 0 |  | 0 |  |  | 0 |
| Crosswalk Width（ft） | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type：Other |  |  |  |  |  |  |
| Control Type：Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 16．6\％ICU Level of Service A |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |




|  | $\rangle$ | $\rightarrow$ |  | 7 |  | 4 | 4 | 4 | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | $\uparrow{ }^{\text {t }}$ |  |
| Traffic Volume (vph) | 85 | 31 | 111 | 28 | 15 | 43 | 110 | 689 | 44 | 41 | 666 | 62 |
| Future Volume (vph) | 85 | 31 | 111 | 28 | 15 | 43 | 110 | 689 | 44 | 41 | 666 | 62 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.883 |  |  | 0.889 |  |  | 0.991 |  |  | 0.987 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1533 | 0 | 1662 | 1556 | 0 | 1662 | 3291 | 0 | 1662 | 3282 | 0 |
| Flt Permitted | 0.717 |  |  | 0.661 |  |  | 0.253 |  |  | 0.356 |  |  |
| Satd. Flow (perm) | 1255 | 1533 | 0 | 1157 | 1556 | 0 | 443 | 3291 | 0 | 623 | 3282 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 118 |  |  | 46 |  |  | 10 |  |  | 13 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance ( ft ) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 0\% | 2\% | 0\% | 0\% | 0\% |
| Adj. Flow (vph) | 90 | 33 | 118 | 30 | 16 | 46 | 117 | 733 | 47 | 44 | 709 | 66 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 90 | 151 | 0 | 30 | 62 | 0 | 117 | 780 | 0 | 44 | 775 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(t) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector ( t ) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (tt) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |


|  | 4 |  |  |  |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 8 | 8 |  | 4 | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| Minimum Split (s) | 22.0 | 22.0 |  | 28.0 | 28.0 |  | 13.0 | 28.8 |  | 13.0 | 22.8 |  |
| Total Split (s) | 29.0 | 29.0 |  | 29.0 | 29.0 |  | 18.0 | 47.0 |  | 14.0 | 43.0 |  |
| Total Split (\%) | 32.2\% | 32.2\% |  | 32.2\% | 32.2\% |  | 20.0\% | 52.2\% |  | 15.6\% | 47.8\% |  |
| Maximum Green (s) | 25.0 | 25.0 |  | 25.0 | 25.0 |  | 13.2 | 42.2 |  | 9.2 | 38.2 |  |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 |  | 4.3 | 4.3 |  | 4.3 | 4.3 |  |
| All-Red Time (s) | 0.5 | 0.5 |  | 0.5 | 0.5 |  | 0.5 | 0.5 |  | 0.5 | 0.5 |  |
| Lost Time Adjust (s) | 0.5 | 0.5 |  | 0.5 | 0.5 |  | 0.5 | 0.5 |  | 0.5 | 0.5 |  |
| Total Lost Time (s) | 4.5 | 4.5 |  | 4.5 | 4.5 |  | 5.3 | 5.3 |  | 5.3 | 5.3 |  |
| Lead/Lag |  |  |  |  |  |  | Lead | Lag |  | Lead | Lag |  |
| Lead-Lag Optimize? |  |  |  |  |  |  | Yes | Yes |  | Yes | Yes |  |
| Vehicle Extension (s) | 2.5 | 2.5 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 8.0 |  | 8.0 | 8.0 |  | 8.0 | 8.0 |  | 8.0 | 8.0 |  |
| Time To Reduce (s) | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| Recall Mode | None | None |  | None | None |  | None | Min |  | None | Min |  |
| Walk Time (s) | 7.0 | 7.0 |  | 7.0 | 7.0 |  |  | 7.0 |  |  | 7.0 |  |
| Flash Dont Walk (s) | 11.0 | 11.0 |  | 17.0 | 17.0 |  |  | 17.0 |  |  | 11.0 |  |
| Pedestrian Calls (\#/hr) | 1 | 1 |  | 0 | 0 |  |  | 0 |  |  | 1 |  |
| Act Effct Green (s) | 8.8 | 8.8 |  | 8.8 | 8.8 |  | 24.2 | 21.7 |  | 20.6 | 16.3 |  |
| Actuated g/C Ratio | 0.20 | 0.20 |  | 0.20 | 0.20 |  | 0.54 | 0.49 |  | 0.46 | 0.37 |  |
| v/c Ratio | 0.37 | 0.38 |  | 0.13 | 0.18 |  | 0.29 | 0.48 |  | 0.11 | 0.64 |  |
| Control Delay | 21.0 | 9.2 |  | 17.4 | 9.3 |  | 7.1 | 11.2 |  | 6.0 | 16.3 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 21.0 | 9.2 |  | 17.4 | 9.3 |  | 7.1 | 11.2 |  | 6.0 | 16.3 |  |
| LOS | C | A |  | B | A |  | A | B |  | A | B |  |
| Approach Delay |  | 13.6 |  |  | 12.0 |  |  | 10.6 |  |  | 15.7 |  |
| Approach LOS |  | B |  |  | B |  |  | B |  |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 44.5 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 70 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.64 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 13.1 |  |  |  | Intersection LOS: B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 57.6\% |  |  |  | ICU Level of Service B |  |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases: 1: HWY 101 \& 35th Street


## 2020 PM DESIGN HOUR CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/15/2020


|  | $\rangle$ | $\rightarrow$ |  | $\dagger$ | - | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | $\$$ |  |  | \$ |  |  | \$ |  |
| Traffic Volume (vph) | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.933 |  |  |  |  |  | 0.959 |  |  |  |  |
| Flt Protected |  |  |  |  | 0.981 |  |  | 0.966 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1633 | 0 | 0 | 1717 | 0 | 0 | 1621 | 0 | 0 | 1750 | 0 |
| Flt Permitted |  |  |  |  | 0.981 |  |  | 0.966 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1633 | 0 | 0 | 1717 | 0 | 0 | 1621 | 0 | 0 | 1750 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 225 |  |  | 457 |  |  | 251 |  |
| Travel Time (s) |  | 8.6 |  |  | 6.1 |  |  | 12.5 |  |  | 5.7 |  |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Adj. Flow (vph) | 0 | 72 | 70 | 35 | 56 | 0 | 49 | 0 | 21 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 142 | 0 | 0 | 91 | 0 | 0 | 70 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(tt) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 21.2\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.2 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | * |  |  | * |  |  | * |  |
| Traffic Vol, veh/h | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 72 | 70 | 35 | 56 | 0 | 49 | 0 | 21 | 0 | 0 | 0 |



## 2020 PM DESIGN HOUR CONDITIONS

Lanes, Volumes, Timings
3: HWY 101 \& RIRO SITE ACCESS


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{r}$ | 个 |  |  |  |
| Traffic Vol, veh/h | 0 | 0 | 817 | 0 | 0 | 769 |
| Future Vol, veh/h | 0 | 0 | 817 | 0 | 0 | 769 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 0 | 869 | 0 | 0 | 818 |



## 2021 AM DESIGN HOUR NO- BUILD CONDITIONS

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | 中 ${ }^{\text {P }}$ |  |
| Traffic Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 346 | 5 | 22 | 372 | 45 |
| Future Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 346 | 5 | 22 | 372 | 45 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.865 |  |  | 0.918 |  |  | 0.998 |  |  | 0.984 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1500 | 0 | 1662 | 1606 | 0 | 1662 | 3223 | 0 | 1662 | 3208 | 0 |
| Flt Permitted | 0.709 |  |  | 0.664 |  |  | 0.419 |  |  | 0.492 |  |  |
| Satd. Flow (perm) | 1241 | 1500 | 0 | 1162 | 1606 | 0 | 733 | 3223 | 0 | 861 | 3208 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 131 |  |  | 40 |  |  | 2 |  |  | 17 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Adj. Flow (vph) | 94 | 15 | 131 | 36 | 33 | 40 | 55 | 444 | 6 | 28 | 477 | 58 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 94 | 146 | 0 | 36 | 73 | 0 | 55 | 450 | 0 | 28 | 535 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2021 AM DESIGN HOUR NO- BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/15/2020

|  | 4 | $\rightarrow$ | $\cdots$ | $\checkmark$ |  | 4 | 4 | 4 |  |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  | ${ }^{*}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{*}$ | 中 ${ }^{\text {a }}$ |  |
| Traffic Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 346 | 5 | 22 | 372 | 45 |
| Future Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 346 | 5 | 22 | 372 | 45 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time (s) | 4.5 | 4.5 |  | 4.5 | 4.5 |  | 5.3 | 5.3 |  | 5.3 | 5.3 |  |
| Lane Util. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 0.95 |  | 1.00 | 0.95 |  |
| Frt | 1.00 | 0.87 |  | 1.00 | 0.92 |  | 1.00 | 1.00 |  | 1.00 | 0.98 |  |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1662 | 1501 |  | 1662 | 1606 |  | 1662 | 3223 |  | 1662 | 3207 |  |
| Flt Permitted | 0.71 | 1.00 |  | 0.66 | 1.00 |  | 0.42 | 1.00 |  | 0.49 | 1.00 |  |
| Satd. Flow (perm) | 1242 | 1501 |  | 1162 | 1606 |  | 734 | 3223 |  | 861 | 3207 |  |
| Peak-hour factor, PHF | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 94 | 15 | 131 | 36 | 33 | 40 | 55 | 444 | 6 | 28 | 477 | 58 |
| RTOR Reduction (vph) | 0 | 107 | 0 | 0 | 33 | 0 | 0 | 1 | 0 | 0 | 11 | 0 |
| Lane Group Flow (vph) | 94 | 39 | 0 | 36 | 40 | 0 | 55 | 449 | 0 | 28 | 524 | 0 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |
| Actuated Green, G (s) | 6.9 | 6.9 |  | 6.9 | 6.9 |  | 15.9 | 14.0 |  | 13.9 | 13.0 |  |
| Effective Green, g (s) | 6.4 | 6.4 |  | 6.4 | 6.4 |  | 14.9 | 13.5 |  | 12.9 | 12.5 |  |
| Actuated g/C Ratio | 0.18 | 0.18 |  | 0.18 | 0.18 |  | 0.42 | 0.38 |  | 0.36 | 0.35 |  |
| Clearance Time (s) | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.8 | 4.8 |  | 4.8 | 4.8 |  |
| Vehicle Extension (s) | 2.5 | 2.5 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Lane Grp Cap (vph) | 224 | 271 |  | 210 | 290 |  | 345 | 1229 |  | 322 | 1132 |  |
| v/s Ratio Prot |  | 0.03 |  |  | 0.03 |  | c0.01 | 0.14 |  | 0.00 | c0.16 |  |
| v/s Ratio Perm | c0.08 |  |  | 0.03 |  |  | 0.06 |  |  | 0.03 |  |  |
| v/c Ratio | 0.42 | 0.14 |  | 0.17 | 0.14 |  | 0.16 | 0.37 |  | 0.09 | 0.46 |  |
| Uniform Delay, d1 | 12.9 | 12.2 |  | 12.3 | 12.2 |  | 6.2 | 7.9 |  | 7.3 | 8.9 |  |
| Progression Factor | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 0.9 | 0.2 |  | 0.3 | 0.2 |  | 0.2 | 0.1 |  | 0.1 | 0.2 |  |
| Delay (s) | 13.8 | 12.4 |  | 12.5 | 12.3 |  | 6.3 | 8.0 |  | 7.4 | 9.1 |  |
| Level of Service | B | B |  | B | B |  | A | A |  | A | A |  |
| Approach Delay (s) |  | 12.9 |  |  | 12.4 |  |  | 7.8 |  |  | 9.0 |  |
| Approach LOS |  | B |  |  | B |  |  | A |  |  | A |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 9.5 |  | HCM 2000 | evel of | Service |  | A |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.43 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 35.4 |  | Sum of los | time (s) |  |  | 15.1 |  |  |  |
| Intersection Capacity Utilization |  |  | 39.7\% |  | ICU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |
| c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |


|  | $\rangle$ | $\rightarrow$ |  | $\dagger$ | - | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\$$ |  |  | \$ |  |  | \$ |  |
| Traffic Volume (vph) | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.934 |  |  |  |  |  | 0.973 |  |  |  |  |
| Flt Protected |  |  |  |  | 0.988 |  |  | 0.962 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1634 | 0 | 0 | 1729 | 0 | 0 | 1638 | 0 | 0 | 1750 | 0 |
| Flt Permitted |  |  |  |  | 0.988 |  |  | 0.962 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1634 | 0 | 0 | 1729 | 0 | 0 | 1638 | 0 | 0 | 1750 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 493 |  |  | 457 |  |  | 274 |  |
| Travel Time (s) |  | 8.6 |  |  | 13.4 |  |  | 12.5 |  |  | 6.2 |  |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Adj. Flow (vph) | 0 | 26 | 25 | 19 | 62 | 0 | 48 | 0 | 12 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 51 | 0 | 0 | 81 | 0 | 0 | 60 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(tt) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 20.3\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.7 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  |  | ¢ |  |  | ¢ |  |  |
| Traffic Vol, veh/h | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |  |
| Future Vol, veh/h | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |  |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control F | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - |  | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 |  |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Mvmt Flow | 0 | 26 | 25 | 19 | 62 | 0 | 48 | 0 | 12 | 0 | 0 | 0 |  |



## 2021 AM DESIGN HOUR NO- BUILD CONDITIONS

Lanes, Volumes, Timings
3: HWY 101 \& RIRO SITE ACCESS

|  |  |  |  |  |  | $\frac{1}{1}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 中 ${ }^{\text {a }}$ |  |  | 44 |
| Traffic Volume (vph) | 0 | 0 | 450 | 0 | 0 | 439 |
| Future Volume (vph) | 0 | 0 | 450 | 0 | 0 | 439 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  |  |  |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd. Flow (prot) | 0 | 1750 | 3260 | 0 | 0 | 3260 |
| Flt Permitted |  |  |  |  |  |  |
| Satd. Flow (perm) | 0 | 1750 | 3260 | 0 | 0 | 3260 |
| Link Speed (mph) | 30 |  | 40 |  |  | 40 |
| Link Distance (ft) | 243 |  | 716 |  |  | 923 |
| Travel Time (s) | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Heavy Vehicles (\%) | 0\% | 0\% | 2\% | 0\% | 0\% | 2\% |
| Adj. Flow (vph) | 0 | 0 | 549 | 0 | 0 | 535 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 549 | 0 | 0 | 535 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 0 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 16.8\% <br> ICU Level of Service A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

## 2021 AM DESIGN HOUR NO- BUILD CONDITIONS

HCM 2010 TWSC
3: HWY 101 \& RIRO SITE ACCESS



## 2021 PM DESIGN HOUR NO- BUILD CONDITIONS

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 中 ${ }^{\text {c }}$ |  |
| Traffic Volume (vph) | 89 | 31 | 120 | 28 | 15 | 43 | 126 | 689 | 44 | 41 | 666 | 68 |
| Future Volume (vph) | 89 | 31 | 120 | 28 | 15 | 43 | 126 | 689 | 44 | 41 | 666 | 68 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.881 |  |  | 0.889 |  |  | 0.991 |  |  | 0.986 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1530 | 0 | 1662 | 1556 | 0 | 1662 | 3291 | 0 | 1662 | 3278 | 0 |
| Flt Permitted | 0.717 |  |  | 0.655 |  |  | 0.249 |  |  | 0.356 |  |  |
| Satd. Flow (perm) | 1255 | 1530 | 0 | 1146 | 1556 | 0 | 436 | 3291 | 0 | 623 | 3278 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 128 |  |  | 46 |  |  | 10 |  |  | 15 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 0\% | 2\% | 0\% | 0\% | 0\% |
| Adj. Flow (vph) | 95 | 33 | 128 | 30 | 16 | 46 | 134 | 733 | 47 | 44 | 709 | 72 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 95 | 161 | 0 | 30 | 62 | 0 | 134 | 780 | 0 | 44 | 781 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2021 PM DESIGN HOUR NO- BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street

|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

Splits and Phases: 1: HWY 101 \& 35th Street


## 2021 PM DESIGN HOUR NO- BUILD CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/15/2020


|  | $\stackrel{ }{*}$ |  |  | 1 |  |  |  | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | ¢ |  |  | * |  |
| Traffic Volume (vph) | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.933 |  |  |  |  |  | 0.959 |  |  |  |  |
| Flt Protected |  |  |  |  | 0.981 |  |  | 0.966 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1633 | 0 | 0 | 1717 | 0 | 0 | 1621 | 0 | 0 | 1750 | 0 |
| Flt Permitted |  |  |  |  | 0.981 |  |  | 0.966 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1633 | 0 | 0 | 1717 | 0 | 0 | 1621 | 0 | 0 | 1750 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (t) |  | 317 |  |  | 225 |  |  | 457 |  |  | 251 |  |
| Travel Time (s) |  | 8.6 |  |  | 6.1 |  |  | 12.5 |  |  | 5.7 |  |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Adj. Flow (vph) | 0 | 72 | 70 | 35 | 56 | 0 | 49 | 0 | 21 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 142 | 0 | 0 | 91 | 0 | 0 | 70 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 21.2\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street



|  | 7 | 4 | 9 |  |  | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 4\% |  |  | 44 |
| Traffic Volume (vph) | 0 | 0 | 821 | 0 | 0 | 775 |
| Future Volume (vph) | 0 | 0 | 821 | 0 | 0 | 775 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  |  |  |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd. Flow (prot) | 0 | 1750 | 3325 | 0 | 0 | 3325 |
| Flt Permitted |  |  |  |  |  |  |
| Satd. Flow (perm) | 0 | 1750 | 3325 | 0 | 0 | 3325 |
| Link Speed (mph) | 30 |  | 40 |  |  | 40 |
| Link Distance (ft) | 243 |  | 716 |  |  | 923 |
| Travel Time (s) | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adj. Flow (vph) | 0 | 0 | 873 | 0 | 0 | 824 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 873 | 0 | 0 | 824 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 0 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 28.0\% |  |  |  |  | Level | Service A |
| Analysis Period (min) 15 |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement V | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 7 | 㻢 |  |  | 中4 |
| Traffic Vol, veh/h | 0 | 0 | 821 | 0 | 0 | 775 |
| Future Vol, veh/h | 0 | 0 | 821 | 0 | 0 | 775 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 0 | 873 | 0 | 0 | 824 |



## 2026 AM DESIGN HOUR NO- BUILD CONDITIONS

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{*}$ | t |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 性 |  |
| Traffic Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 347 | 5 | 22 | 373 | 45 |
| Future Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 347 | 5 | 22 | 373 | 45 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.865 |  |  | 0.918 |  |  | 0.998 |  |  | 0.984 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1500 | 0 | 1662 | 1606 | 0 | 1662 | 3223 | 0 | 1662 | 3208 | 0 |
| Flt Permitted | 0.709 |  |  | 0.664 |  |  | 0.419 |  |  | 0.491 |  |  |
| Satd. Flow (perm) | 1241 | 1500 | 0 | 1162 | 1606 | 0 | 733 | 3223 | 0 | 859 | 3208 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 131 |  |  | 40 |  |  | 2 |  |  | 17 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Adj. Flow (vph) | 94 | 15 | 131 | 36 | 33 | 40 | 55 | 445 | 6 | 28 | 478 | 58 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 94 | 146 | 0 | 36 | 73 | 0 | 55 | 451 | 0 | 28 | 536 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ | CI+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2026 AM DESIGN HOUR NO- BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/15/2020

|  | 4 | $\rightarrow$ | $\cdots$ | $\checkmark$ |  | 4 | 4 | 4 |  |  | $\dagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  | ${ }^{*}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{*}$ | 中 ${ }^{\text {a }}$ |  |
| Traffic Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 347 | 5 | 22 | 373 | 45 |
| Future Volume (vph) | 73 | 12 | 102 | 28 | 26 | 31 | 43 | 347 | 5 | 22 | 373 | 45 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time (s) | 4.5 | 4.5 |  | 4.5 | 4.5 |  | 5.3 | 5.3 |  | 5.3 | 5.3 |  |
| Lane Util. Factor | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 0.95 |  | 1.00 | 0.95 |  |
| Frt | 1.00 | 0.87 |  | 1.00 | 0.92 |  | 1.00 | 1.00 |  | 1.00 | 0.98 |  |
| Flt Protected | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 |  | 0.95 | 1.00 |  |
| Satd. Flow (prot) | 1662 | 1501 |  | 1662 | 1606 |  | 1662 | 3223 |  | 1662 | 3207 |  |
| Flt Permitted | 0.71 | 1.00 |  | 0.66 | 1.00 |  | 0.42 | 1.00 |  | 0.49 | 1.00 |  |
| Satd. Flow (perm) | 1242 | 1501 |  | 1162 | 1606 |  | 733 | 3223 |  | 860 | 3207 |  |
| Peak-hour factor, PHF | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 94 | 15 | 131 | 36 | 33 | 40 | 55 | 445 | 6 | 28 | 478 | 58 |
| RTOR Reduction (vph) | 0 | 107 | 0 | 0 | 33 | 0 | 0 | 1 | 0 | 0 | 11 | 0 |
| Lane Group Flow (vph) | 94 | 39 | 0 | 36 | 40 | 0 | 55 | 450 | 0 | 28 | 525 | 0 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |
| Actuated Green, G (s) | 6.9 | 6.9 |  | 6.9 | 6.9 |  | 15.9 | 14.0 |  | 13.9 | 13.0 |  |
| Effective Green, g (s) | 6.4 | 6.4 |  | 6.4 | 6.4 |  | 14.9 | 13.5 |  | 12.9 | 12.5 |  |
| Actuated g/C Ratio | 0.18 | 0.18 |  | 0.18 | 0.18 |  | 0.42 | 0.38 |  | 0.36 | 0.35 |  |
| Clearance Time (s) | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.8 | 4.8 |  | 4.8 | 4.8 |  |
| Vehicle Extension (s) | 2.5 | 2.5 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Lane Grp Cap (vph) | 224 | 271 |  | 210 | 290 |  | 345 | 1229 |  | 322 | 1132 |  |
| v/s Ratio Prot |  | 0.03 |  |  | 0.03 |  | c0.01 | 0.14 |  | 0.00 | c0.16 |  |
| v/s Ratio Perm | c0.08 |  |  | 0.03 |  |  | 0.06 |  |  | 0.03 |  |  |
| v/c Ratio | 0.42 | 0.14 |  | 0.17 | 0.14 |  | 0.16 | 0.37 |  | 0.09 | 0.46 |  |
| Uniform Delay, d1 | 12.9 | 12.2 |  | 12.3 | 12.2 |  | 6.2 | 7.9 |  | 7.3 | 8.9 |  |
| Progression Factor | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  |
| Incremental Delay, d2 | 0.9 | 0.2 |  | 0.3 | 0.2 |  | 0.2 | 0.1 |  | 0.1 | 0.2 |  |
| Delay (s) | 13.8 | 12.4 |  | 12.5 | 12.3 |  | 6.3 | 8.0 |  | 7.4 | 9.1 |  |
| Level of Service | B | B |  | B | B |  | A | A |  | A | A |  |
| Approach Delay (s) |  | 12.9 |  |  | 12.4 |  |  | 7.8 |  |  | 9.0 |  |
| Approach LOS |  | B |  |  | B |  |  | A |  |  | A |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 9.5 |  | HCM 2000 | evel of | Service |  | A |  |  |  |
| HCM 2000 Volume to Capacity ratio |  |  | 0.43 |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length (s) |  |  | 35.4 |  | Sum of los | time (s) |  |  | 15.1 |  |  |  |
| Intersection Capacity Utilization |  |  | 39.7\% |  | ICU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |
| c Critical Lane Group |  |  |  |  |  |  |  |  |  |  |  |  |


|  | $\rangle$ | $\rightarrow$ |  | $\dagger$ | - | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\$$ |  |  | \$ |  |  | \$ |  |
| Traffic Volume (vph) | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.934 |  |  |  |  |  | 0.973 |  |  |  |  |
| Flt Protected |  |  |  |  | 0.988 |  |  | 0.962 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1634 | 0 | 0 | 1729 | 0 | 0 | 1638 | 0 | 0 | 1750 | 0 |
| Flt Permitted |  |  |  |  | 0.988 |  |  | 0.962 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1634 | 0 | 0 | 1729 | 0 | 0 | 1638 | 0 | 0 | 1750 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 493 |  |  | 457 |  |  | 274 |  |
| Travel Time (s) |  | 8.6 |  |  | 13.4 |  |  | 12.5 |  |  | 6.2 |  |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Adj. Flow (vph) | 0 | 26 | 25 | 19 | 62 | 0 | 48 | 0 | 12 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 51 | 0 | 0 | 81 | 0 | 0 | 60 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(tt) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 20.3\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.7 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \& |  |  | \$ |  |  | * |  |
| Traffic Vol, veh/h | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 20 | 19 | 15 | 48 | 0 | 37 | 0 | 9 | 0 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 26 | 25 | 19 | 62 | 0 | 48 | 0 | 12 | 0 | 0 | 0 |



|  | 7 | 4 | $\uparrow$ | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 中t |  |  | 个4 |
| Traffic Volume（vph） | 0 |  | 451 | 0 | 0 | 440 |
| Future Volume（vph） | 0 | 0 | 451 | 0 | 0 | 440 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util．Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  |  |  |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd．Flow（prot） | 0 | 1750 | 3260 | 0 | 0 | 3260 |
| FIt Permitted |  |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 1750 | 3260 | 0 | 0 | 3260 |
| Link Speed（mph） | 30 |  | 40 |  |  | 40 |
| Link Distance（ft） | 243 |  | 716 |  |  | 923 |
| Travel Time（s） | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Heavy Vehicles（\％） | 0\％ | 0\％ | 2\％ | 0\％ | 0\％ | 2\％ |
| Adj．Flow（vph） | 0 | 0 | 550 | 0 | 0 | 537 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 0 | 550 | 0 | 0 | 537 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width（ft） | 0 |  | 12 |  |  | 12 |
| Link Offset（ft） | 0 |  | 0 |  |  | 0 |
| Crosswalk Width（tt） | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type：Other |  |  |  |  |  |  |
| Control Type：Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 16．9\％ |  |  |  | ICU Level of Service A |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{7}$ | 4t |  |  |  |
| Traffic Vol, veh/h | 0 | 0 | 451 | 0 | 0 | 440 |
| Future Vol, veh/h | 0 | 0 | 451 | 0 | 0 | 440 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 |
| Heavy Vehicles, $\%$ | 0 | 0 | 2 | 0 | 0 | 2 |
| Mvmt Flow | 0 | 0 | 550 | 0 | 0 | 537 |



## 2026 PM DESIGN HOUR NO- BUILD CONDITIONS

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 性 |  |
| Traffic Volume (vph) | 89 | 31 | 120 | 28 | 15 | 43 | 126 | 690 | 44 | 41 | 667 | 68 |
| Future Volume (vph) | 89 | 31 | 120 | 28 | 15 | 43 | 126 | 690 | 44 | 41 | 667 | 68 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.881 |  |  | 0.889 |  |  | 0.991 |  |  | 0.986 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1530 | 0 | 1662 | 1556 | 0 | 1662 | 3291 | 0 | 1662 | 3278 | 0 |
| Flt Permitted | 0.717 |  |  | 0.655 |  |  | 0.249 |  |  | 0.356 |  |  |
| Satd. Flow (perm) | 1255 | 1530 | 0 | 1146 | 1556 | 0 | 436 | 3291 | 0 | 623 | 3278 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 128 |  |  | 46 |  |  | 10 |  |  | 15 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 0\% | 2\% | 0\% | 0\% | 0\% |
| Adj. Flow (vph) | 95 | 33 | 128 | 30 | 16 | 46 | 134 | 734 | 47 | 44 | 710 | 72 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 95 | 161 | 0 | 30 | 62 | 0 | 134 | 781 | 0 | 44 | 782 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2026 PM DESIGN HOUR NO- BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street

|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

Splits and Phases: 1: HWY 101 \& 35th Street


## 2026 PM DESIGN HOUR NO- BUILD CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/15/2020


|  | 4 | $\rightarrow$ |  | 7 |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | ¢ |  |  | \$ |  |
| Traffic Volume (vph) | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.933 |  |  |  |  |  | 0.959 |  |  |  |  |
| Flt Protected |  |  |  |  | 0.981 |  |  | 0.966 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1633 | 0 | 0 | 1717 | 0 | 0 | 1621 | 0 | 0 | 1750 | 0 |
| Flt Permitted |  |  |  |  | 0.981 |  |  | 0.966 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1633 | 0 | 0 | 1717 | 0 | 0 | 1621 | 0 | 0 | 1750 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 225 |  |  | 457 |  |  | 251 |  |
| Travel Time (s) |  | 8.6 |  |  | 6.1 |  |  | 12.5 |  |  | 5.7 |  |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Adj. Flow (vph) | 0 | 72 | 70 | 35 | 56 | 0 | 49 | 0 | 21 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 142 | 0 | 0 | 91 | 0 | 0 | 70 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 21.2\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.2 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \& |  |  | \$ |  |  | * |  |
| Traffic Vol, veh/h | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Future Vol, veh/h | 0 | 59 | 57 | 29 | 46 | 0 | 40 | 0 | 17 | 0 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 72 | 70 | 35 | 56 | 0 | 49 | 0 | 21 | 0 | 0 | 0 |



|  | 7 | 4 | 9 |  |  | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 4\% |  |  | 44 |
| Traffic Volume (vph) | 0 | 0 | 822 | 0 | 0 | 776 |
| Future Volume (vph) | 0 | 0 | 822 | 0 | 0 | 776 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  |  |  |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd. Flow (prot) | 0 | 1750 | 3325 | 0 | 0 | 3325 |
| Flt Permitted |  |  |  |  |  |  |
| Satd. Flow (perm) | 0 | 1750 | 3325 | 0 | 0 | 3325 |
| Link Speed (mph) | 30 |  | 40 |  |  | 40 |
| Link Distance (ft) | 243 |  | 716 |  |  | 923 |
| Travel Time (s) | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adj. Flow (vph) | 0 | 0 | 874 | 0 | 0 | 826 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 874 | 0 | 0 | 826 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 0 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 28.0\% |  |  |  |  | Level | Service A |
| Analysis Period (min) 15 |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{7}$ | 作 |  |  |  |
| Traffic Vol, veh/h | 0 | 0 | 822 | 0 | 0 | 776 |
| Future Vol, veh/h | 0 | 0 | 822 | 0 | 0 | 776 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 0 | 874 | 0 | 0 | 826 |



# APPENDIX H Build 

## SYNCHRO Performance Calculations

## 2021 AM DESIGN HOUR BUILD CONDITIONS

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{*}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{*}$ | 中 ${ }^{\text {a }}$ |  |
| Traffic Volume (vph) | 73 | 17 | 102 | 56 | 31 | 31 | 43 | 340 | 26 | 48 | 358 | 45 |
| Future Volume (vph) | 73 | 17 | 102 | 56 | 31 | 31 | 43 | 340 | 26 | 48 | 358 | 45 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.872 |  |  | 0.925 |  |  | 0.989 |  |  | 0.983 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1513 | 0 | 1662 | 1619 | 0 | 1662 | 3199 | 0 | 1662 | 3204 | 0 |
| Flt Permitted | 0.705 |  |  | 0.660 |  |  | 0.461 |  |  | 0.475 |  |  |
| Satd. Flow (perm) | 1234 | 1513 | 0 | 1155 | 1619 | 0 | 807 | 3199 | 0 | 831 | 3204 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 131 |  |  | 40 |  |  | 10 |  |  | 17 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Adj. Flow (vph) | 94 | 22 | 131 | 72 | 40 | 40 | 55 | 436 | 33 | 62 | 459 | 58 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 94 | 153 | 0 | 72 | 80 | 0 | 55 | 469 | 0 | 62 | 517 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2021 AM DESIGN HOUR BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


## 2021 AM DESIGN HOUR BUILD CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/18/2020


## 2021 AM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings
2: REDWOOD ST/Site \& 35th Street

|  | $\stackrel{ }{ }$ |  |  | 7 |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | ¢ |  |  | \$ |  |
| Traffic Volume (vph) | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Future Volume (vph) | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.972 |  |  |  |  |  | 0.974 |  |  | 0.874 |  |
| Flt Protected |  | 0.972 |  |  | 0.988 |  |  | 0.963 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1653 | 0 | 0 | 1729 | 0 | 0 | 1641 | 0 | 0 | 1530 | 0 |
| Flt Permitted |  | 0.972 |  |  | 0.988 |  |  | 0.963 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1653 | 0 | 0 | 1729 | 0 | 0 | 1641 | 0 | 0 | 1530 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 493 |  |  | 457 |  |  | 274 |  |
| Travel Time (s) |  | 8.6 |  |  | 13.4 |  |  | 12.5 |  |  | 6.2 |  |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Adj. Flow (vph) | 68 | 26 | 25 | 19 | 62 | 0 | 48 | 3 | 12 | 0 | 3 | 43 |
| Shared Lane Trafic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 119 | 0 | 0 | 81 | 0 | 0 | 63 | 0 | 0 | 46 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 28.0\%
ICU Level of Service A
Analysis Period (min) 15

## 2021 AM DESIGN HOUR BUILD CONDITIONS

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.6 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \& |  |  | \$ |  |  | * |  |
| Traffic Vol, veh/h | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Future Vol, veh/h | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 68 | 26 | 25 | 19 | 62 | 0 | 48 | 3 | 12 | 0 | 3 | 43 |



## 2021 AM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings
3: HWY 101 \& RIRO SITE ACCESS


## 2021 AM DESIGN HOUR BUILD CONDITIONS

HCM 2010 TWSC
3: HWY 101 \& RIRO SITE ACCESS



## 2021 PM DESIGN HOUR BUILD CONDITIONS

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | 个 |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{*}$ | 中 ${ }^{\text {F }}$ |  |
| Traffic Volume（vph） | 89 | 35 | 120 | 50 | 19 | 43 | 126 | 683 | 62 | 62 | 654 | 68 |
| Future Volume（vph） | 89 | 35 | 120 | 50 | 19 | 43 | 126 | 683 | 62 | 62 | 654 | 68 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length（ft） | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length（ft） | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util．Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.884 |  |  | 0.895 |  |  | 0.988 |  |  | 0.986 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（prot） | 1662 | 1535 | 0 | 1662 | 1566 | 0 | 1662 | 3280 | 0 | 1662 | 3278 | 0 |
| Flt Permitted | 0.714 |  |  | 0.653 |  |  | 0.267 |  |  | 0.331 |  |  |
| Satd．Flow（perm） | 1250 | 1535 | 0 | 1143 | 1566 | 0 | 467 | 3280 | 0 | 579 | 3278 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  | 128 |  |  | 46 |  |  | 14 |  |  | 15 |  |
| Link Speed（mph） |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance（ft） |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time（s） |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles（\％） | 0\％ | 0\％ | 1\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 2\％ | 0\％ | 0\％ | 0\％ |
| Adj．Flow（vph） | 95 | 37 | 128 | 53 | 20 | 46 | 134 | 727 | 66 | 66 | 696 | 72 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 95 | 165 | 0 | 53 | 66 | 0 | 134 | 793 | 0 | 66 | 768 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset（ft） |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width（ft） |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector（ft） | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector（ft） | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position（ft） | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size（ft） | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex |  | Cl＋Ex | Cl＋Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue（s） | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay（s） | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position（ft） | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size（ft） | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl＋Ex | Cl＋Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend（s） | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2021 PM DESIGN HOUR BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


## 2021 PM DESIGN HOUR BUILD CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/18/2020


## 2021 PM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings
2: REDWOOD ST/Site \& 35th Street
06/18/2020

|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | $\uparrow$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ${ }_{*}$ |  |  | ${ }_{4}$ |  |  | \$ |  |
| Traffic Volume (vph) | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Future Volume (vph) | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.951 |  |  |  |  |  | 0.961 |  |  | 0.869 |  |
| Flt Protected |  | 0.987 |  |  | 0.981 |  |  | 0.967 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1643 | 0 | 0 | 1717 | 0 | 0 | 1626 | 0 | 0 | 1521 | 0 |
| Flt Permitted |  | 0.987 |  |  | 0.981 |  |  | 0.967 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1643 | 0 | 0 | 1717 | 0 | 0 | 1626 | 0 | 0 | 1521 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 225 |  |  | 457 |  |  | 251 |  |
| Travel Time (s) |  | 8.6 |  |  | 6.1 |  |  | 12.5 |  |  | 5.7 |  |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Adj. Flow (vph) | 52 | 72 | 70 | 35 | 56 | 0 | 49 | 2 | 21 | 0 | 1 | 32 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 194 | 0 | 0 | 91 | 0 | 0 | 72 | 0 | 0 | 33 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(t) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other
Control Type: Unsignalized
Intersection Capacity Utilization 28.3\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 4.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | * |  |  | 4 |  |  | * |  |
| Traffic Vol, veh/h | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Future Vol, veh/h | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 52 | 72 | 70 | 35 | 56 | 0 | 49 | 2 | 21 | 0 | 1 | 32 |



## 2021 PM DESIGN HOUR BUILD CONDITIONS

Lanes，Volumes，Timings
3：HWY 101 \＆RIRO SITE ACCESS

|  | $\dagger$ | 4 | $\dagger$ | $>$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 性 |  |  | 个个 |
| Trafic Volume（vph） | 0 | 20 | 809 | 6 | 0 | 784 |
| Future Volume（vph） | 0 | 20 | 809 | 6 | 0 | 784 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util．Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  | 0.865 | 0.999 |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd．Flow（prot） | 0 | 1514 | 3322 | 0 | 0 | 3325 |
| Flt Permitted |  |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 1514 | 3322 | 0 | 0 | 3325 |
| Link Speed（mph） | 30 |  | 40 |  |  | 40 |
| Link Distance（ft） | 243 |  | 716 |  |  | 923 |
| Travel Time（s） | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adj．Flow（vph） | 0 | 21 | 861 | 6 | 0 | 834 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 21 | 867 | 0 | 0 | 834 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width（ft） | 0 |  | 12 |  |  | 12 |
| Link Offset（ft） | 0 |  | 0 |  |  | 0 |
| Crosswalk Width（ft） | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type：Other |  |  |  |  |  |  |
| Control Type：Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 34．5\％ |  |  |  | ICU Level of Service A |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |

## 2021 PM DESIGN HOUR BUILD CONDITIONS

HCM 2010 TWSC
3: HWY 101 \& RIRO SITE ACCESS

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{T}$ | 个 |  |  |  |
| Traffic Vol, veh/h | 0 | 20 | 809 | 6 | 0 | 784 |
| Future Vol, veh/h | 0 | 20 | 809 | 6 | 0 | 784 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 21 | 861 | 6 | 0 | 834 |



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | \% | $\uparrow$ |  | \% | 中 ${ }^{\text {a }}$ |  | \% | 中 ${ }^{\text {c }}$ |  |
| Traffic Volume (vph) | 73 | 17 | 102 | 56 | 31 | 31 | 43 | 340 | 26 | 48 | 358 | 45 |
| Future Volume (vph) | 73 | 17 | 102 | 56 | 31 | 31 | 43 | 340 | 26 | 48 | 358 | 45 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 110 |  | 0 | 141 |  | 0 | 146 |  | 0 | 91 |  | 0 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 100 |  |  | 55 |  |  | 66 |  |  | 54 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 |
| Frt |  | 0.872 |  |  | 0.925 |  |  | 0.989 |  |  | 0.983 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1662 | 1513 | 0 | 1662 | 1619 | 0 | 1662 | 3199 | 0 | 1662 | 3204 | 0 |
| Flt Permitted | 0.705 |  |  | 0.660 |  |  | 0.461 |  |  | 0.475 |  |  |
| Satd. Flow (perm) | 1234 | 1513 | 0 | 1155 | 1619 | 0 | 807 | 3199 | 0 | 831 | 3204 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 131 |  |  | 40 |  |  | 10 |  |  | 17 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 40 |  |  | 40 |  |
| Link Distance (ft) |  | 499 |  |  | 317 |  |  | 925 |  |  | 716 |  |
| Travel Time (s) |  | 13.6 |  |  | 8.6 |  |  | 15.8 |  |  | 12.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Heavy Vehicles (\%) | 0\% | 0\% | 1\% | 0\% | 0\% | 0\% | 0\% | 3\% | 0\% | 0\% | 2\% | 2\% |
| Adj. Flow (vph) | 94 | 22 | 131 | 72 | 40 | 40 | 55 | 436 | 33 | 62 | 459 | 58 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 94 | 153 | 0 | 72 | 80 | 0 | 55 | 469 | 0 | 62 | 517 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  | Yes |  |  |  |  |  | Yes |  |  | Yes |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Side St | Side St |  | Side St | Side St |  | Left |  |  | Left |  |  |
| Leading Detector (ft) | 78 | 78 |  | 78 | 78 |  | 78 | 223 |  | 78 | 223 |  |
| Trailing Detector (ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Position(ft) | 2 | 2 |  | 2 | 2 |  | 2 | 157 |  | 2 | 157 |  |
| Detector 1 Size(ft) | 16 | 16 |  | 16 | 16 |  | 16 | 6 |  | 16 | 6 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 | 72 |  | 72 | 72 |  | 72 | 217 |  | 72 | 217 |  |
| Detector 2 Size(ft) | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Perm | NA |  | Perm | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases |  | 8 |  |  | 4 |  | 1 | 6 |  | 5 | 2 |  |
| Permitted Phases | 8 |  |  | 4 |  |  | 6 |  |  | 2 |  |  |

## 2026 AM DESIGN HOUR BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


## 2026 AM DESIGN HOUR BUILD CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/18/2020


## 2026 AM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings
2: REDWOOD ST/Site \& 35th Street

|  | $\stackrel{ }{ }$ |  |  | 7 |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | ¢ |  |  | \$ |  |
| Traffic Volume (vph) | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Future Volume (vph) | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.972 |  |  |  |  |  | 0.974 |  |  | 0.874 |  |
| Flt Protected |  | 0.972 |  |  | 0.988 |  |  | 0.963 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1653 | 0 | 0 | 1729 | 0 | 0 | 1641 | 0 | 0 | 1530 | 0 |
| Flt Permitted |  | 0.972 |  |  | 0.988 |  |  | 0.963 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1653 | 0 | 0 | 1729 | 0 | 0 | 1641 | 0 | 0 | 1530 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 493 |  |  | 457 |  |  | 274 |  |
| Travel Time (s) |  | 8.6 |  |  | 13.4 |  |  | 12.5 |  |  | 6.2 |  |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Adj. Flow (vph) | 68 | 26 | 25 | 19 | 62 | 0 | 48 | 3 | 12 | 0 | 3 | 43 |
| Shared Lane Trafic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 119 | 0 | 0 | 81 | 0 | 0 | 63 | 0 | 0 | 46 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 28.0\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.6 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \& |  |  | \$ |  |  | * |  |
| Traffic Vol, veh/h | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Future Vol, veh/h | 52 | 20 | 19 | 15 | 48 | 0 | 37 | 2 | 9 | 0 | 2 | 33 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 | 77 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 68 | 26 | 25 | 19 | 62 | 0 | 48 | 3 | 12 | 0 | 3 | 43 |



## 2026 AM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings
3: HWY 101 \& RIRO SITE ACCESS


## 2026 AM DESIGN HOUR BUILD CONDITIONS

HCM 2010 TWSC
3: HWY 101 \& RIRO SITE ACCESS

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{T}$ | 个 |  |  |  |
| Traffic Vol, veh/h | 0 | 24 | 437 | 7 | 0 | 451 |
| Future Vol, veh/h | 0 | 24 | 437 | 7 | 0 | 451 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 |
| Heavy Vehicles, \% | 0 | 0 | 2 | 0 | 0 | 2 |
| Mvmt Flow | 0 | 29 | 533 | 9 | 0 | 550 |



|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |

Synchro 7 - Report Page 1

## 2026 PM DESIGN HOUR BUILD CONDITIONS

## Lanes, Volumes, Timings

1: HWY 101 \& 35th Street


Splits and Phases: 1: HWY 101 \& 35th Street


## 2026 PM DESIGN HOUR BUILD CONDITIONS

HCM Signalized Intersection Capacity Analysis
1: HWY 101 \& 35th Street
06/18/2020


|  | 4 | $\rightarrow$ |  | 7 | - | 4 |  | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ${ }_{\$}$ |  |  | ${ }_{\$}$ |  |  | \$ |  |
| Traffic Volume (vph) | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Future Volume (vph) | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.951 |  |  |  |  |  | 0.961 |  |  | 0.869 |  |
| Flt Protected |  | 0.987 |  |  | 0.981 |  |  | 0.967 |  |  |  |  |
| Satd. Flow (prot) | 0 | 1643 | 0 | 0 | 1717 | 0 | 0 | 1626 | 0 | 0 | 1521 | 0 |
| Flt Permitted |  | 0.987 |  |  | 0.981 |  |  | 0.967 |  |  |  |  |
| Satd. Flow (perm) | 0 | 1643 | 0 | 0 | 1717 | 0 | 0 | 1626 | 0 | 0 | 1521 | 0 |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 30 |  |
| Link Distance (ft) |  | 317 |  |  | 225 |  |  | 457 |  |  | 251 |  |
| Travel Time (s) |  | 8.6 |  |  | 6.1 |  |  | 12.5 |  |  | 5.7 |  |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Adj. Flow (vph) | 52 | 72 | 70 | 35 | 56 | 0 | 49 | 2 | 21 | 0 | 1 | 32 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 194 | 0 | 0 | 91 | 0 | 0 | 72 | 0 | 0 | 33 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 35 |  |
| Crosswalk Width(tt) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 28.3\%
ICU Level of Service A
Analysis Period (min) 15

HCM 2010 TWSC
2: REDWOOD ST/Site \& 35th Street

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 4.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  |  | \& |  |  | * |  |  | * |  |
| Traffic Vol, veh/h | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Future Vol, veh/h | 43 | 59 | 57 | 29 | 46 | 0 | 40 | 2 | 17 | 0 | 1 | 26 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 | 82 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 52 | 72 | 70 | 35 | 56 | 0 | 49 | 2 | 21 | 0 | 1 | 32 |



## 2026 PM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings
3: HWY 101 \& RIRO SITE ACCESS

|  |  | 4 | 9 |  |  | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | F | 4\% |  |  | 44 |
| Traffic Volume (vph) | 0 | 20 | 810 | 6 | 0 | 785 |
| Future Volume (vph) | 0 | 20 | 810 | 6 | 0 | 785 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Lane Util. Factor | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 0.95 |
| Frt |  | 0.865 | 0.999 |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd. Flow (prot) | 0 | 1514 | 3322 | 0 | 0 | 3325 |
| Flt Permitted |  |  |  |  |  |  |
| Satd. Flow (perm) | 0 | 1514 | 3322 | 0 | 0 | 3325 |
| Link Speed (mph) | 30 |  | 40 |  |  | 40 |
| Link Distance (ft) | 243 |  | 716 |  |  | 923 |
| Travel Time (s) | 5.5 |  | 12.2 |  |  | 15.7 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Adj. Flow (vph) | 0 | 21 | 862 | 6 | 0 | 835 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 21 | 868 | 0 | 0 | 835 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 0 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 12 |  | 12 |  |  | 12 |
| Two way Left Turn Lane |  |  | Yes |  |  | Yes |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 34.5\% |  |  |  |  | Level | Service A |
| Analysis Period (min) 15 |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | $\mathbf{7}$ | 个 |  |  |  |
| Traffic Vol, veh/h | 0 | 20 | 810 | 6 | 0 | 785 |
| Future Vol, veh/h | 0 | 20 | 810 | 6 | 0 | 785 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 |
| Heavy Vehicles, \% | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 0 | 21 | 862 | 6 | 0 | 835 |



## APPENDIX I

Existing and No-Build SimTraffic Queue Length Calculations

Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 66 | 66 | 49 | 50 | 54 | 122 | 77 | 53 | 119 | 142 |
| Average Queue (ft) | 40 | 44 | 24 | 31 | 26 | 71 | 33 | 21 | 74 | 76 |
| 95th Queue (ft) | 73 | 67 | 57 | 58 | 58 | 121 | 81 | 62 | 125 | 143 |
| Link Distance) (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  | 91 |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 | 0 |  |  | 4 |  |
| Storage Blk Time (\%) |  |  |  |  |  | 0 |  |  | 1 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 79 | 64 | 40 | 64 | 39 | 107 | 59 | 44 | 101 | 75 |
| Average Queue (ft) | 32 | 35 | 18 | 27 | 16 | 48 | 20 | 9 | 48 | 32 |
| 95th Queue (ft) | 66 | 60 | 46 | 57 | 43 | 89 | 50 | 34 | 83 | 67 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  | 0 |
| Storage Blk Time (\%) | 0 |  |  |  |  | 0 |  |  | 0 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 84 | 66 | 54 | 64 | 54 | 127 | 82 | 63 | 124 | 142 |
| Average Queue (ft) | 34 | 37 | 19 | 28 | 18 | 53 | 23 | 12 | 54 | 42 |
| 95th Queue (ft) | 68 | 62 | 49 | 58 | 48 | 100 | 59 | 43 | 98 | 97 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 1 |  |  |
| Storage Blk Time (\%) | 0 |  |  |  |  | 0 |  |  | 0 |  |

Queuing and Blocking Report 2020 CONDITIONS

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 16 | 41 |
| Average Queue (ft) | 2 | 29 |
| 95th Queue (ft) | 19 | 56 |
| Link Distance (ft) | 468 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | NB |
| :--- | ---: |
| Directions Served | LTR |
| Maximum Queue (ft) | 40 |
| Average Queue (ft) | 28 |
| 95th Queue (ft) | 52 |
| Link Distance (ft) | 427 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 16 | 46 |
| Average Queue (ft) | 1 | 28 |
| 95th Queue (ft) | 9 | 53 |
| Link Distance (ft) | 468 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Network Summary |
| Network wide Queuing Penalty, Interval \#1: 1 |
| Network wide Queuing Penalty, Interval \#2: 0 |
| Network wide Queuing Penalty, All Intervals: 0 |

Queuing and Blocking Report
EXISTING
Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 74 | 100 | 49 | 53 | 84 | 153 | 132 | 78 | 150 | 124 |
| Average Queue (ft) | 43 | 59 | 25 | 27 | 50 | 89 | 73 | 35 | 97 | 82 |
| 95th Queue (ft) | 78 | 105 | 59 | 59 | 87 | 152 | 134 | 84 | 160 | 134 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) |  | 1 |  |  |  | 1 |  |  | 10 |  |
| Queuing Penalty (veh) |  | 1 |  |  |  | 1 |  |  | 4 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 91 | 117 | 69 | 69 | 92 | 138 | 126 | 70 | 175 | 158 |
| Average Queue (ft) | 44 | 53 | 19 | 31 | 45 | 77 | 60 | 24 | 95 | 82 |
| 95th Queue (ft) | 81 | 95 | 53 | 58 | 78 | 130 | 104 | 58 | 154 | 145 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 | 8 |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |  | 0 |  |  | 3 |  |
| Queuing Penalty (veh) | 0 | 0 |  |  |  | 0 |  |  |  |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 96 | 126 | 70 | 69 | 102 | 161 | 150 | 108 | 178 | 158 |
| Average Queue (ft) | 44 | 54 | 21 | 30 | 46 | 80 | 63 | 27 | 96 | 82 |
| 95th Queue (ft) | 80 | 97 | 55 | 59 | 80 | 136 | 113 | 65 | 155 | 142 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 9 |  |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |  | 0 |  |  | 9 |  |

Queuing and Blocking Report EXISTING

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 26 | 55 |
| Average Queue (ft) | 4 | 37 |
| 95th Queue (ft) | 28 | 57 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 21 | 52 |
| Average Queue (ft) | 2 | 34 |
| 95th Queue (ft) | 12 | 55 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 37 | 61 |
| Average Queue (ft) | 2 | 35 |
| 95th Queue (ft) | 17 | 56 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Queuing and Blocking Report EXISTING

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Network Summary |
| Network wide Queuing Penalty, Interval \#1: 6 |
| Network wide Queuing Penalty, Interval \#2: 4 |
| Network wide Queuing Penalty, All Intervals: 4 |

## 2021 AM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2021 AM No-Build Conditions
Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 101 | 76 | 35 | 54 | 54 | 110 | 105 | 49 | 121 | 119 |
| Average Queue (ft) | 53 | 42 | 20 | 35 | 35 | 73 | 49 | 18 | 80 | 62 |
| 95th Queue (ft) | 102 | 71 | 48 | 56 | 58 | 119 | 105 | 51 | 132 | 124 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 5 | 1 |  |
| Storage Blk Time (\%) | 1 |  |  |  |  | 0 |  |  |  |  |
| Queuing Penalty (veh) | 1 |  |  |  |  | 0 |  |  |  |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 81 | 64 | 45 | 59 | 54 | 84 | 66 | 39 | 119 | 117 |
| Average Queue (ft) | 37 | 37 | 14 | 26 | 20 | 45 | 19 | 12 | 48 | 35 |
| 95th Queue (ft) | 72 | 61 | 44 | 56 | 50 | 82 | 54 | 38 | 91 | 84 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 | 1 |  |
| Storage Blk Time (\%) | 0 |  |  |  |  |  |  |  | 0 |  |
| Queuing Penalty (veh) | 0 |  |  |  |  |  |  |  |  |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 117 | 76 | 45 | 65 | 65 | 110 | 105 | 54 | 143 | 137 |
| Average Queue (ft) | 41 | 38 | 16 | 28 | 24 | 52 | 27 | 13 | 56 | 42 |
| 95th Queue (ft) | 82 | 64 | 45 | 57 | 54 | 96 | 73 | 42 | 106 | 97 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 9 |  |  |
| Storage Blk Time (\%) | 0 |  |  |  |  | 0 |  |  | 0 |  |

## 2021 AM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2021 AM No-Build Conditions

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | NB |
| :--- | ---: |
| Directions Served | LTR |
| Maximum Queue (ft) | 56 |
| Average Queue (ft) | 36 |
| 95th Queue (ft) | 57 |
| Link Distance (ft) | 427 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 10 | 46 |
| Average Queue (ft) | 0 | 25 |
| 95th Queue (ft) | 6 | 52 |
| Link Distance (ft) | 468 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 10 | 56 |
| Average Queue (ft) | 0 | 27 |
| 95th Queue (ft) | 5 | 55 |
| Link Distance (ft) | 468 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## 2021 AM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2021 AM No-Build Conditions
Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Network Summary |
| Network wide Queuing Penalty, Interval \#1: 2 |
| Network wide Queuing Penalty, Interval \#2: 0 |
| Network wide Queuing Penalty, All Intervals: 1 |

Queuing and Blocking Report
2021 PM Background Conditions
Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 76 | 106 | 54 | 63 | 102 | 167 | 151 | 66 | 173 | 157 |
| Average Queue (ft) | 41 | 60 | 22 | 32 | 63 | 94 | 74 | 29 | 108 | 92 |
| 95th Queue (ft) | 81 | 116 | 56 | 68 | 102 | 160 | 149 | 73 | 175 | 163 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 | 1 |  |  |  | 1 |  |  | 11 |  |
| Queuing Penalty (veh) | 0 | 1 |  |  |  | 1 |  |  | 5 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 99 | 109 | 44 | 71 | 86 | 165 | 129 | 65 | 173 | 183 |
| Average Queue (ft) | 47 | 49 | 16 | 29 | 47 | 77 | 59 | 24 | 88 | 81 |
| 95th Queue (ft) | 79 | 90 | 46 | 62 | 84 | 136 | 112 | 56 | 146 | 149 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage BIk Time (\%) | 0 | 0 |  | 0 |  | 1 |  | 0 | 7 |  |
| Queuing Penalty (veh) | 0 | 0 |  | 0 |  | 1 |  | 0 | 3 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 105 | 126 | 54 | 75 | 104 | 195 | 168 | 81 | 182 | 185 |
| Average Queue (ft) | 46 | 52 | 18 | 29 | 51 | 81 | 63 | 25 | 93 | 84 |
| 95th Queue (ft) | 80 | 97 | 48 | 64 | 90 | 143 | 123 | 60 | 155 | 152 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 | 0 |  | 0 |  | 1 |  | 0 | 8 |  |
| Queuing Penalty (veh) | 0 | 0 |  | 0 |  | 1 |  | 0 | 3 |  |

## 2021 PM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2021 PM Background Conditions
Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 36 | 55 |
| Average Queue (ft) | 8 | 34 |
| 95th Queue (ft) | 34 | 64 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 16 | 52 |
| Average Queue (ft) | 1 | 28 |
| 95th Queue (ft) | 10 | 54 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 36 | 65 |
| Average Queue (ft) | 3 | 30 |
| 95th Queue (ft) | 18 | 57 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## 2021 PM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2021 PM Background Conditions

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2
Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Bk Time (\%) |
| Queuing Penalty (veh) |
| Network Summary |
| Network wide Queuing Penalty, Interval \#1: 7 |
| Network wide Queuing Penalty, Interval \#2: 4 |
| Network wide Queuing Penalty, All Intervals: 5 |

## 2026 AM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2026 AM No-Build Conditions
Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 68 | 64 | 35 | 54 | 52 | 134 | 74 | 34 | 100 | 105 |
| Average Queue (ft) | 42 | 43 | 21 | 30 | 27 | 63 | 37 | 16 | 67 | 54 |
| 95th Queue (ft) | 75 | 73 | 48 | 65 | 59 | 124 | 87 | 43 | 108 | 103 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) |  |  |  |  |  | 1 |  |  | 1 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 70 | 64 | 45 | 55 | 50 | 105 | 66 | 39 | 99 | 81 |
| Average Queue (ft) | 32 | 35 | 15 | 28 | 21 | 49 | 23 | 15 | 50 | 35 |
| 95th Queue (ft) | 65 | 63 | 44 | 53 | 50 | 91 | 55 | 41 | 85 | 73 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  |  |  |  |  |  | 1 |
| Storage Blk Time (\%) |  |  |  |  |  |  |  |  | 0 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 77 | 74 | 45 | 60 | 53 | 136 | 84 | 39 | 105 | 110 |
| Average Queue (ft) | 35 | 37 | 17 | 28 | 22 | 53 | 26 | 15 | 54 | 39 |
| 95th Queue (ft) | 69 | 66 | 46 | 56 | 52 | 101 | 65 | 42 | 93 | 83 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) |  |  |  |  |  | 0 |  |  | 1 |  |
| Queuing Penalty (veh) |  |  |  |  |  | 0 |  |  | 0 |  |

## 2026 AM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2026 AM No-Build Conditions

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | NB |
| :--- | ---: |
| Directions Served | LTR |
| Maximum Queue (ft) | 35 |
| Average Queue (ft) | 27 |
| 95th Queue (ft) | 51 |
| Link Distance (ft) | 427 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 10 | 61 |
| Average Queue (ft) | 1 | 30 |
| 95th Queue (ft) | 8 | 56 |
| Link Distance (ft) | 468 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 10 | 61 |
| Average Queue (ft) | 1 | 29 |
| 95th Queue (ft) | 7 | 55 |
| Link Distance (ft) | 468 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## 2026 AM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2026 AM No-Build Conditions

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2
Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Network Summary |
| Network wide Queuing Penalty, Interval \#1: 1 |
| Network wide Queuing Penalty, Interval \#2: 0 |
| Network wide Queuing Penalty, All Intervals: 0 |

Queuing and Blocking Report

Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 81 | 84 | 53 | 58 | 89 | 133 | 154 | 88 | 171 | 148 |
| Average Queue (ft) | 42 | 51 | 20 | 32 | 52 | 88 | 66 | 36 | 110 | 100 |
| 95th Queue (ft) | 86 | 83 | 55 | 65 | 93 | 153 | 140 | 88 | 179 | 163 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |  | 1 |  |  | 10 |  |
| Queuing Penalty (veh) | 1 | 0 |  |  |  | 1 |  |  | 4 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 94 | 100 | 54 | 68 | 92 | 154 | 131 | 76 | 182 | 166 |
| Average Queue (ft) | 47 | 50 | 21 | 30 | 46 | 77 | 60 | 24 | 94 | 83 |
| 95th Queue (ft) | 82 | 86 | 52 | 60 | 77 | 133 | 108 | 61 | 159 | 148 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |  | 0 |  | 0 | 8 |  |
| Queuing Penalty (veh) | 0 | 0 |  |  |  | 0 |  | 0 | 3 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 98 | 105 | 64 | 78 | 99 | 156 | 160 | 106 | 200 | 178 |
| Average Queue (ft) | 45 | 50 | 21 | 30 | 47 | 80 | 61 | 27 | 97 | 87 |
| 95th Queue (ft) | 83 | 85 | 53 | 61 | 81 | 139 | 117 | 69 | 165 | 153 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |  | 0 |  | 0 | 8 |  |
| Queuing Penalty (veh) | 0 | 0 |  |  |  | 1 |  | 0 | 3 |  |

Queuing and Blocking Report
2026 PM No-Build Conditions

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 25 | 56 |
| Average Queue (ft) | 6 | 35 |
| 95th Queue (ft) | 26 | 59 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 26 | 57 |
| Average Queue (ft) | 3 | 32 |
| 95th Queue (ft) | 17 | 54 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | WB | NB |
| :--- | ---: | ---: |
| Directions Served | LTR | LTR |
| Maximum Queue (ft) | 36 | 62 |
| Average Queue (ft) | 3 | 32 |
| 95th Queue (tt) | 19 | 56 |
| Link Distance (ft) | 201 | 427 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## 2026 PM DESIGN HOUR NO- BUILD

Queuing and Blocking Report
2026 PM No-Build Conditions

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2
Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |
| Network Summary |
| Network wide Queuing Penalty, Interval \#1: 6 |
| Network wide Queuing Penalty, Interval \#2: 4 |
| Network wide Queuing Penalty, All Intervals: 5 |

# APPENDIX J Build 

## SimTraffic Queue Length Calculations

Queuing and Blocking Report

Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 83 | 55 | 64 | 60 | 59 | 122 | 93 | 60 | 94 | 105 |
| Average Queue (ft) | 41 | 41 | 34 | 34 | 32 | 77 | 46 | 36 | 66 | 62 |
| 95th Queue (ft) | 81 | 60 | 70 | 67 | 64 | 129 | 100 | 68 | 105 | 113 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 |  |  |  |  | 0 |  | 0 | 2 |  |
| Queuing Penalty (veh) | 0 |  |  |  |  | 0 |  | 0 | 1 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 65 | 69 | 60 | 54 | 54 | 125 | 96 | 60 | 96 | 89 |
| Average Queue (ft) | 33 | 37 | 32 | 26 | 23 | 52 | 32 | 23 | 51 | 37 |
| 95th Queue (ft) | 66 | 64 | 59 | 55 | 53 | 100 | 75 | 55 | 89 | 78 |
| Link Distance (ft) |  | 456 |  | 212 |  | 89 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 9 | 0 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 85 | 69 | 65 | 65 | 60 | 143 | 120 | 65 | 105 | 106 |
| Average Queue (ft) | 35 | 38 | 32 | 28 | 26 | 58 | 35 | 26 | 55 | 43 |
| 95th Queue (ft) | 70 | 63 | 62 | 58 | 56 | 110 | 82 | 60 | 94 | 90 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) | 0 |  |  |  |  | 0 |  | 0 | 1 |  |
| Queuing Penalty (veh) | 0 |  |  |  |  | 0 |  | 0 | 0 |  |

Queuing and Blocking Report

Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 9 | 5 | 41 | 34 |
| Average Queue (ft) | 1 | 1 | 28 | 26 |
| 95th Queue (ft) | 10 | 8 | 53 | 48 |
| Link Distance (ft) | 212 | 468 | 427 | 240 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 17 | 10 | 46 | 45 |
| Average Queue (ft) | 1 | 0 | 29 | 21 |
| 95th Queue (ft) | 11 | 6 | 55 | 50 |
| Link Distance (ft) | 212 | 468 | 427 | 240 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 22 | 15 | 52 | 45 |
| Average Queue (ft) | 1 | 1 | 29 | 23 |
| 95th Queue (ft) | 11 | 6 | 54 | 50 |
| Link Distance (ft) | 212 | 468 | 427 | 240 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Queuing and Blocking Report

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 43 |
| Average Queue (ft) | 23 |
| 95th Queue (ft) | 52 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 40 |
| Average Queue (ft) | 17 |
| 95th Queue (ft) | 46 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 43 |
| Average Queue (ft) | 19 |
| 95th Queue (ft) | 48 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
|  |  |
| Network Summary |  |
| Network wide Queuing Penalty, Interval \#1: 2 |  |
| Network wide Queuing Penalty, Interval \#2: 0 |  |
| Network wide Queuing Penalty, All Intervals: 1 |  |

## 2021 PM DESIGN HOUR BUILD CONDITIONS

Queuing and Blocking Report

Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 93 | 80 | 53 | 54 | 83 | 161 | 161 | 81 | 172 | 167 |
| Average Queue (ft) | 54 | 55 | 27 | 31 | 52 | 88 | 70 | 35 | 102 | 87 |
| 95th Queue (ft) | 92 | 84 | 60 | 55 | 87 | 147 | 135 | 85 | 176 | 169 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 0 | 12 |  |
| Storage Blk Time (\%) | 1 |  |  |  |  | 1 |  | 1 | 8 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| irections Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 81 | 111 | 64 | 63 | 81 | 149 | 140 | 102 | 172 | 167 |
| Average Queue (ft) | 44 | 53 | 33 | 30 | 46 | 82 | 68 | 37 | 93 | 85 |
| 95th Queue (ft) | 74 | 90 | 63 | 62 | 75 | 134 | 124 | 78 | 151 | 150 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) |  | 0 |  |  |  | 0 |  | 0 | 7 |  |
| Queuing Penalty (veh) |  | 0 |  |  |  | 0 |  | 0 | 5 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 94 | 112 | 64 | 64 | 95 | 170 | 167 | 119 | 199 | 190 |
| Average Queue (ft) | 46 | 53 | 31 | 31 | 47 | 84 | 69 | 36 | 95 | 86 |
| 95th Queue (ft) | 79 | 89 | 62 | 60 | 78 | 138 | 127 | 80 | 158 | 155 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 0 | 9 |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |  | 0 |  | 0 | 5 |  |

Queuing and Blocking Report

Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 2 | 15 | 65 | 35 |
| Average Queue (ft) | 0 | 4 | 37 | 21 |
| 95th Queue (ft) | 2 | 18 | 67 | 48 |
| Link Distance (ft) | 211 | 201 | 427 | 216 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 16 | 27 | 52 | 35 |
| Average Queue (ft) | 1 | 3 | 32 | 20 |
| 95th Queue (ft) | 8 | 20 | 54 | 47 |
| Link Distance (ft) | 211 | 201 | 427 | 216 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 16 | 27 | 71 | 35 |
| Average Queue (ft) | 1 | 3 | 33 | 20 |
| 95th Queue (ft) | 7 | 20 | 58 | 48 |
| Link Distance (ft) | 211 | 201 | 427 | 216 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## 2021 PM DESIGN HOUR BUILD CONDITIONS

Queuing and Blocking Report

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 35 |
| Average Queue (ft) | 22 |
| 95th Queue (ft) | 48 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 45 |
| Average Queue (ft) | 21 |
| 95th Queue (ft) | 49 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 45 |
| Average Queue (ft) | 21 |
| 95th Queue (ft) | 49 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
|  |  |
| Network Summary |  |
| Network wide Queuing Penalty, Interval \#1: 11 |  |
| Network wide Queuing Penalty, Interval \#2: 5 |  |
| Network wide Queuing Penalty, All Intervals: 7 |  |

Queuing and Blocking Report

Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 74 | 93 | 64 | 53 | 54 | 131 | 90 | 44 | 101 | 109 |
| Average Queue (ft) | 47 | 48 | 38 | 30 | 29 | 66 | 46 | 26 | 66 | 58 |
| 95th Queue (ft) | 78 | 94 | 69 | 64 | 57 | 127 | 93 | 52 | 105 | 103 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 146 |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  |  | 0 |  |  | 2 | 1 |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| irections Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 65 | 62 | 62 | 65 | 63 | 123 | 96 | 86 | 107 | 113 |
| Average Queue (ft) | 32 | 35 | 30 | 25 | 23 | 54 | 26 | 25 | 49 | 36 |
| 95th Queue (ft) | 57 | 59 | 58 | 55 | 54 | 104 | 71 | 63 | 86 | 76 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream BIk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) |  |  |  |  |  | 0 |  | 0 | 1 |  |
| Queuing Penalty (veh) |  |  |  |  |  | 0 |  | 0 | 0 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 79 | 93 | 66 | 73 | 64 | 136 | 101 | 86 | 125 | 127 |
| Average Queue (ft) | 36 | 38 | 32 | 26 | 24 | 57 | 31 | 25 | 53 | 41 |
| 95th Queue (ft) | 64 | 70 | 61 | 58 | 55 | 110 | 79 | 61 | 93 | 85 |
| Link Distance (ft) |  | 456 |  | 212 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 0 | 1 |  |
| Storage Blk Time (\%) |  | 0 |  |  |  | 0 |  | 0 | 0 |  |

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Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 5 | 15 | 46 | 34 |
| Average Queue (ft) | 1 | 2 | 31 | 28 |
| 95th Queue (ft) | 5 | 14 | 61 | 48 |
| Link Distance (ft) | 212 | 468 | 427 | 240 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR |
| Maximum Queue (ft) | 8 | 50 | 39 |
| Average Queue (ft) | 1 | 24 | 23 |
| 95th Queue (ft) | 5 | 52 | 49 |
| Link Distance (ft) | 212 | 427 | 240 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 10 | 15 | 52 | 39 |
| Average Queue (ft) | 1 | 1 | 26 | 24 |
| 95th Queue (ft) | 5 | 6 | 55 | 49 |
| Link Distance (ft) | 212 | 468 | 427 | 240 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |

Queuing and Blocking Report

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 39 |
| Average Queue (ft) | 21 |
| 95th Queue (ft) | 50 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 48 |
| Average Queue (ft) | 19 |
| 95th Queue (ft) | 49 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 48 |
| Average Queue (ft) | 20 |
| 95th Queue (ft) | 49 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
|  |  |
| Network Summary |  |
| Network wide Queuing Penalty, Interval \#1: 1 |  |
| Network wide Queuing Penalty, Interval \#2: 1 |  |
| Network wide Queuing Penalty, All Intervals: 1 |  |

Queuing and Blocking Report

Intersection: 1: HWY 101 \& 35th Street, Interval \#1

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 64 | 107 | 74 | 59 | 97 | 143 | 122 | 64 | 196 | 192 |
| Average Queue (ft) | 42 | 59 | 44 | 30 | 60 | 94 | 74 | 34 | 124 | 116 |
| 95th Queue (ft) | 79 | 104 | 80 | 63 | 102 | 159 | 130 | 64 | 202 | 201 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage Blk Time (\%) |  | 1 |  |  |  | 1 |  | 0 | 15 |  |
| Queuing Penalty (veh) |  | 1 |  |  |  | 1 |  | 0 | 10 |  |

Intersection: 1: HWY 101 \& 35th Street, Interval \#2

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 110 | 115 | 79 | 74 | 129 | 177 | 189 | 113 | 191 | 199 |
| Average Queue (ft) | 46 | 54 | 32 | 30 | 48 | 88 | 73 | 37 | 98 | 85 |
| 95th Queue (ft) | 89 | 100 | 67 | 65 | 93 | 152 | 144 | 78 | 162 | 150 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 |  |  | 91 |  |  |
| Storage BIk Time (\%) | 0 | 1 |  |  | 0 | 1 |  | 0 | 10 |  |
| Queuing Penalty (veh) | 1 | 1 |  |  | 0 | 1 |  | 0 | 6 |  |

Intersection: 1: HWY 101 \& 35th Street, All Intervals

| Movement | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | T | TR | L | T | TR |
| Maximum Queue (ft) | 110 | 136 | 89 | 79 | 130 | 184 | 189 | 113 | 225 | 218 |
| Average Queue (ft) | 45 | 55 | 35 | 30 | 51 | 90 | 73 | 36 | 104 | 92 |
| 95th Queue (ft) | 87 | 101 | 71 | 65 | 96 | 154 | 141 | 75 | 174 | 166 |
| Link Distance (ft) |  | 456 |  | 211 |  | 894 | 894 |  | 661 | 661 |
| Upstream Blk Time (\%) |  |  |  |  |  |  |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  | 146 |  |  | 91 |  |  |
| Storage Bay Dist (ft) | 110 |  | 141 |  | 146 | 1 |  | 0 | 11 |  |
| Storage Blk Time (\%) | 0 | 1 |  |  | 0 | 1 |  | 0 | 7 |  |

Queuing and Blocking Report

Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#1

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 2 | 15 | 57 | 40 |
| Average Queue (ft) | 0 | 4 | 37 | 27 |
| 95th Queue (ft) | 0 | 20 | 61 | 52 |
| Link Distance (ft) | 211 | 201 | 427 | 216 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |

Intersection: 2: REDWOOD ST/Site \& 35th Street, Interval \#2

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 4 | 25 | 59 | 41 |
| Average Queue (ft) | 0 | 3 | 36 | 20 |
| 95th Queue (ft) | 2 | 16 | 61 | 48 |
| Link Distance (ft) | 211 | 201 | 427 | 216 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 2: REDWOOD ST/Site \& 35th Street, All Intervals

| Movement | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR | LTR |
| Maximum Queue (ft) | 5 | 25 | 76 | 46 |
| Average Queue (ft) | 0 | 3 | 36 | 21 |
| 95th Queue (ft) | 2 | 17 | 61 | 49 |
| Link Distance (ft) | 211 | 201 | 427 | 216 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Queuing and Blocking Report

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#1

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 35 |
| Average Queue (ft) | 21 |
| 95th Queue (ft) | 49 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, Interval \#2

| Movement | WB |
| :--- | ---: |
| Directions Served | R |
| Maximum Queue (ft) | 50 |
| Average Queue (ft) | 17 |
| 95th Queue (ft) | 48 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: HWY 101 \& RIRO SITE ACCESS, All Intervals

| Movement | WB |
| :--- | :---: |
| Directions Served | R |
| Maximum Queue (ft) | 50 |
| Average Queue (ft) | 18 |
| 95th Queue (ft) | 48 |
| Link Distance (ft) | 201 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
|  |  |
| Network Summary |  |
| Network wide Queuing Penalty, Interval \#1: 12 |  |
| Network wide Queuing Penalty, Interval \#2: 8 |  |
| Network wide Queuing Penalty, All Intervals: 9 |  |


[^0]:     ELUDING / ATTEMPT TO ELUD
    ARELESS DRIVING
    PEED RACING (PER PAR)
    ROSSING AT INTERSECTION OLLLOWING TOO CLOSELY (MUST BE ON OFFI
    STRADDLING OR DRIVING ON WRONG LANES
    IMPROPER CHANGE OF TRAFFIC LANES
    CODE DESCRIPTION FULL FAILED TO DECREASE SPEED FOR SLOWER MOVING VEHICLE
    
    DRIVING TOO FAST FOR CONDITIONS (NOT EXCEEDING POSTED SPEED) OPENED DOOR INTO ADJACENT TRAFFIC LAN

    IMPEDING TRAFFIC
    RECKLESS DRIVING (PER PAR)
    CROSSING AT INTERSECTION, NO TRAFFIC SIGNAL PRESENT
    CROSSING AT INTERSECTION, TRAFFIC SIGNAL PRESENT
    CROSSING BETWEEN INTERSECTIONS WALKING, RUNNING, RIDING, ETC., ON SHOULDER WITH TRAFFIC
    WALKING, RUNNING, RIDING, ETC., ON SHOULDER FACING
    PLAYING IN STREET OR ROAD
    WUSHING OR WORKING ON VEHICLE IN
    STANDING OR LYING IN ROADWAY BY NON-MOTORIST
    IMPROPER USE OF TRAFFIC LANE
    ELUDING / ATTEMPT TO ELUDE
    FAILED TO MAINTAIN LANE
    RAN OFF ROAD
    RRIVER MISJUDGED CLEARANCE
    OVER-CORRECTING

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