

# BURGER KING TRAFFIC IMPACT ANALYSIS Florence, Oregon

BEI Project 20-113

Prepared for: 2812 Architecture, LLC 2812 Colby Ave Everett, WA 98201





June 30, 2020

civil transportation structural geotechnical SURVEYING

www.BranchEngineering.com

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310 5th Street Springfield, OR 97477 p: 541.746.0637 f: 541.746.0389 Contents Page No.

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### 1.0 EXECUTIVE SUMMARY

#### 1.1 DESCRIPTION

The development site is located at the northeast quadrant of HWY 101 at 35<sup>th</sup> Street in Florence, Oregon. The site is located on tax lot 06800 of assessor's map 18-12-23-22. The site is currently undeveloped and vacant. The site is planned to be developed with a 2,992SF fast food restaurant with drive-through window. The subject site is zoned and designated for highway commercial land uses that supports the proposed use. Construction is expected to begin late in 2020 and the site is expected to be open for the second quarter in 2021.

### 1.2 TRAFFIC

The proposed development is projected to generate 120 AM and 98 PM peak hour automobile trips accessing the site at the street frontages at HWY 101 and at 35<sup>th</sup> Street. On average, 58 of the AM trips and 48 of the PM trips are expected to be pass-by trips already on the public roadway system. Traffic impacts are evaluated for full development build-out for the years 2021 and 2026 during the AM and PM peak hours.

## 1.3 VEHICLE SAFETY, ACCESS

Crash history is evaluated for the intersection of 35<sup>th</sup> Street and HWY 101. The crash analysis did not identify a significantly high intersection crash frequency or unusual crash pattern occurring at the intersection of 35<sup>th</sup> St. and HWY 101. Driveway queuing and onsite vehicle and pedestrian circulation are evaluated to determine the operational safety of the site's proposed access configuration.

The site currently has a grant of access for one driveway approach on HWY 101 that includes provisions for restricting the operation of the access to right-in and right-out only turning movements. As planned, development and operation at the site will include utilizing the access entitlement to HWY 101, constructing a pork chop to comply with the permitted operation of the approach, and constructing an unrestricted access connection at the 35<sup>th</sup> Street frontage. The access at the 35<sup>th</sup> Street frontage will provide primary access via the traffic signal at 35<sup>th</sup> Street and HWY 101.

#### 1.4 RESULTS

In summary, the proposed development will not result in any identifiable significant impacts to the performance or operation of background traffic conditions with the added traffic from the proposed development during the anticipated year of opening 2021 peak hour conditions nor the projected year 2026 horizon year peak hour conditions.

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# 2.0 INTRODUCTION

Branch Engineering Inc. has been retained to evaluate potential traffic impacts associated with the proposed site development. This report strives to meet the City of Florence Traffic Impact Study requirements as presented in a scoping memorandum from March 2020, which is included as Appendix A. Included in the analysis is a summary of existing and proposed transportation infrastructure, documentation of observed traffic volumes, projected post development 'Build' traffic volumes, an analysis of crashes at study area street intersections, an analysis of City of Florence access management code, an intersection performance evaluation for the studied intersections, and a queuing and blocking analysis at site driveways and the studied public street intersection at HWY 101 and 35th Street. A site plan is included in Appendix B.

The following is a list of analysis year scenarios analyzed in this report:

- ❖ Year 2020 'Existing' Traffic Volumes (AM and PM Design Peak Hours);
- ❖ Year 2021 'Background' Traffic Volumes (AM and PM Design Peak Hours);
- ❖ Year 2021 'Build' Traffic Volumes (AM and PM Design Peak Hours);
- Year 2026 'Background' Traffic Volumes (AM and PM Design Peak Hours), and;
- ❖ Year 2026 'Build' Traffic Volumes (AM and PM Design Peak Hours).

The aerial below illustrates the location of the proposed development site.



Figure 1: Site Location (Aerial Photography from Lane County Maps)

## 3.0 EXISTING CONDITIONS

### 3.1 ROADWAY NETWORK

The relevant roadways for this study are:

- ❖ 35<sup>th</sup> Street;
- Redwood Street, and;
- ❖ HWY 101.

The following table shows characteristics of the study area roadways in the vicinity of the site:

Table 1: Roadway Network										
Roadway	Classifica tion	Travel Lanes	Sidewalks	Bike Lanes	On-Street parking	Posted Speed				
HWY 101	Major/ Principal Arterial		Yes	Yes	No	40МРН				
35 <sup>th</sup> Street	Minor Arterial <sup>2</sup>	2	Yes	Yes	No	25МРН				
Redwood Street	Local	2	Yes³	Yes	No	25МРН				

<sup>&</sup>lt;sup>1</sup> HWY 101 is developed with a continuous center two-way left-turn lane (CTWLTL)

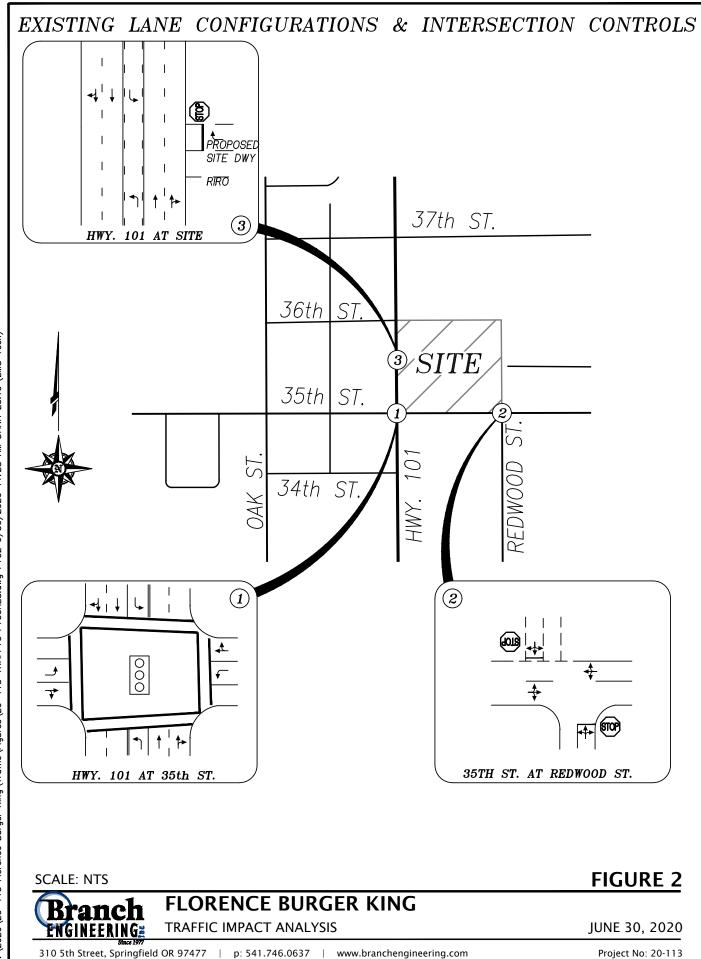
HWY 101 is within ODOT's jurisdiction and is maintained by ODOT. 35<sup>th</sup> street is within the City of Florence's jurisdiction and is maintained by the City. The intersection of 35<sup>th</sup> Street and HWY 101 is fully controlled by a traffic signal that is maintained by ODOT. The traffic signal features protected and permissive left-turn phasing with dedicated left turn lane approaches developed from a center two-way left-turn lane that is present on northbound and southbound approaches to 35<sup>th</sup> Street. The eastbound and westbound side street approaches on 35<sup>th</sup> Street to HWY 101 feature permissive left turn phasing and are each developed with dedicated left-turn lanes for storage.

The intersection at Redwood and  $35^{th}$  Streets is a three-way stopped controlled ('tee") intersection, with the northbound Redwood Street approach to  $35^{th}$  Street stopped and the approaches on  $35^{th}$  Street free movements.

Figure 2 shows the lane configurations and intersection controls of the study area intersections and roadways.

 $<sup>^2</sup>$  The 2012 City of Florence TSP identifies  $35^{\text{th}}$  Street as a Minor Arterial, while ODOT maps identify it as a Major/Urban Collector

<sup>&</sup>lt;sup>3</sup> Sidewalk present on east side only



Z:\2020\20-113 Florence Burger King\Traffic\Figures\20-113 TRAFFIC FIGURES.dwg FIG2 6/30/2020 11:28 AM DANH 23.1s (LMS Tech)

### 3.2 NON-MOTORIZED FACILITIES AND TRANSIT

The site frontages include adjacent curbside sidewalks to accommodate pedestrian traffic and striped bike lanes to accommodate bicycles. Rhody Express is the local transit service and is a subsidiary of Lane Transit District. Rhody Express makes two route loops in Florence, with one north loop and one south loop that meet along 21st Street. The north loop travels north and south on Spruce Street between 21st and 42nd Streets with a slight detour to Redwood Street between 32nd and 35th Streets. At 42nd Street, the north loop travels to HWY 101, then via HWY 101 to Bi Mart and then to Fred Meyer before turning around and traveling south back to 21st Street. The Rhody express operates on a 60-minute circuit around the loop Tuesday through Friday from 10:00AM through 6:00PM. The routes (north and south loops) provide public transportation to key parts of Florence, including retail centers, schools, food share, the Old Town district, the hospital and several city parks. The availability of transit, pedestrian and bicycle facilities can help to reduce the reliance on single occupant motorized vehicles. A Rhody Express route schedule is included as Appendix C.

### 3.3 INTERSECTIONS

With City concurrence, the following intersections are evaluated with background and proposed post-development traffic scenarios:

- ❖ 35<sup>th</sup> Street at HWY 101;
- ❖ HWY 101 at proposed site right-in right-out only driveway, and;
- 35th Street at proposed driveway access and Redwood Street (treated as one intersection).

The intersection of 35<sup>th</sup> Street at Redwood Street is currently a three-approach ("tee") intersection. The proposed driveway approach on 35<sup>th</sup> Street would add a southbound approach and a northbound receiving lane to the site, and would operate effectively as a fourth leg to the intersection at 35<sup>th</sup> and Redwood Street. The southbound site driveway approach alignment to 35<sup>th</sup> Street would be offset from the northbound Redwood Street approach alignment by approximately 35 feet at the proposed centerline of the site.

### 3.4 CRASHES

An analysis of crash history is performed for the public street intersections at HWY 101 and 35<sup>th</sup> Street and 35<sup>th</sup> Street at Redwood Street. Detailed crash data reports were downloaded from the Oregon Department of Transportation's (ODOT's) crash data system online for the most recent five-years of available data, which at the time of this report is the period from January 1, 2014 through December 31, 2018. Crash data reports are available from ODOT's Crash Analysis and Reporting Unit web tool at: <a href="https://www.oregon.gov/odot/Data/Pages/Crash.aspx">https://www.oregon.gov/odot/Data/Pages/Crash.aspx</a>. The current crash reporting methodology utilized by ODOT accounts for crashes only if an injury occurred or if a loss in property value of \$1,500 or more occurred as a result of the crash. There were no crashes reported to have occurred at the intersection of 35<sup>th</sup> Street and Redwood Street. The following table illustrates the overall five-year crash history and frequency for the intersection at 35<sup>th</sup> Street and HWY 101.

Т	Table 2: Intersection Crashes at HWY 101 and 35 <sup>th</sup> Street										
Year	AADT Volume	Head- On	Ped	Angle/ SS	Turn Mvmts	Rear End	Fixed Object/Other	Total Crashes	Crash Rate <sup>1</sup>	Critical Rate <sup>2</sup>	
2014	16,000	0	0	0	2	0	0	2	0.07	0.62	
2015	16,000	0	0	0	0	0	$1^3$	1	0.17	0.62	
2016	16,000	0	0	1	2	0	0	3	0.51	0.62	
2017	16,000	0	0	0	1	0	1	2	0.07	0.62	
2018	16,000	0	0	2	2	0	0	4	0.68	0.62	
Totals	16,000	0	0	3	7	0	2	12	0.41	0.62	

1. Crash Rate (Crashes per Million Entering Vehicles = CPMEV) = (total 5-year crashes x 1,000,000)/(5 x ADT x 365)

While the year 2018 annual crash rate was greater than the referenced critical five-year average crash rate of 0.68 crashes per million entering vehicles for comparable signalized four-way urban intersections, the cumulative average of 0.41 crashes per million entering vehicles over the five-year analysis period was below the critical rate of 0.62 CPMEV, which is the standard applicable criteria. The crash frequency and types of crashes that are reported to have occurred are not considered abnormal and no additional crash reducing measures are necessary.

ODOT Crash Data System intersection crash data reports are included as Appendix D.

### 3.5 EXISTING TRAFFIC VOLUMES

AM and PM peak hour traffic volumes were collected by manual traffic counts conducted by Gary's Traffic Data and Branch Engineering in March 2020, and are included as Appendix E. The turning movement traffic counts at HWY 101 and 35<sup>th</sup> Street were collected between 7:00AM and 9:00AM and between 4:00PM and 6:00PM on a typical weekday (Tuesday-Thursday, non-holiday week), which was Thursday March 19, 2020. Peak hours extrapolated from the collected traffic count volumes were determined to occur between 8:00 AM and 9:00 AM and between 4:30 PM and 5:30 PM. Using the peak hour collected traffic count data for HWY 101 at 35<sup>th</sup> Street are balanced through for design hour conditions to the proposed driveway intersection on HWY 101. The observed traffic volumes are provided on Figure 3 on page 9.

### 3.6 SPECIAL TRANSPORTATION TREND ADJUSTMENT

Because traffic count data was collected during the week prior to the Oregon Governor's Stay Home Stay Safe (SHSS) guidelines that were implemented on March 23<sup>rd</sup>, 2020 due to concerns about spreading the Corona Virus/COVID-19, an adjustment factor is necessary to adjust collected traffic count data to account for reduced approaching traffic volume and trip making trends on the nearby transportation system. During the weeks leading to the SHSS guidelines, traffic had been declining, and public school was first temporary closed starting the week of March 16<sup>th</sup>, which was the week traffic count data was collected for the subject site and this traffic impact analysis. The Oregon Department of Transportation has been keeping track of traffic trends on statewide highways where there are Automated Traffic Recorder (ATR) Stations installed that continuously collect and record traffic volume data in real time. ODOT Weekly COVID-19 Traffic Trend Reports are available online at: <a href="https://www.oregon.gov/odot/Data/Pages/Traffic-Counting.aspx">https://www.oregon.gov/odot/Data/Pages/Traffic-Counting.aspx</a>. For the

<sup>2.</sup> Critical rate per Highway Safety Manual (HSM) methodology 3. One reported crash was the result of a backing maneuver

week immediately prior to the Governor's SHSS order, the data is available by clicking the link for April 10, 2020 – Observed Statewide Traffic Volume Patterns, which will take the user to collected data for the years 2019 and 2020 corresponding to the week that that traffic count data was collected for the site. Per the statewide data, the overall average daily traffic trend for the week of March 16th, 2020 was approximately 19 percent lower than 2019 traffic volume data for the ATR stations, therefore; an adjustment factor of 1.19 is assumed appropriate for adjusting collected traffic count data to simulate the effects of COVID-19 on transportation trends that were already occurring the week prior to the Governor's SHSS guidelines. The Oregon Department of Transportation's April 10, 2020 Traffic Report comparing year 2019 and 2020 traffic data collected at ATR stations during the week of March 16th, 2020 that data was collected for the site, is included in Appendix E with the collected traffic count data.

## 3.7 SEASONAL ADJUSTMENT

Typically, peak hour traffic data collected for interior City streets within urbanized areas do not require seasonal adjustments to represent design hourly approaching traffic volumes (i.e. 30th highest hour conditions), however, since the application of the Special Transportation Trend Adjustment Factor discussed and developed in the previous section is based on Statewide highways where there is seasonal fluctuation, and for the purpose of providing a conservative analysis herein, the collected year 2020 turning movement traffic count data with adjustments for the effects of COVID-19 are factored again by a calculated seasonal adjustment factor. The seasonal adjustment factor is calculated by using the most recent available ODOT Seasonal Trend Table (2018) available online at the ODOT Analysis Procedures Manual website (https://www.oregon.gov/odot/Planning/Pages/APM.aspx) with the coastal destination traffic trend assumed applicable for each approach/movement and by applying linear interpolation to the count date from tabulated values for the corresponding dates of those values. The adjustment methodology is consistent with the methodology outlined in the current edition of the Oregon Department of Transportation's (ODOT's) Analysis Procedures Manual (APM) at the link above. Tabulated seasonal trend values contained within the ODOT Seasonal Trend Table are available for the 1<sup>st</sup> and the 15<sup>th</sup> days of each month throughout the year. Per the 2018 Seasonal Trend Table, the coastal destination trend's peak seasonal period is represented during the period from July 1st to July 15th with the Seasonal Trend Peak Period Factor of 0.8074. For approaching traffic data collected on the March 19th, 2020 count date, the seasonal trend is linearly interpolated for four days for tabulated values of between 1.04732 and 1.0504 corresponding to dates of March 15th and April 1st. The interpolated seasonal trend value is 1.048 for March 19th, and the calculated seasonal adjustment factors applied to the turning movement traffic count data is 1.298.

Design Hour year 2020 traffic volumes with the product of the Special Transportation Trend and the calculated Seasonal Adjustment Factors applied are provided on Figure 4 on page 10.

### 3.8 PIPELINE TRAFFIC VOLUMES

In addition to existing roadway traffic volumes, pending local area land use development projects in construction that have been approved by the City of Florence but not yet issued occupancy may add traffic to the study area intersections and travel routes that would not be accounted for when traffic count data was collected. Motor vehicle trips that are included in a traffic impact analysis that has been approved, but not yet subject to post-development occupancy level day-to-day design hour trip generation conditions are known as pipeline trips. At the time of this traffic study, one approved traffic impact analyses for a residential subdivision located north of 35th Street on

Rhododendron Drive northwest of the site with 31 single family residential detached housing units and 101 apartment units was approved but not built-out yet. The City of Florence furnished the approved traffic study for said development, which will add left-turns at the northbound and eastbound approaches to HWY 101 at 35<sup>th</sup> Street and eastbound right-turns during design hour traffic conditions by the end of the year 2021. Figure 5 on page 11 shows the pipeline trips in the study area.

### 3.9 TRAFFIC GROWTH AND FUTURE TRAFFIC

The City of Florence codified Traffic Impact Analysis requirements do not contain analysis details pertaining to analysis years that need to be included in a TIA. Per the Oregon Department of Transportation's (ODOT's) development review guidelines, Table 3.3, proposed developments that that generate between 1,000 and 2,999 average daily trips (ADT) and include a single-phase year of opening are suggested to include the year of opening and a five-year post-development planning horizon year in the analysis scenarios. Although ODOT does not require an analysis for this proposed development, the analysis required by the City of Florence is proposed to include the year of opening (anticipated to be 2021), and a five-year post-development horizon year, 2026, consistent with the suggested analysis year scenarios by ODOT.

To grow year 2020 design hour traffic volumes displayed on Figure 4 to represent year 2021 and year 2026 background design hour conditions, an average annual growth rate (AAGR) is calculated from the ODOT Future Volume Tables (FVT). The ODOT FVT provides base year 2018 and future year 2038 traffic volume data based on collected annual average daily traffic volumes for the base year and statewide traffic growth models that account for land use, transportation and employment trends associated with projected population growth and planned transportation improvements. The FVT for HWY 101 near the study area includes three sites on HWY 101 (ODOT HWY #9). The following table provides the local area FVT tabulated values for three sites near the subject development site.

Table 3: ODOT Future Volumes Table Data (AADT)									
Site ID	HWY	Description	2018	2038	RSQ				
1170	9	0.03 Mile South Munsel Lake Rd	9500	9600	0.2347				
1171	9	0.02 Mile South of 36th Street	12500	12600	0.4298				
1172	9	0.02 Mile South of 29th Street	14100	14200	0.805				

As shown in the table, one site has an RSQ ( $R^2$ ) value greater than 0.75 that generally indicates that the data is reliable and usable. The usable data was collected at site 1172, which is located approximately 0.375 mile south of the intersection at 35<sup>th</sup> Street and HWY 101. Using the data for site 1172, the average annual growth rate (AAGR) calculation over the 20-year period between the 2018 base year and the 2038 future year is as follows:

$$AAGR = (14,200 - 14,100)/(14,100 \times 20) = 0.0355\%/year$$

The AAGR is then factored for one year and six years to project year 2021 year of opening background traffic volumes and year 2026 five-year post development horizon design hour background traffic volumes. The AAGR and growth rate factors are calculated and applied using

the linear growth methodology discussed in the current ODOT Analysis Procedures Manual (APM), Section 6.4.

Pipeline trips discussed in section 3.8, previously, are expected to be added to projected background growth calculated in the above steps by the year 2021 analysis year, therefore, background traffic volumes for the year 2021 and 2026 design hours will include growth of 2020 design hour traffic and pipeline trips. Background traffic volumes for the 2021 design hours are provided on Figure 6 on page 13. Year 2026 background traffic volumes are provided on Figure 7 on page 14.

2: \2020\20-113 Florence Burger King\Traffic\Figures\20-113 TRAFFIC FIGURES.dwg FIG3 6\30/2020 11:28 AM DANH 23.1s (LMS Tech)

= AM PEAK HOUR (xx) = PM PEAK HOUR

FIGURE 3 SCALE: NTS



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\*BALANCED TO 35TH AND HWY IOI

IN & OUT FLOWS

= AM PEAK HOUR (xx) = PM PEAK HOUR

\*BALANCED TO 35TH AND HWY IOI

IN & OUT FLOWS

FIGURE 4 SCALE: NTS



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TRAFFIC IMPACT ANALYSIS ANALYSIS

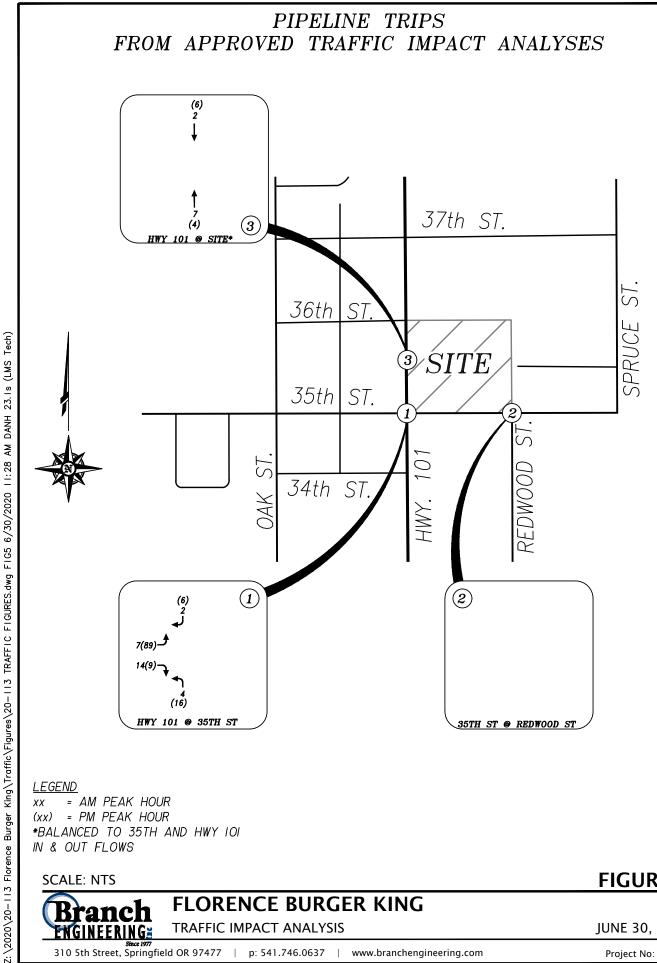
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2: \2020\20-113 Florence Burger King\Traffic\Figures\20-113 TRAFFIC FIGURES.dwg FIG4 6\30/2020 11:28 AM DANH 23.1s (LMS Tech)

<u>LEGEND</u>



= AM PEAK HOUR (xx) = PM PEAK HOUR

\*BALANCED TO 35TH AND HWY IOI

FIGURE 5 SCALE: NTS



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IN & OUT FLOWS

= AM PEAK HOUR (xx) = PM PEAK HOUR

\*BALANCED TO 35TH AND HWY IOI

IN & OUT FLOWS

FIGURE 6 SCALE: NTS



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2: \2020\20-113 Florence Burger King\Traffic\Figures\20-113 TRAFFIC FIGURES.dwg FIG6 6\30/2020 11:28 AM DANH 23.1s (LMS Tech)

= AM PEAK HOUR (xx) = PM PEAK HOUR

\*BALANCED TO 35TH AND HWY IOI

IN & OUT FLOWS

FIGURE 7 SCALE: NTS



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2: \2020\20-113 Florence Burger King\Traffic\Figures\20-113 TRAFFIC FIGURES.dwg FIG7 6\30/2020 11:28 AM DANH 23.1s (LMS Tech)

## 4.0 DEVELOPMENT CONDITIONS

### 4.1 LAND USE

The potential developer entity is seeking land use approval for development of a 2.992 KSF GFA single occupant fast food restaurant with a drive-through window on the site. As planned and shown on the current site plan, the remaining land area in the area of disturbance abutting 35<sup>th</sup> Street and approximately mid-block between 35<sup>th</sup> Street and the extension of the right-of-way of 36<sup>th</sup> Street will be utilized for parking and drive-through facilities to serve the proposed use. The drive through facilities will feature two parallel ordering kiosks that will allow two entrance lanes that will merge downstream into a single drive-through lane. The dual kiosks will allow approximately six vehicles to queue in three vehicle lengths from the entrance to the merge point in the drive-through lane downstream of the kiosks. As discussed in the introduction, a site plan is included as Appendix B.

### 4.2 SITE ACCESS

The site has frontage on HWY 101 between 35<sup>th</sup> Street and the projected extension of the centerline of 36<sup>th</sup> Street across the north subject property line. The HWY 101 frontage has an existing curb cut at approximately mid-block between 35<sup>th</sup> and 36<sup>th</sup> Streets, where there was previously a right-of-way provided in the 1891 Frasier and Berry's Subdivision Plat between lots 1-10 and lots 11-20 of Block 10. This right-of-way was vacated within the last couple of decades, and the approximate location of the existing curb cut is planned to be improved and utilized with right-in right-out access controls between the site and HWY 101. Early correspondence with ODOT indicated that an access permit to occupy at this location will be supported by ODOT, given the proposed configuration and that all the applicant will need to do is submit for a driveway approach permit or for a construction permit for work within the right-of-way of HWY 101.

The site entitlements currently include a crossover access easement to the existing driveway approach at the adjacent site to the north, which is currently an unrestricted approach. The site plan does not currently show a connection to this existing driveway, which is aligned with the centerline of 36<sup>th</sup> Street.

### 4.3 35<sup>™</sup> STREET ACCESS

The site plan includes a proposed 25-foot wide unrestricted access connection on the site's 35<sup>th</sup> Street frontage that is shown as a fourth leg (dog-leg) to the existing three-way intersection at Redwood Street. The proposed driveway centerline shown on the site plan is located approximately 35 feet west of the existing centerline of Redwood Street. Redwood Street's centerline is aligned approximately with the existing eastern property line at the site. The placement of this access is shown on the site plan to maximize the spacing from the stop bar at the westbound 35<sup>th</sup> Street approach to HWY 101, where the designer presumably is concerned with impacting the existing striped queue storage and driveway operation within close proximity to the intersection of HWY 101 at 35<sup>th</sup> Street. The City of Florence Code requires a minimum of 25 feet of spacing distance between access and intersections (Title 8, section 8-2-3-4(D)), but it is not clear from this section of the code if the code is applicable to the centerlines or the nearside of the edges of the approach throats when offsetting driveway and public street driveway approaches are across the street from

one another. The subdivision code, Title 10, Chapter 35, also discusses approach spacing in section 10-35-2-7, where it says the minimum separation of accesses and public street intersections on the same side of the street shall be 30 feet between the near side surfacing of the approach and the near right-of way of collector streets. Title 10-35-2-7 also includes a minimum offset of 50 feet of separation between near side edge of surfacing of the approach and the right-of way of areterial streets. The proposed access location offset from the intersection at Redwood Street shown on the site plan does not involve conflicting left turns, since left-turns are allowed to be executed in front of one another approaching the driveway from the west and approaching Redwood Street from the east. The approach also leads to an access easement, which is presumably for future development of parcels to the north that are not currently proposed to be developed.

### 4.4 TRIP GENERATION

To project trip generation for the proposed post development traffic conditions, references were made to the <u>Trip Generation Manual</u>, 10<sup>th</sup> <u>Edition</u>, and the <u>Trip Generation Handbook</u>, 3<sup>rd</sup> <u>Edition</u>, both published by the Institute of Transportation Engineers (ITE). The table on the following page summarizes the projected peak hour site generated traffic based on the available published data.

### 4.5 PASS-BY TRIPS

The development site's type of land use is expected generate a significant amount of pass-by and diverted linked traffic during AM and PM peak hour trip generation periods, since most motorists do not make exclusive or long trips across town, passing similar competing markets on the way to make fast food restaurant purchases, but rather visit these types of establishments on the way to or from a primary origination/destination in the vicinity. The presence of competing markets nearby will have a significant effect on the distance motorists are willing to travel to make a trip to the site, since the HWY 101 corridor is developed with a significant density of competing uses similar to the site's proposed use.

According to the ITE <u>Trip Generation Handbook</u>, 3<sup>rd</sup> <u>Edition</u>, "Pass-by trips are trips that are made as intermediate stops on the way from an origin to a destination without a route diversion. Pass-by trips are attracted from traffic passing the site on an adjacent street or roadway that offers direct access to the trip generator. Pass-by trips are not diverted from another roadway." The <u>Trip Generation Handbook</u> contains the following average pass-by trip rate percentages for the proposed use as a fast food restaurant with drive through window: 49 percent of AM and 50 percent of PM trip generation during the peak hours of adjacent street traffic.

Based on the site's proposed access configuration, most northbound pass-by trips by familiar drivers on HWY 101 would be expected to enter the site via a right-turn at 35<sup>th</sup> Street to access the site driveway, traverse the drive-through, then exit the site at the proposed RIRO driveway, which will be similar to a diverted linked trip. Similarly, southbound pass-by trips are likely to be disguised as diverted linked trips, that will enter the site via a left-turn from HWY 101 at 35<sup>th</sup> Street to access the driveway, traverse the drive-through and re-enter southbound traffic on HWY 101 as left-turns from 35<sup>th</sup> Street's westbound approach.

The trip generation potential for the development of the site is illustrated in the following table.

Table 4: Site Trip Generation										
Land Use/ ITE Land Use Code	Ind. Variable	Units (QTY)			Trips IN (%/#)	Trips OUT (%/#)				
AM Peak Hour										
Fast Food Restaurant with Drive-Through Window/934	KSF GFA	2.992	40.19	120	51%/61	49%/59				
	Pa	ass-By Trips:	49%	58*	29	29				
	ı	PM Peak Hou	r							
Fast Food Restaurant with Drive-Through Window/934	KSF GFA	2.992	32.67	98	52%/51	48%/47				
	P	ass-By Trips:	50%	48*	24	24				
		ADT								
Fast Food Restaurant with Drive-Through Window/934	KSF GFA	2.992	470.95	1,409	50%/705	50%/704				
*AM and PM Pass-by trips (59 and 49) are rounded down to nearest even number for 50% even directional split in/out										

As indicated above, the proposed post-development traffic conditions could result in an average of 120 AM peak hour vehicle trips and 98 PM peak hour vehicle trips accessing the site. Out of the total peak hour vehicle trips, 49 percent of the AM and 50 percent of the PM peak hour trips are expected to be pass-by trips, which are trips already on the roadway that would be expected to make a route diversion at the site driveway to enter and exit the site, but will otherwise continue in the same direction they were traveling upon leaving the site between primary trip origins and destinations. Per City of Florence Title 10, Chapter 1, Section 10-1-1-4(E), any development that generates 250 or more average daily trips is required to provide a traffic impact analysis. Because the proposed development will generate 1,409 average daily trips during typical weekday, the proposed development requires a traffic impact analysis.

### 4.6 TRIP DISTRIBUTION AND ASSIGNMENT

To project the proposed development's trip distribution, references were made to the ODOT Future Volumes Table (FVT) for HWY 101 locations, projected design hour traffic volumes, recent traffic impact analyses and the location of the site with respect to local land use patterns (trip origin and destination pairs). The ODOT FVT includes daily traffic volumes that were most recently collected in 2018 at a location 0.02 miles south of the intersection at 36<sup>th</sup> Street that is north of the site and has the highest AADT reported (12,500) between the site and the north UGB on HWY 101. A second location south of the site located approximately 0.02 mile south of the intersection at 17<sup>th</sup> and HWY 101 had an AADT of 16,400, and was the highest ADT reported for the corridor between the south UGB and the site. Using these data and the approaching traffic volumes on east and west approaches to the intersection at HWY 101 and 35<sup>th</sup> Street, and the relative distribution of residential land uses near the site, the expected primary non-pass-by trip assignment is anticipated to be split with the following assignments:

- ❖ 35% of primary trip origins and destinations north via HWY 101;
- ❖ 45% of primary trip origins and destinations south (east and west) via HWY 101;

- ❖ 15% of primary trip origins and destinations west via 35<sup>th</sup> Street, and:
- ❖ 5% of primary trip origins and destinations south and east via Redwood Street and Spruce Streets.

Anticipated pass-by (non-primary) trip assignments are based on AM and PM design hour approaching traffic volumes, which are generally evenly split between northbound and southbound directions at the site frontage during AM and PM design hour conditions. The distribution of pass-by trips is therefore evenly split, with approximately 50 percent of pass-by trips coming from and destined to each the northbound and southbound direction at the site frontage. As described previously, the site's access configuration will result in a significant number of would-be pass-by trips at the site's driveway on HWY 101 being redistributed from through movements to turning movements at the intersection at 35th Street and HWY 101 because the site driveway is limited to right-in and right-out only turning movements and will be located at the northern extent of the site. The arrangement of access on the site places the RIRO approach on HWY 101 north of the drive-through window entrance and parking area, with the site located on the east side of the highway. The approaching traffic volumes on 35th Street are not likely to contribute significantly to the pass-by trip making phenomenon, and it is assumed that there will not be a significant number of pass-by trips out of 35th Street's approaching traffic volume.

An illustration of the site's peak hour site generated traffic distribution is provided on the following page as Figure 8.

To analyze the post-development build scenario traffic conditions in the design hours of the analysis years, the site generated traffic was added to the year 2021 and 2026 background traffic volumes. These "build" total traffic volumes are provided on Figures 9 and 10 on pages 20 and 21.

PEAK HOUR SITE GENERATED

AM PEAK HOUR (xx) = PM PEAK HOUR

-(-) = AM & (PM) PASS-BY TRIPS

PEAK	PRIMARY TRIPS		PASS TR	S-BY IPS	TOTAL		
HOUR	IN	OUT	IN	OUT	IN	OUT	
AM	32	30	29	29	61	59	
PM	27	23	24	24	51	47	

FIGURE 8 SCALE: NTS



# **FLORENCE BURGER KING**

TRAFFIC IMPACT ANALYSIS

JUNE 30, 2020

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= AM PEAK HOUR (xx) = PM PEAK HOUR

FIGURE 9 SCALE: NTS



# FLORENC BURGER KING

TRAFFIC IMPACT ANALYSIS

JUNE 30, 2020

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### 4.7 INTERSECTION PERFORMANCE

Study area intersections are evaluated to determine Level of Service (LOS) based on average vehicle delay. Calculations for the signalized and unsignalized intersections are performed with the computer program SYNCHRO 10, by Trafficware, which utilizes selectable Highway Capacity Manual (HCM) methodologies to analyze intersection and approach lane performance characteristics. Per the Oregon Department of Transportation Analysis Procedures Manual's (APM's) recommended analysis procedure, the HCM 2000 methodology is utilized to analyze signalized intersections and the HCM 2010 methodology is utilized to analyze unsignalized intersection and approach performances.

Level of service is classified by a letter scale from 'A' to 'F'. LOS 'A' represents optimum operating conditions and minimal delay. LOS 'F' indicates over capacity conditions causing unacceptable delay. The City of Florence's current transportation system plan (TSP) identifies with LOS 'D' at all signalized and all-way stop controlled intersections if the V/C ratio is not higher than 1.0 for the facility. The City of Florence standard for unsignalized two-way stop-controlled intersections is LOS 'E' at the critical movement (poorest performing approach). Mitigation measures may be necessary to remedy performance level conditions when level(s) of service fall(s) below the codified City standard or when the ODOT V/C performance standard is not met and the condition is identified to be the result of development impacts.

The LOS determined by average delay per vehicle as established in the Highway Capacity Manual, 2000 and HCM, 2010 are as follows:

Table 5: HCM Level of Service Criteria										
Level of Service	Unsignalized – HCM 2010	Signalized – HCM 2000								
A	< 10 sec	< 10 sec								
В	>10 and ≤ 15 sec	>10 and ≤ 20 sec								
С	$>15$ and $\leq 25$ sec	>20 and ≤ 35 sec								
D	>25 and ≤ 35 sec	>35 and ≤ 55 sec								
Е	$>$ 35 and $\leq$ 50 sec	>55 and ≤ 80 sec								
F	> 50 sec	> 80 sec								

Another measure of performance and congestion is volume to capacity ratio (v/c), which can indicate the level of demand as a proportion of the roadway's theoretical capacity. As described previously, ODOT does not require a formal traffic impact analysis for the proposed development, but HWY 101 is an ODOT facility and ODOT performance standards apply to their facilities. The ODOT mobility performance standard "target" that would be applicable to a statewide highway with a speed limit of 40 MPH, not designated a freight route and within an urban growth boundary of a non MPO area, is to maintain a not-to exceed v/c of 0.85 for the entire intersection at signalized intersections or state highway approaches to unsignalized intersections. In order to maintain safe operation, non-state highway approaches at unsignalized intersections are expected to meet or not to exceed the v/c of 0.90, based on the same area and speed characteristics, applicable for District/Local interest roads. The v/c of 0.90 is applicable to stop controlled driveway approaches to the state highway. The basic peak hour performance standard for evaluation of intersection performance at HWY 101 at  $35^{th}$  Street would be the not-to-exceed v/c ratio of 0.85 based on the highway designation.

To analyze the study area intersections, the intersection peak hour factors calculated from collected traffic count data are utilized for all existing year design hour conditions and for future build and background/no-build traffic conditions, as shown on the figures representing traffic volumes. For driveway approaches that do not currently serve any inflow or outflow traffic (background/no-build conditions) peak hour factors from the approaches at the intersection of HWY 101 and 35th Street are calculated from balancing the approaching traffic stream and utilized in the build conditions, with appropriate adjustments made consistent with the Oregon Department of Transportation's (ODOT's) Analysis Procedures Manual for future conditions (where applicable).

Per the Analysis Procedures Manual, the default saturation flow rate of 1750 pchpl is utilized in the SYNCHRO analysis software program models for all background and build traffic analysis scenarios, since the site is within the Florence Urban Growth Boundary, which is not identified as an MPO area. Saturation flow rate is defined by the ODOT Analysis Procedures Manual (APM) as the maximum departure (queue discharge) flow rate achieved by vehicles departing from the queue during the green period at traffic signals.

Results of the intersection performance analysis calculations are documented in Appendices G and H and are summarized in the following table:

i			_	_	-						
Table (	Table 6: Intersection Performance Analysis										
Intersection	Performance Standard <sup>(1)</sup>	2020 Existing Conditions	2021 No- Build	2021 Build	2026 No- Build	2026 Build					
		AM Peak Hou	ır								
HWY 101 at 35 <sup>th</sup> Street	LOS D/ v/c 0.85	LOS A/ v/c 0.41	LOS A/ v/c 0.43	LOS A/ v/c 0.43	LOS A/ v/c 0.43	LOS A/ v/c 0.43					
35 <sup>th</sup> Street at Redwood Street/Site Dwy <sup>(2)</sup>	LOS E	LOS A	LOS A	LOS B	LOS A	LOS B					
HWY 101 at Site Dwy <sup>(2)</sup>	LOS E/ v/c 0.85	LOS A <sup>(3)</sup>	LOS A <sup>(3)</sup>	LOS B/ v/c 0.04	LOS A <sup>(3)</sup>	LOS B/ v/c 0.04					
		PM Peak Hou	r								
HWY 101 at 35 <sup>th</sup> Street	LOS D/ v/c 0.85	LOS B/ v/c 0.50	LOS B/ v/c 0.52	LOS B/ v/c 0.52	LOS B/ v/c 0.52	LOS B/ v/c 0.52					
35 <sup>th</sup> Street at Redwood Street/Site Dwy <sup>(2)</sup>	LOS E	LOS B	LOS B	LOS B	LOS B	LOS B					
HWY 101 at Site Dwy <sup>(2)</sup>	LOS E/ v/c 0.85	LOS A <sup>(3)</sup>	LOS A <sup>(3)</sup>	LOS A/ v/c 0.04	LOS A <sup>(3)</sup>	LOS B/ v/c 0.04					
<sup>(1)</sup> LOS = Level Of Service <sup>(2)</sup> LOS & v/c at stop controlled approach <sup>(3)</sup> HCM 2010 analysis methodology of unsignalized intersections (driveways) that are not serving traffic do not											

<sup>&</sup>lt;sup>(3)</sup>HCM 2010 analysis methodology of unsignalized intersections (driveways) that are not serving traffic do not report a v/c for free movements or stop controlled approaches

As shown in the table, under the build scenario traffic conditions, the study area street and site driveway intersections are projected to continue to operate within the identified City of Florence and Oregon Department of Transportation mobility standards for level of service and v/c. Background/No-Build and Build HCM 2000 performance reports for the signalized intersection at HWY 101 and 35<sup>th</sup> Street and HCM 2010 performance reports for the unsignalized driveway intersections are included as Appendices G and H.

## 4.8 VEHICLE QUEUING ANALYSIS

In addition to mobility performance calculations, vehicle queue length calculations are performed for the public street intersections at HWY 101 and 35<sup>th</sup> Street and 35<sup>th</sup> Street at Redwood Street, as well as the proposed site driveway approach on the HWY 101 site frontage. To simulate and review approximate vehicle queue lengths, the analysis software program SimTraffic 10, by TrafficWare is utilized to model development scenario traffic conditions to determine if vehicle queue lengths are increased significantly with traffic added from the proposed development, or if blocking conditions exist or are worsened with the development traffic. SimTraffic utilizes random number seeding to generate approaching traffic scenarios and to simulate resulting traffic conditions. For queuing reports, a minimum five runs are simulated with the vehicle queue lengths calculated from the average of the five runs.

To accurately model the signalized intersection approach queue lengths at the site driveway on HWY 101 the driveway approach intersection node representing the driveway location was relocated out of the potential traffic signal queue area at the street intersection at HWY 101 and 35<sup>th</sup> Street. The iteration was performed because SimTraffic does not report overlapping queues appropriately when an approach is located within an intersection approach queue, and it is unknown if the intersection's southbound approach queue is as long or longer than the link length.

The SimTraffic report splits the queue length between the two intersection nodes and reports through movement queues for the uncontrolled approaches at the upstream intersection and the downstream intersection, rather than reporting the queue for the intersection where vehicles are required to stop downstream. In some instances, SimTraffic also reports through movement queues at uncontrolled approaches that are not influenced by downstream metering at partially controlled (unsignalized) intersections. For these reasons, ODOT allows alternate manual calculation methods for calculating vehicle queue lengths at stopped controlled approaches, such as the AASHTO two-minute rule.

Background/No-Build and Build output files from SimTraffic documenting calculated vehicle queue lengths are included as Appendices I and J. The following table summarizes the SimTraffic queuing microsimulation results:

	Table 7: Design Hour Intersection Queuing									
Intersection:	Approach Movement	Queue Storage Length (ft)	Current Year 2020 Ave/95 <sup>th</sup>	2021 "No-Build' Ave/95 <sup>th</sup>	2021 'Build' Ave/95 <sup>th</sup>	2026 'No- Build' Ave/95 <sup>th</sup>	2026 'Build' Ave/95 <sup>th</sup>			
AM Peak Hour										
	EBL	110'	50/75	50/75	50/75	50/75	50/75			
	EBTR	250+	50/75	50/75	50/75	50/75	50/75			
	WBL	140'	25/50	25/50	25/75	25/50	25/50			
	WBTR	275'	25/50	25/50	25/50	25/50	25/50			
HWY 101 at	NBL	150'	25/50	25/50	25/50	25/50	25/50			
35 <sup>th</sup> Street	NBT	250'	50/100	50/100	50/125	50/100	25/50			
	NBTR	250'	25/50	25/75	50/75	25/75	50/75			
	SBL	93'	25/50	25/50	25/75	25/50	25/75			
	SBT	140'	50/100	50/100	50/100	50/100	50/100			
	SBTR	245'	50/100	50/100	50/100	50/100	50/100			
35 <sup>th</sup> Street at	NBLTR	150'+	25/50	25/50	25/50	25/50	25/50			
Redwood	SBLTR	55'	0/0	0/0	25/50	0/0	25/50			
HWY 101 at Site Dwy	WBR	50'	0/0	0/0	25/50	0/0	25/50			
			PM Pea	k Hour						
	EBL	110'	50/75	50/75	50/75	50/75	50/100			
	EBTR	250+	50/100	50/100	50/100	50/100	50/100			
	WBL	140'	25/50	25/50	25/50	25/50	50/75			
	WBTR	275'	25/50	25/75	25/75	25/75	25/75			
HWY 101 at	NBL	150'	50/75	50/100	50/75	50/75	50/100			
35 <sup>th</sup> Street	NBT	250'	75/150	75/150	75/150	75/150	100/150			
	NBTR	250'	75/125	75/125	75/125	75/150	75/150			
	SBL	93'	25/75	25/75	50/75	25/75	50/75			
	SBT	140'	100/150	100/150	100/150	100/175	100/175			
	SBTR	245'	75/150	75/150	100/150	100/150	100/175			
35 <sup>th</sup> Street at	NBLTR	150'+	50/50	25/50	25/50	25/50	50/75			
Redwood	SBLTR	55'	0/0	0/0	25/50	0/0	25/50			
HWY 101 at Site Dwy	WBR	50'	0/0	0/0	25/50	0/0	25/50			

As shown in the table, the projected vehicle queuing on the study area roadways is not significantly increased with the addition post-development traffic.

As discussed previously, approaches that are unsignalized are not modeled as well as signalized approaches in the SimTraffic microsimulation analysis software. Although ODOT allows alternate manual calculations for unsignalized intersections, such as the AASHTO 2-minute rule, the results reported above are from SimTraffic for both signalized and unsignalized intersection approaches within the study area. The SimTraffic reports also show a westbound queue at the 35th Street approach to Redwood Street for no-build conditions and east- and westbound queues for the build traffic scenarios. The eastbound and westbound approaches are not likely to result in tangible queues, since the approaches are free movements, and most reported queues are less than one standard car length which is generally considered 25 feet. The ODOT Analysis Procedures Manual

discusses the use of the AASHTO 2-minute rule for approximating unsignalized intersection vehicle queues, specifically at stop-controlled approaches. The AASHTO 2-minute rule does not calculate through street non-stopped movement traffic queues, which are also not shown here.

### 4.9 VEHICLE CIRCULATION

The site plan includes a proposed unrestricted driveway approach on 35th street that is currently shown offset west from the centerline of Redwood Street. Redwood Street approaches 35th street from the south with the site driveway approach at the north side of 35th Street approximately 35 feet west of Redwood Street. During the scoping process, the City of Florence mentioned the proposed offset of the driveway to the intersection of Redwood Street and requested a discussion about the operation of offsetting left turns from 35th Street to the site and from 35th Street to Redwood Street. Optimally, the site driveway would be aligned with the Redwood Street approach centerline, except the site's eastern property boundary is aligned with the centerline of Redwood Street and does not allow such a configuration to be constructed. To add to the discussion regarding the offset intersection and driveway approach on 35th Street that was provided in section 4.3 previously, a driveway approach movement simulation is provided as Figure 11 on the following page that simulates vehicle maneuvers from the site directly to Redwood Street and from Redwood Street directly into the site. The turning movement simulations are prepared based on AUTOTURN® version 10.2.3.70, and demonstrate that a full-size crew cab pickup truck with a 168inch (14.0 foot) wheel base is capable of maneuvering to the site driveway from Redwood Street and the reverse, simultaneously, without any noted physical conflicts. There are not likely to be a large number of vehicles making these maneuvers, as was demonstrated in section 4,6 and on Figures 8 through 10 (trip distribution and build traffic volumes).

Vehicular traffic destined to the drive-through can either enter the site at the RIRO (right-in right-out only) approach on HWY 101, which requires circulation through the parking area to the drive-through, or enter via the unrestricted approach on 35<sup>th</sup> Street which is generally aligned with the drive-through entrance. The site plan shows stacking for up to 13 vehicles total with approximately 20 feet of linear space for each vehicle. The drive-through features two ordering kiosks (two) that will be located in the southeast quadrant of the parcel that allow simultaneous ordering for two vehicles. Downstream of the ordering kiosks (between the kiosks and the pick-up window), there is a merge point where the two lines from the two kiosks merge to form one single lane through the drive-through pick-up window. Including vehicles at the ordering kiosks, there is stacking available for approximately 7 vehicles at the ordering kiosks and upstream to the drive-through entrance in the parking area. The drive-through window is shown in the northwest portion of the building where drive-through egress vehicles can circulate counter clockwise back through the site parking area to the unrestricted driveway approach on 35<sup>th</sup> Street for destinations to the south or to the west, or make a right turn followed by a left turn to get to HWY 101 and travel northbound.

Although the drive-through appears to work as it is shown with available space for up to 13 vehicles, and 7 of those vehicles at the ordering kiosks and upstream within the designated aisle, if/when the number of vehicles waiting to order is greater than 8 vehicles, there is a propensity for traffic entering the site to be blocked from accessing the parking area from the driveway. It is recommended that the driveway approach be relocated 70 to 75 feet west to avoid a direct alignment with the drive-through queue and to allow more vehicles to be stored on site leading to the drive-through facilities, and to improve access to the parking area when the drive-through is experiencing peak usage. A scenario showing a possible access location on 35th Street is shown

on Figure 12. The relocated proposed access shown on the site plan would be accommodated with parking adjustments that would replace the area currently shown on the site plan with the proposed driveway. There may be issues with crossover access as is shown on the site plan for future use if the parcels to the north are developed, however the proposed driveway access stub shown at the RIRO is assumed to provide access to the property north of the Burger King Site as well for future considerations. If there is currently a recorded deed covenant regarding an access easement, the currently shown location of the approach will meet the City's codified requirements, but is otherwise recommended to be widened to 35 feet to allow two ingress lanes, one of which will allow the drive-through to be by-passed during peak usage to access parking on the site.

In addition to the recommended access relocation, some additional motor vehicle traffic related circulation improvements are recommended to keep onsite circulation flowing at optimal flow conditions and to maintain a high level of safety and situational awareness for the traveling public. The recommended improvements include signage for egress vehicles reminding drivers not to block the driveway entrance on HWY 101 when departing the site (turn left against a queue when queue is present to turn right onto HWY 101 northbound – mostly for future consideration when parcels to the north are developed and the driveway is extended); do not enter signage for vehicles entering the site designating the operation of the drive-through egress; do-not enter signage for southbound traffic on HWY 101 to remind drivers not to enter the RIRO approach via left-turn, and; signage showing the channelization of the approach at HWY 101, which will be restricted to right-in right-out only. These recommended improvements are shown on Figure 12 on page 30.

### 4.10 PEDESTRIAN ACCESS AND CIRCULATION

Florence Development Code Title 10, Chapter 35, Section 3-2 (10-35-3-2B) provides direction on pedestrian access and circulation from public rights-of-way at development site street frontages to building entrances on site. A few recommended improvements to the site plan are included on Figure 12 on Page 30. The recommended improvements include the following:

- An ADA compliant raised walkway adjacent to the parking stalls at the north side of the western parking area that connects to the HWY 101 sidewalk at the frontage. The walkway should lead to the building entrance area near the door facing HWY 101 and should include accessible ramps as needed as well as crosswalk markings across the area exposed to vehicular traffic;
- ❖ An ADA compliant walkway from the 35<sup>th</sup> street frontage to the main building entrance facing 35<sup>th</sup> street. There is currently an ADA accessway between two ADA parking stalls shown that would be a suitable location. The walkway should connect to the existing sidewalk on 35<sup>th</sup> Street and there may be a need for a ramp in the parking area. The crossing is already shown with suitable markings

The site plan layout provides pedestrian access and circulation around the perimeter of the building where applicable (not including adjacent to the drive-through lane, where it would be between the drive-through and the building).

- RECOMMENDED SIGN 1: STOP SIGN
- RECOMMENDED SIGN 2: DO NOT BLOCK DRIVEWAY

DANH

- RECOMMENDED SIGN 3: DO NOT ENTER
- RECOMMENDED SIGN 4: RIGHT-TURN
- RECOMMENDED ADDITIONAL MARKED CROSSWALK
- RECOMMENDED SHORTEN CONCRETE ISLAND
- RECOMMENDED WIDEN SIDEWALK TO 8' FOR CLEARANCE AROUND EXTERIOR DOOR SWING (OPENS OUT). NARROW DRIVE-AISLE TO 24.0 FEET.
- RECOMMENDED RELOCATE DRIVEWAY APPROACH







SIGN 3





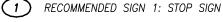


GRAPHIC SCALE

15

RECOMMENDED **ONSITE TRAFFIC** CONTROL

1/1







SIGN 4

⋛ TRAFFIC IMPACT ANALSYSIS W. 11th AVE EUGENE, OREGON KING BURGER

SITE

AVE

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JUNE 30, 2020

20-113 FIGURE 12

# 5.0 RECOMMENDATIONS AND CONCLUSION

#### **Crash Analysis**

No crash reducing measures are necessary to accommodate the proposed project. The accident histories show that the intersection at 35<sup>th</sup> Street and HWY 101 has been relatively safe for the five-year period of available crash data received from ODOT. HWY 101 at the site frontage is not identified as a 95<sup>th</sup> percentile ODOT Safety Priority Index System (SPIS) location on the most recent SPIS maps.

#### **Access**

Based on access locations and geometries, there are no unusual safety related concerns associated with the existing and proposed access locations or conditions. The site's proposed driveway approach on the HWY 101 is proposed as a right-in right-out only restricted access. The site plan shows a second unrestricted access on 35th Street that may have an access easement associated with it for future development of the parcels on the site that are not proposed to be developed at this time. It is recommended that the site driveway approach on 35th Street be considered for relocation to approximately 70 to 75 feet to the west to improve onsite circulation during peak drive-through usage. There appears to be adequate available queue storage at the westbound approach to HWY 101 with the recommended relocation of the driveway and the recommended driveway location meets the spacing criteria from the intersection at HWY 101 required by City of Florence code. All accesses are necessary to accommodate existing on-site circulation of the expected site generated levels of vehicle traffic and for access and circulation of delivery vehicles.

#### **Trip Generation**

The proposed development is projected to generate an average of 120 AM and 98 PM new vehicle trips and 1,409 new average daily trips accessing the site split between the approach at the site driveway at the frontage on HWY 101 and the unrestricted access on  $35^{\rm th}$  Street .

#### **Intersection Performance**

The studied public street intersection at 35<sup>th</sup> Street and HWY 101 and the site driveway approaches are projected to operate at acceptable levels of service under all analyzed scenarios. The LOS are currently within the mobility standards of the City of Florence for LOS E or better and are not projected to exceed that standard under the analyzed conditions for the anticipated year 2021 year of opening or the future year 2026 post development analysis scenarios. As modeled, the vehicle queue lengths are accommodated within available storage and no modifications are necessary to accommodate incremental traffic increases from the proposed development on the public street frontages and approaches.

#### Site Traffic Control

The site plan was reviewed and recommendations were made to improve access and circulation on the site and to and from the public streets. The improvements are mostly related to providing inexpensive signage and pavement markings to enhance situational awareness of pedestrians on the site and provide logical wayfinding for the general traveling public to ensure traffic flows on and off the site efficiently and safely.

#### Conclusion

In summary, the proposed development project will not cause significant adverse impact on the performance of the public transportation system.

### APPENDIX A SCOPE OF WORK MEMO AND E-MAILS

<sup>©</sup> Branch Engineering, Inc.

### Dan H

To: Mike Miller Subject:

RE: 35th St BK Site TIA

From: Mike Miller <mike.miller@ci.florence.or.us>

**Sent:** Thursday, March 19, 2020 1:46 PM To: Dan H <danh@branchengineering.com>

Cc: Wendy Farley-Campbell <wendy.farleycampbell@ci.florence.or.us>; planningdepartment

<planningdepartment@ci.florence.or.us>

Subject: RE: 35th St BK Site TIA

Hi Dan,

Myself and one of our engineers reviewed the scoping memo (attached for the benefit of our planning staff) for the Burger King TIA. We do not have any concerns, but wanted you to be aware of a couple of items:

- The 35<sup>th</sup> and Hwy 101 intersection will have some impact due to a new proposed residential development at Rhody and 35<sup>th</sup>. You might want to include the information in your analysis (I have attached their TIA)
- If a traffic count is used for the study, we recommend if possible, to complete a count in July/August instead of just relying on a seasonal adjustment.

Thank you!

Mike

From: Dan H < danh@branchengineering.com > Sent: Monday, March 16, 2020 9:33 AM

To: Mike Miller <mike.miller@ci.florence.or.us> Subject: [Suspected SPAM] 35th St BK Site TIA

Importance: Low

Hi Mike,

I got your voice mail last week, and have prepared the attached memo with the proposed scope of work for the forthcoming TIA. Per your request, we will look at the driveway on 35th with respect to offsetting left turns with the intersection of Redwood Street and discuss the access configuration as well as looking at the intersections of 35th and Redwood and 35<sup>th</sup> and HWY 101 for operations and safety.

Please let me know if this sounds reasonable.

Thanks,

DAN HAGA, P.E.

**Project Engineer** 

### **BRANCH ENGINEERING, INC.**

310 5th Street, Springfield, OR 97477

**p**\_ 541.746.0637

### **TECHNICAL MEMORANDUM**





**DATE:** March 16, 2020

**PROJECT:** HWY 101/35<sup>th</sup> St. BK Site, BEI Project 20-113

TO: Mike Miller

City of Florence Public Works

**FROM:** Dan Haga, P.E.

Branch Engineering

**RE:** Burger King Development Site,

Map lot: 18-12-23-22-06800





EXPIRES: JUNE 30, 2021

In an effort to assist the City of Florence in the process of concurring on the scope of traffic impact analysis required to address applicable approval criteria for the proposed development of the subject site referred to in the subject line, identified as taxlot 06800 of tax map 18-12-23-22, I am supplying this memorandum summarizing the land use assumptions and associated site generated traffic, the proposed study area and details to be included in the forthcoming analysis. The proposed uses of the site are in compliance with the City of Florence's existing Highway Commercial zone and Comprehensive Plan Map designations for the site. The result of this memorandum is intended to be the basis for concurring on the scope of analysis required to analyze development impacts in compliance with City of Florence Development Code, Title 10, Chapter 1, Section 10-1-1-4: E and Title 10, Chapter 35, Section 10-35-2-5.

### **Existing Conditions**

The subject site is located in the northeast quadrant of the intersection of HWY 101 at 35<sup>th</sup> Street. The site is within the urban growth boundary and city limits and is within an urbanized area of Florence, Oregon. The property is approximately 1.85 acres in size and is currently vacant and undeveloped. The aerial photograph on the following page shows the approximate site location.



Site Location (Aerial by Lane County Maps)

The currently proposed development of the site includes a 2.992KSF fast-food restaurant with a drive-through window and improvements for parking on the site. Primary access to the proposed development would be at a right-in right-out approach to be constructed at the site's frontage on the east side of HWY 101, and at an unrestricted access at the site's frontage on the north side of 35<sup>th</sup> Street west of the intersection at Redwood Street. There is a possibility for secondary access to the vacated right-of-way of 36<sup>th</sup> Street that serves as the primary access to the adjacent property to the north. The vacated 36<sup>th</sup> Street right-of-way to the north borders the subject site's north property boundary. The adjacent property to the north is developed as a full-service restaurant.

### **Trip Generation**

To project trip generation for the proposed development's use, a reference was made to the *Trip Generation Manual, 10<sup>th</sup> Edition*, and the *Trip Generation Handbook, 3<sup>rd</sup> Edition*, both published by the Institute of Transportation Engineers (ITE). The table on the following page summarizes the projected peak hour site generated traffic based on the available published data:

Branch Engineering, Inc.

Florence	e Burger K	ing Site	Trip Gen	eration		
Land Use/ ITE Land Use Code	Ind. Variable	Units (QTY)	TG Rate Trips/Unit	Trips	Trips IN (%/#)	Trips OUT (%/#)
	AM	Peak Hour				
Fast Food Restaurant with Drive- Through Window/934	KSF GFA	2.992	40.19	120	51%/61	49%/59
	Pas	s-By Trips:	49%	59	30	29
	PM	Peak Hour				
Fast Food Restaurant with Drive- Through Window/934	KSF GFA	2.992	32.67	98	52%/51	48%/47
	Pas	s-By Trips:	50%	49	25	24
		ADT				
Fast Food Restaurant with Drive- Through Window/934	KSF GFA	2.992	470.95	1,409	50%/705	50%/704

As indicated above, the proposed post-development traffic conditions could result in up to 120 new AM peak hour vehicle trips and up to 98 new PM peak hour vehicle trips accessing the site. Out of the total peak hour vehicle trip totals, 49 percent of the AM and 50 percent of the PM peak hour ingress and egress trips would be expected to be pass-by trips, which are trips already on the roadway that would be expected to make a route diversion at the driveway to enter and exit the site, but will otherwise continue in the same direction they were traveling upon leaving the site. Pass-by trip occur between primary origins and destinations and do not add traffic to intersections or facilities, other than the site driveway approach. Per City of Florence Zoning Administration, any development that generates 250 or more vehicle trips per day is required to provide a traffic impact analysis. Because the proposed development will generate a potential 1,409 ADT (daily) trips during typical weekday traffic conditions, the proposed development requires a traffic impact analysis.

### Study Area and Analysis Scoping

The site plan is proposing a primary access at the site's frontage on HWY 101, that will be restricted to right-in and right-out only turning movements. HWY 101 at the frontage is within the jurisdiction of the Oregon Department of Transportation. A second unrestricted primary access is proposed to be located on the site's 35<sup>th</sup> Street frontage, which is within the City of Florence's jurisdiction. A secondary access connection may be provided with a crossover access agreement with the adjacent neighbor to the north, where there is an existing unrestricted access that serves the existing restaurant at that site. For the scope of this traffic impact analysis, all post-development site generated traffic will be assumed to utilize the site's driveways for access. The potential crossover access connection to the adjacent unrestricted access is unlikely to detract a lot of additional turning movements, since it is out of direction for left-turns from the site to HWY 101 southbound. Once developed and a connection is established, there may be a few left-turns into the shared access from the north, but there does not appear to be a particular benefit to turning left at the shared approach over turning left at the traffic signal at HWY 101 and 35<sup>th</sup> Street, since the public roads have a higher rate of speed than a route through the parking area, and the site's parking area and drive-through are aligned the accommodate arrivals from the site's frontage on 35<sup>th</sup> Street. Per City of Florence

Branch Engineering, Inc.

comments, offsetting left -turns at the proposed site driveway approach on 35<sup>th</sup> Street with respect to the public street intersection at 35<sup>th</sup> St. and Redwood St. will be evaluated and discussed.

The Oregon Department of Transportation (ODOT) has indicated that ODOT will not require a traffic impact analysis with this development as the proposed access on HWY 101 remains proposed as a right-in right-out restricted access. The City of Florence requires a traffic impact analysis and has indicated that the TIA should include both of the site's driveway approaches, the intersection at HWY 101 and 35<sup>th</sup> St. and at the intersection of 35<sup>th</sup> Street and Redwood St., therefore; these intersections will be analyzed for operational performance, queuing and safety.

### Analysis Year(s)

The City of Florence Traffic Impact Analysis requirements do not contain analysis details that include a definition of the study area that the TIA needs to analyze, nor what analysis years need to be included in a TIA. Per the Oregon Department of Transportation's development review guidelines, Table 3.3, proposed developments that require a traffic impact analysis from ODOT that generate between 1,000 and 2,999 ADT and include a single-phase development horizon year are suggested to include the year of opening and a five-year post-development planning horizon year in the analysis scenarios. Although ODOT does not require an analysis for this proposed development, the analysis required by the City of Florence is proposed to include the year of opening (assumed to be the current year 2020/year of application), and a five-year post-development horizon year, 2025, consistent with the suggested analysis year scenarios by ODOT. The forthcoming analysis will include AM and PM peak hour scenarios with and without additional traffic from the proposed development during those years.

### **Pipeline Traffic**

Branch Engineering is not aware of any nearby pending land use applications or traffic impact analyses that may add a significant amount of incremental traffic to the study area that is in the approval process at the time of scoping for this application. If there are any known in-process developments that have pending approved TIA applications in with the City and have not been constructed and opened yet at the time of this scoping exercise, please provide any information on those if they include pipeline trips at the site's frontages or at the intersection of 35<sup>th</sup> Street and HWY 101.

### Conclusion

In summary, based on the City of Florence Traffic Impact Analysis trip generation threshold of 250 ADT trips provided in Title 10, Chapter 1, Section 10-1-1-4: E, the proposed Burger King development site will require a traffic impact analysis to determine if there is an identifiable impact to existing and background future traffic performance conditions during the design hour of the year of application (year 2020) and at the end of a five (5) year post-development planning horizon (year 2025). The forthcoming analysis will determine traffic impacts based on any updates to the preliminary site plan. We look forward to working with you to get concurrence on the scope of work to be provided through this traffic impact analyses and land development approval process. Please do not hesitate to contact me if I can provide any additional information.

Branch Engineering, Inc.

### Dan H

From: NELSON Brian S \* Scott < Brian.S.NELSON@odot.state.or.us>

**Sent:** Tuesday, March 10, 2020 1:52 PM

To: Dan H; BLAIR Keith P

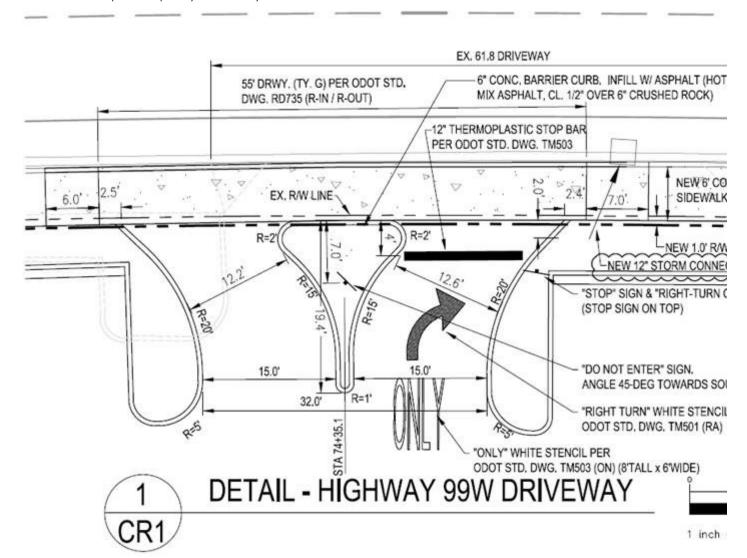
**Cc:** (mike.miller@ci.florence.or.us); BAUMGARTNER Douglas G

**Subject:** RE: Florence BK TIA

Hi Dan, our concerns related to the new private approach would be addressed with a restriction to right-in/right-out. If the developer agrees to this no TIA will be required with the approach permit process. If the developer is not in agreement with the right-in/right-out approach they should expect a TIA addressing our operational and safety concerns to be required.

If the City's code requires a TIA that looks at highway intersections we are always happy to assist with that review. Keith is handling a few positions right now as he was promoted to the Region 2 Traffic Manager, but we will find resources to take a look at it if the City wishes to include us.

Below is a design for a curb tight sidewalk right-in/right-out porkchop that was done about 5 years ago in McMinnville. It operates pretty effectively.



### **Thanks**

### B Scott Nelson, P.E.

Region 2 Access Management Engineer



455 Airport Rd SE, Bldg. B Salem, OR 97301 Office 503.986.2882 Cell 503.602.0703

From: Dan H <danh@branchengineering.com> Sent: Tuesday, March 10, 2020 12:04 PM

To: NELSON Brian S \* Scott <Brian.S.NELSON@odot.state.or.us>; BLAIR Keith P <Keith.P.BLAIR@odot.state.or.us>

Cc: (mike.miller@ci.florence.or.us) <mike.miller@ci.florence.or.us>

Subject: Florence BK TIA

### Hi Scott and Keith,

I spoke with Scott a while back regarding a site in Florence at the NE corner of HWY 101 and 35<sup>th</sup> Street regarding access and TIA applicability. It sounded like a TIA may not necessarily be required by ODOT. I have attached a preliminary site plan for the proposed development. It looks like the plan includes approximately 2.992KSF GFA, which equates to 98 PM peak hour trips, with 50% of those as pass-by trips per the 10<sup>th</sup> Edition ITE and Trip Gen Handbook, 3<sup>rd</sup> Edition. The AM trip gen comes to 120 trips, with 48% of those as pass-by. The ADT based on the ITE rate would be 1409.

Can we get confirmation if ODOT will or will not require a TIA for this site? If a study is required, please let me know if you need anything for scoping.

Also, FYI, the City's Trip Generation threshold is 250 ADT trips, so it looks like we will at least need to provide a TIA to meet the City's requirements. I contacted Mike Miller (Public Works Director) regarding scoping for that, and am waiting for a response to see what the City requires, or whom I should contact as engineer of record for the City to negotiate scoping for the City. Will ODOT want to review the TIA if it isn't required by ODOT? I suspect that most of the potential for impact would be on HWY 101, with primary trip origins and destinations filtering out fairly evenly to side streets in the vicinity. It looks like they are proposing access control on HWY 101 at the proposed driveway approach.

Thanks,

DAN HAGA, P.E. **Project Engineer** 

### **BRANCH ENGINEERING, INC.**

310 5th Street, Springfield, OR 97477

**p**\_ 541.746.0637

www.branchengineering.com

### APPENDIX B SITE PLAN



### SITE PLAN SCALE: 1" = 20'-0"

DEFERRED SUBMITTALS: SIGNS

### GENERAL NOTES:

1. ALL DIMENSIONS ARE TO FACE OF STUD, FACE OF CONC. OR CENTERLINE OF COLUMN, U.N.O.

2. CONTRACTOR TO VERIFY ALL DIMENSIONS IN THE FIELD W/
DIMENSIONS SHOWN ON PLAN PRIOR TO CONSTRUCTION & ORDERING
MATERIALS. ARCHITECT OF RECORD SHALL BE NOTIFIED IN WRITING
OF ANY DISCREPANCIES BETWEEN THESE DRAWINGS AND ACTUAL FIELD

3. CONTRACTOR TO VERIFY DIMENSIONS OF MANUFACTURED PRODUCTS TO ASSURE COMPATIBILITY WITH THE PLANS.



### **NEW BURGER KING ROC-60T RESTAURANT**

W/ BKOT IMAGE

### PROJECT CRITERIA:

PARCEL NO.: 1812232206800000

LOT NUMBER 1-20 BLOCK 10 SEC 23 TWN 18 RNG 12 LEGAL DESCRIPTION:

BUILDING CRITERIA:

JURISDICTION: CITY OF FLORENCE

CODE: 2015 IBC ZONING: HIGHWAY LOT SIZE: 1.07 ACRES BUILDING AREA: 2,992 S.F. DINING SEATS: 60-62 SEATS

CONSTRUCTION: V-B (NOT-SPRINKLERED) A-2 RESTAURANT OCCUPANCY TYPE:

1 PER 125SF FLOOR AREA PARKING REQUIRED: 2,992SF / 125SF =

24 STALLS REQUIRED

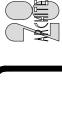
PARKING PROVIDED: 22 STANDARD STALLS 2 ACCESSIBLE STALLS

24 TOTAL PARKING STALLS









A NEW ROCEOT BUILDING FOR:
BURGER KING RESTAURANT

SP-

### APPENDIX C RHODY EXPRESS ROUTE SCHEDULES



# RHODY EXPRESS IS ACCESSIBLE

The Rhody Express vehicle is accessible to people who use mobility devices. The driver will assist with lift boarding and securement. Anyone can request the lift to help board the bus.

# **BUS BUDDIES ARE HERE TO HELP**

If you need help learning how to use the bus or would like a buddy to ride with you the first few times, call the Rhody Express and ask for a Bus Buddy. These experienced riders are available and ready to help. 541-902-2067 or 7-1-1 (TTY – Oregon Relay).

# WHAT IF I CANNOT ACCESS THE BUS?

If you experience a disabling condition that prevents you from using the Rhody Express, you may qualify for the Rhody Dial-a-Ride, a service that can take you from point A to point B. You must live within three-quarters of a mile from the bus route and participate in an in-person eligibility assessment to access this service. For more information, including eligibility questions, call the RideSource Call Center toll free at 1-877-800-9899 or 7-1-1 (TTY – Oregon Relay).



### WELCOME ABOARD

Welcome to the Rhody Express, your public transportation provider in Florence. We are here to help.

541-902-2067 or 7-1-1 (TTY - Oregon Relay)

### SERVICE HOURS

Monday - Friday 10:00 a.m. - 6:00 p.m.

### HOLIDAY SERVICE

The Rhody Express does not operate on New Year's Day, Memorial Day, 4th of July, Labor Day, Thanksgiving Day and Christmas Day.

### FARES

\$1 for single trip

\$2 for day pass

\$20 for 20-Ride ticket book

Ticket books sold only at River Cities Taxi/ Rhody Express, 1699 Pine Street

# **HOW TO READ THE TIMETABLE**

The Rhody Express route is divided into two loops – the North Loop and the South Loop. The timetable contains timepoints for major stops along the route. Locate the timepoint before the stop you where you want to catch the bus. Read down the column to find the time you want to travel, and wait at your stop at the time listed on the timetable. For example, if you are boarding between the stop at Grocery Outlet and the stop at the Dollar Tree, be sure to be at your stop at the time listed for the Grocery Outlet rather than the time listed for the Dollar Tree.

# RHODY EXPRESS CAPACITY

The Rhody Express bus has capacity for 16 people seated facing forward. The front seats are designated for older adults and people with disabilities. A bike rack on the front bumper has capacity for two bicycles.



### FLORENCE PUBLIC TRANSIT





### Rhody Express

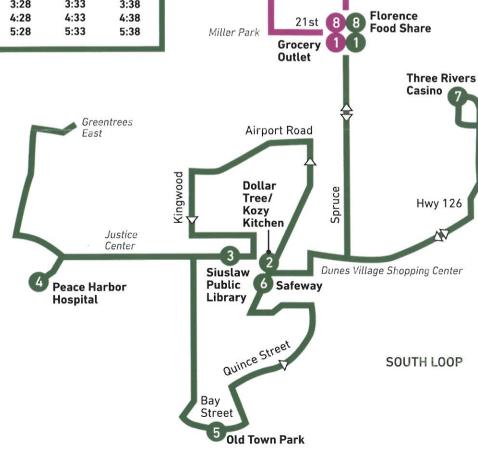
Leave Florence Food Share	Bi-Mart	Fred Meyer	Arrive Grocery Outlet
8	9	10	1
10:38	10:45	10:47	10:58
11:38	11:45	11:47	11:58
12:38	12:45	12:47	12:58
1:38	1:45	1:47	1:58
2:38	2:45	2:47	2:58
3:38	3:45	3:47	3:58
4:38	4:45	4:47	4:58
5:38	5:45	5:47	5:58

SOUTH L	00P						
Leave Grocery Outlet	Dollar Tree/ Kozy Kitchen	Siuslaw Public Library	Peace Harbor Hospital	Old Town Park	Safeway	Three Rivers Casino	Arrive Florence Food Share
0	2	3	4	5	6	7	8
10:00	10:05	10:10	10:17	10:23	10:28	10:33	10:38
11:00	11:05	11:10	11:17	11:23	11:28	11:33	11:38
12:00	12:05	12:10	12:17	12:23	12:28	12:33	12:38
1:00	1:05	1:10	1:17	1:23	1:28	1:33	1:38
2:00	2:05	2:10	2:17	2:23	2:28	2:33	2:38
3:00	3:05	3:10	3:17	3:23	3:28	3:33	3:38
4:00	4:05	4:10	4:17	4:23	4:28	4:33	4:38
5:00	5:05	5:10	5:17	5:23	5:28	5:33	5:38

### RIDING FROM SOUTH TO NORTH

The Rhody Express operates a 60-minute circuit by first traveling the South Loop and then the North Loop.

If your destination is not on the loop on which you are boarding, remain on the bus as it switches to the next loop, and you will reach your destination.



Fred Meyer 110

43rd

**Dak Street** 

Lane Community College

Siuslaw High School

> Siuslaw Middle School

Rhododendron Elementary School Rite Aid Pharmacy **NORTH LOOP** 

**Bi-Mart** 

35th

42nd

### APPENDIX D CRASH DATA

General & Site Inform	nation
Analyst:	DNH
Agency/Company:	Branch Engineering, Inc
Date:	6/15/2020
Project Name:	Burger King TIA

		Intersection	on Crash Data	1			
	Intersection			Year			
Intersection	Туре	2014	2015	2016	2017	2018	Total
35th at HWY 101	Urban 4SG	2	1	3	2	4	12
35th at Redwood	Urban 3ST	0	0	0	0	0	0
							0
							0
							0
							0
							0
							0
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							0
							0
							0
	Total	2	1	3	2	4	12

Intersection P	opulation Typ	e Crash Rate		
Average Crash	Rate per inte	rsection type		
_			Avg Crash	
	Sum of	Sum of 5-	Rate for Ref	
Intersection Pop. Type	Crashes	year MEV	Pop.	INT in Pop
Rural 3SG	0	0		
Rural 3ST	0	0		
Rural 4SG	0	0		
Rural 4ST	0	0		
Urban 3ST	0	2	0.0000	1
Urban 3SG	0	0		
Urban 4ST	0	0		
Urban 4SG	12	29	0.4110	1

			Critical Rate	Calculation				
				Intersection	I	Reference		
	AADT Entering			Population	Intersection	Population Crash	Critical	Over
Intersection	Intersection	5-year MEV	Crash Total	Type	Crash Rate	Rate	Rate	Critical
35th at HWY 101		29.2	12	Urban 4SG	0.41	0.41	0.62	Under
35th at Redwood	1,200	2.2	0	Urban 3ST	0.00	0.00	0.23	Under
					-			
					-			
					-			
					<del> </del>			

OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION

Page:

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CDS150 06/15/2020 CRASH SUMMARIES BY YEAR BY COLLISION TYPE

35TH ST at REDWOOD ST, City of Florence, Lane County, 01/01/2014 to 12/31/2018

ROAD OFF-SECTION RELATED INTER-INTER-SECTION DARK DAY WET SURF DRY TRUCKS PEOPLE INJURED PEOPLE KILLED TOTAL CRASHES ONLY DAMAGE PROPERTY NON-FATAL CRASHES FATAL CRASHES COLLISION TYPE

FINAL TOTAL

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit can not Reporting Unit can not guarantee the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are represented nor can assurances be made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirements, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION

Page:

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CDS150 04/09/2020 CRASH SUMMARIES BY YEAR BY COLLISION TYPE

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

		NON-	PROPERTY										INTER-	
COLLISION TYPE	FATAL CRASHES	FATAL CRASHES	DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET	DAY	DARK	INTER- SECTION	SECTION RELATED	OFF- ROAD
YEAR: 2018														
ANGLE	0	П	1	2	0	П	0	-	П	П	$\vdash$	2	0	0
TURNING MOVEMENTS	0	2	0	7	0	m	0	2	0	7	0	2	0	0
YEAR 2018 TOTAL	0	м	1	4	0	4	0	ю	н	m	Н	4	0	0
YEAR: 2017														
FIXED / OTHER OBJECT	0	0	1	$\vdash$	0	0	0	$\Box$	0	П	0	$\vdash$	0	П
TURNING MOVEMENTS	0	П	0	$\vdash$	0	2	0	□	0	П	0	$\vdash$	0	0
YEAR 2017 TOTAL	0	Н	1	7	0	ĸ	0	7	0	7	0	7	0	н
YEAR: 2016														
ANGLE	0	$\vdash$	0	$\vdash$	0	П	0	П	0	Η	0	$\vdash$	0	0
TURNING MOVEMENTS	0	2	0	2	0	5	0	2	0	2	0	2	0	0
YEAR 2016 TOTAL	0	m	0	ю	0	9	0	м	0	ю	0	m	0	0
YEAR: 2015														
BACKING	0	0	□	$\vdash$	0	0	0	⊣	0	□	0	T	0	0
YEAR 2015 TOTAL	0	0	Н	Н	0	0	0	н	0	п	0	П	0	0
YEAR: 2014														
TURNING MOVEMENTS	0	0	2	7	0	0	0	7	0	7	0	7	0	0
YEAR 2014 TOTAL	0	0	2	7	0	0	0	7	0	8	0	7	0	0
FINAL TOTAL	0	7	ις	12	0	15	0	11	н	11	1	12	0	Н

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit can not Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is the responsibility of the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are represented nor can assurances be made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirements, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

ORBGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION TRANSPORTATION DATA SECTION - CRASH ANALLYSIS AND REPORTING UNIT URBAN NON-SYSTEM CRASH LISTING

of 12 Crash records shown. 1 - 4

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

CITY OF FLORENCE, LANE COUNTY

CDS380

			Tomes	10	00	00	10	00	10	00	000	0.2	00	000		00	0.2	00	0.5	ć	00	0.2	000	0.0	00		00
			ENGRAD BOX	062	000	000		088	000	011	000		000	000		000		000	000		000		000	000	000		000
			90996	Young		000			016,011,026		000			000		004,028			004,028		000			000	000		004,028
		Ø	G E LICNS PED			00 Unk UNK UNK			00 M OTH-Y OR-?		79 F OR-Y OR<25			52 M OR-Y OR<25		00 Unk UNK UNK			82 M OR-Y OR<25		58 F OR-Y OR<25			54 F OR-Y OR>25	61 M		93 M OR-Y OR<25
			PRIC INJ			01 DRVR NONE			01 DRVR NONE		01 DRVR NONE			01 DRVR NONE		01 DRVR NONE			01 DRVR NONE		01 DRVR NONE			01 DRVR INJC	02 PSNG INJC		01 DRVR INJC
		MOVE	FROM	TURN-L	N E		BACK	M -		STOP E -W		STRGHT	S- N		TURN-L	S -W	TURN-L	M- S		STRGHT	0 1	STRGHT	S-N		STRGHT N -S	TURN-L	M - S
	SPCL USE	TRLR QTY	OWNER	01 NONE 9	N/A	PSNGR CAR	01 NOME 0	PRVTE	PSNGR CAR	02 NONE 0 PRVTE	PSNGR CAR	N OI NOME O	PRVTE	PSNGR CAR	02 NONE 0	UNKN PSNGR CAR	4 01 NONE 0	PRVTE	PSNGR CAR	02 NONE 0	PRVIE PSNGR CAR	N 01 NONE 0	PRVTE	PSNGR CAR	01 NONE 0 PRVTE PSNGR CAR	02 NONE 0	PRVTE PSNGR CAR
		CRASH	COLL	FIX OBJ	FIX	PDO	0-1STOP	BACK	PDO			0-1 L-TURN 01 NONE	TURN	PDO			0-1 L-TURN 01 NONE	TURN	PDO			O-1 L-TURN 01 NONE	TURN	ING			
			SURF		DRY	DAY	CLR	DRY	DAY			CLR	DRY	DAY			CLR	DRY	DAY			CLR	DRY	DAY			
		OFFRD	RNDBT	7	z	Z	Z	z	z			z	Z	z			z	N	z			z	Z	z			
			TRAF-		TRF SIGNAL		N	TRF SIGNAL				z	TRF SIGNAL				N	TRF SIGNAL				N	TRF SIGNAL				
	INT-TYPE	(MEDIAN)	LEGS	CROSS		0	CROSS		0			CROSS		0			CROSS		0			CROSS		0			
		RD CHAR	DIRECT	INTER	ы	0.5	INTER	ы	90			INTER	CN	0.1			INTER	CN	0.1			INTER	CN	01			
	CITY STREET	FIRST STREET	SECOND STREET	OREGON COAST HY	35TH ST	000300100800	OREGON COAST HY	35TH ST				OREGON COAST HY	35TH ST	00000000000			OREGON COAST HY	35TH ST	000000000000			OREGON COAST HY	35TH ST	000000000000			
	CLASS	DIST	FROM	14		-124 6 5.19	17	0	-124 6	4		14		-124 6	5.1887159		14		-124 6	5.1887159		14		-124 6	ñ.		
	S W DATE	C O DAY	H R TIME	N N 08/12/2017	SA	12P 43 59 48.99	N N 08/19/2015	WE	10A 43 59 48.99 -124 6			N N 01/16/2014	HI	1P 43 59	48.9881399		N N 05/03/2014	SA	8P 43 59	48.9881399		N N 02/05/2016	쩐	12P 43 59 48.99 -124 6			
S	Д	E A U	PT E L G N	N C			N N N T					3 N N N	PT				4 N N N	PT				N N N 6	PT				
	SER#	INVEST	RD DPT	02884	CIIX	zz	17770	NONE	zz			00003	NO RPT	zz			01274	NO RPT	zz			00479	NO RPT	zz			

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OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT URBAN NON-SYSTEM CRASH LISTING

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

CDS380 04/09/2020

CITY OF FLORENCE, LANE COUNTY

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ORBOON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT URBAN NON-SYSTEM CRASH LISTING

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

CITY OF FLORENCE, LANE COUNTY

CDS380

of 12 Crash records shown. 2 - 8

		ы																						
		CAUSE	00	00	02	00	0.5	000	0.4	00	0.4	00	0.2	00	0.2	00	00	0	8 00		00		00	
		ACT EVENT	000	000		000	000	000		000	000	000		000	000	000	000	000	000		000		000	
		ERROR		000			004,028	000			020	000			004,028		000		000		000		000	
	FICNS	X RES LOC					OR-Y OR<25	OR-Y OR<25			OR-Y OR<25	OR-Y OR<25			SUSP N-RES						OR-Y	OR>25		
Ø		Y. E		95 E			2 25 F	E 69			71 M	5 62 F			46 M		W 69 W		03 F		3 24 F		3 01 F	
		E SVRTY		1G INJC			7R NONE	AR INJC			/R INJC	/R NONE			7R INJC		IG INJC		IG INJC		AR INJB		IG INJB	
	PRIC	P# TYPE		02 PSNG			01 DRVR	01 DRVR			01 DRVR	01 DRVR			01 DRVR		02 PSNG		03 PSNG		01 DRVR		02 PSNG	
MOVE	FROM	TORN-L	N - S		TURN-L	M -N		STRGHT E -W	STRGHT	S - N		STRGHT E -W	TURN-L	S -W		TURN-L S -W		TURN-L	5	STRGHT	S - N	EHOGES	N -S	
SPCL USE TRLR QTY	OWNER	V# TYPE 02 NONE 0	PRVTE	PSNGR CAR	N 01 NONE 0	PRVTE	PSNGR CAR	02 NONE 0 PRVTE PSNGR CAR	01 NONE 0	PRVTE	PSNGR CAR	02 NONE 0 PRVTE PSNGR CAR	N 01 NONE 0	PRVTE	PSNGR CAR	01 NONE 0 PRVTE	PSNGR CAR	01 NONE 0	PSNGR CAR	02 NONE 0	PRVTE PSNGR CAR	o minom		
CRASH	COLL	SVRTY			0-1 L-TURN 01	TURN	INJ		ANGL-OTH	ANGL	UNI		0-1 L-TURN 01 NONE	TURN	UNI									
		LIGHT			CLR	DRY	DAY		CLR	DRY	DAY		CLR	DRY	DAY									
OFFRD	RNDBT	DRVWY			z	z	z		z	z	z		z	z	Z									
		CONTL			Z	TRF SIGNAL			Z	TRF SIGNAL			z	TRF SIGNAL										
INT-TYPE (MEDIAN)	LEGS	(#LANES)			CROSS		0		CROSS		0		CROSS		0									
RD CHAR	DIRECT	LOCIN			INTER	CN	0.2		INTER	CN	0.1		INTER	CN	10									
CITY STREET FIRST STREET	SECOND STREET	IRS			OREGON COAST HY	35TH ST	000000000000		OREGON COAST HY	35TH ST	008001006000		OREGON COAST HY	35TH ST	000000000000									
CLASS	FROM	LONG			14		-124 6	n H	14		124 6	n H	14		124 6	÷ .								
S W DATE C O DAY	H R TIME	L K LAT			N N 05/26/2016	HI	10A 43 59 48.99 =124 6	7	07/13/2016	WE	2P 43 59 48.99 -124 6	.n	N N 05/07/2017	SU	2P 43 59 48.99 -124 6	n								
A D A	N U U U	N S O			N				NNN				N N											
		UNEOC?			01865	NO RPT	zz		02544	STATE	zz		01567	COUNTY	zz									

OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT URBAN NON-SYSTEM CRASH LISTING

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

CITY OF FLORENCE, LANE COUNTY

CDS380 04/09/2020

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ORBOON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT URBAN NON-SYSTEM CRASH LISTING

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

CITY OF FLORENCE, LANE COUNTY

CDS380 04/09/2020

of 12 Crash records shown. 9 - 12

			CAUSE	0.4	00	0.4	C	000	0.2	00	00	00	0.2	00	0	0.2	00	00	0	05	04,27	000	00	ć	00
			ACT EVENT		000	000	000	0000		000	000	000	000	022			000	000	000	000		000	000	Ç Ç	0000
			ERROR			020		000			000		028,004	000				000		028,004			000		000
		S I	Z			NONE 44 M OTH-Y N-RES		INJC 75 M OR-Y OR<25			INJB 16 F OR-Y OR<25		INJC 83 F OR-Y	MONF 40 F OR-V	4			NONE 74 M OR-Y OR<25		INJC 87 F OR-Y OR<25			NONE 00 UNK UNK		NONE 00 Unk UNK
			P# TYPE SVR			01 DRVR NC		01 DRVR IN			01 DRVR IN		01 DRVR IN	OI DRVR NO	DEVE			01 DRVR NC		01 DRVR IN			01 DRVR NC		01 DRVR NC
		MOVE	TO L	STRGHT	S - N		STRGHT	:	STRGHT	N -S		TURN-L S -W	:	STOP W -E		STRGHT	N-S		TURN-L	1	STRGHT	S - N		STRGHT	리   
	SPCL USE	TRLE QTY	OWNER V# TYPE	01 NONE 0	PRVTE	PSNGR CAR	02 NONE 0	PSNGR CAR	1 01 NONE 0	PRVTE	PSNGR CAR	02 NONE 0 PRVTE	PSNGR CAR	03 NONE 0 PRVTE PSNGE CAR	FORGE CAN	1 01 NONE 0	PRVTE	PSNGR CAR	02 NONE 0	PSNGR CAR	01 NONE 9	N/A	PSNGR CAR	02 NONE 9	N/A PSNGR CAR
		CRASH	SVRTY	ANGL-OTH	ANGL	ING			O-1 L-TURN 01 NONE	TURN	INJ					O-1 L-TURN 01 NONE	TURN	INJ			ANGL-OTH	ANGL	PDO		
		WIHR			WET	DAY			CLR	DRY	DAY					CLR	DRY	DAY			CLR	DRY	DUSK		
		OFFRD	DRVWY	z	Z	Z			z	Z	z					Z	z	z			N	Z	z		
			TKAF =		TRF SIGNAL				z	TRF SIGNAL						z	TRF SIGNAL				z	TRF SIGNAL			
	INT-TYPE	(MEDIAN)	(#LANES)	CROSS		0			CROSS		0					CROSS		0			CROSS		0		
		RD CHAR	LOCIN	INTER	CN	0.1			INTER	CN	0.1					INTER	CN	0.4			INTER	CN	03		
	CITY STREET	FIRST STREET	SECOND SIKEET	OREGON COAST HY	35TH ST	000300100300			OREGON COAST HY	35TH ST	000000000000					OREGON COAST HY	35TH ST	00000000000			OREGON COAST HY	35TH ST	00000000000		
	M		K LAT LONG	04/07/2018	SA	42 43 59 48.99 -124 6	1		05/10/2018 14	TH	3P 43 59 48.99 -124 6	6T:0				09/05/2018 14	WE	2P 43 59 48.99 -124 6	1		04/18/2018 14	WE	7P 43 59 49.03 -124 6	9. T.8	
S D M	P R C S	AUIC	E E E E E E E E E E E E E E E E E E E	NNN	NO RPT	N N			01323 N N N	NO RPT	zz					02744 N N N	NO RPT	z z			01092 N N N	CITY	z z		

OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT URBAN NON-SYSTEM CRASH LISTING

35TH ST at OREGON COAST HY, City of Florence, Lane County, 01/01/2013 to 12/31/2018

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### ACTION CODE TRANSLATION LIST

ACTION	SHORT DESCRIPTION	LONG DESCRIPTION
000	NONE	NO ACTION OR NON-WARRANTED
001	SKIDDED	SKIDDED
002	ON/OFF V	GETTING ON OR OFF STOPPED OR PARKED VEHICLE
003	LOAD OVR	OVERHANGING LOAD STRUCK ANOTHER VEHICLE, ETC.
900	SLOW DN	SLOWED DOWN
007	AVOIDING	AVOIDING MANEUVER
800	PAR PARK	PARALLEL PARKING
600	ANG PARK	ANGLE PARKING
010	INTERFERE	PASSENGER INTERFERING WITH DRIVER
011	STOPPED	STOPPED IN TRAFFIC NOT WAITING TO MAKE A LEFT TURN
012	STP/L TRN	STOPPED BECAUSE OF LEFT TURN SIGNAL OR WAITING, ETC.
013	STP TURN	STOPPED WHILE EXECUTING A TURN
014	EMR V PKD	EMERGENCY VEHICLE LEGALLY PARKED IN THE ROADWAY
015	GO A/STOP	PROCEED AFTER STOPPING FOR A STOP SIGN/FLASHING RED.
016	TRN A/RED	TURNED ON RED AFTER STOPPING
017	LOSICIRL	LOST CONTROL OF VEHICLE
018	EXIT DWY	ENTERING STREET OR HIGHWAY FROM ALLEY OR DRIVEWAY
019	ENTR DWY	ENTERING ALLEY OR DRIVEWAY FROM STREET OR HIGHWAY
020	STR ENTR	BEFORE ENTERING ROADWAY, STRUCK PEDESTRIAN, ETC. ON SIDEWALK OR SHOULDER
021	NO DRVR	CAR RAN AWAY - NO DRIVER
022	PREV COL	STRUCK, OR WAS STRUCK BY, VEHICLE OR PEDESTRIAN IN PRIOR COLLISION BEFORE ACC. STABILIZED
023	STALLED	VEHICLE STALLED OR DISABLED
024	DRVR DEAD	DEAD BY UNASSOCIATED CAUSE
025	FATIGUE	FATIGUED, SLEEPY, ASLEEP
026	SUN	DRIVER BLINDED BY SUN
027	HDLGHTS	DRIVER BLINDED BY HEADLIGHTS
028	ILLNESS	PHYSICALLY ILL
029	THRU MED	VEHICLE CROSSED, PLUNGED OVER, OR THROUGH MEDIAN BARRIER
030	PURSUIT	PURSUING OR ATTEMPTING TO STOP A VEHICLE
031	PASSING	PASSING SITUATION
032	PRKOFFRD	VEHICLE PARKED BEYOND CURB OR SHOULDER
033	CROS MED	VEHICLE CROSSED BARTH OR GRASS MEDIAN
034	X N/SGNL	CROSSING AT INTERSECTION - NO TRAFFIC SIGNAL PRESENT
035	X W/ SGNL	CROSSING AT INTERSECTION - TRAFFIC SIGNAL PRESENT
036	DIAGONAL	CROSSING AT INTERSECTION - DIAGONALLY
037	BTWN INT	CROSSING BETWEEN INTERSECTIONS
038	DISTRACT	DRIVER'S ATTENTION DISTRACTED
039	W/TRAF-S	RIDING, ETC., ON
040	A/TRAF-S	WALKING, RUNNING, RIDING, ETC., ON SHOULDER FACING TRAFFIC
041	W/TRAF-P	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT WITH TRAFFIC
042	A/TRAF-P	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT FACING TRAFFIC
043	PLAYINRD	PLAYING IN STREET OR ROAD
044	PUSH MV	PUSHING OR WORKING ON VEHICLE IN ROAD OR ON SHOULDER
045	WORK ON	WORKING IN ROADWAY OR ALONG SHOULDER
046	W/ TRAFIC	NON-MOTORIST WALKING, RUNNING, RIDING, ETC. WITH TRAFFIC
047	A/ TRAFIC	NON-MOTORIST WALKING, RUNNING, RIDING, ETC. FACING TRAFFIC
020	LAY ON RD	STANDING OR LYING IN ROADWAY
051	ENT OFFRD	ENTERING / STARTING IN TRAFFIC LANE FROM OFF ROAD
052	MERGING	MERGING
055	SPRAY	BLINDED BY WATER SPRAY

### ACTION CODE TRANSLATION LIST

LONG DESCRIPTION	OTHER ACTION UNKNOWN ACTION
DESCRIPTION	OTHER UNK
CODE	088

### CAUSE CODE TRANSLATION LIST

CAUSE	SHORT		COLL	SHORT	
CODE	DESCRIPTION	LONG DESCRIPTION	CODE	DESCRIPTION	LONG DES
00	NO CODE	NO CAUSE ASSOCIATED AT THIS LEVEL	υð	OTH	MISCELL?
0.1	TOO-FAST	TOO FAST FOR CONDITIONS (NOT EXCEED POSTED SPEED	1	BACK	BACKING
02	NO-YIELD	DID NOT YIELD RIGHT-OF-WAY	0	PED	PEDESTR
03	PAS-STOP	PASSED STOP SIGN OR RED FLASHER	1	ANGL	ANGLE
0.4	DIS SIG	DISREGARDED TRAFFIC SIGNAL	2	HEAD	HEAD-ON
05	LEFT-CTR	DROVE LEFT OF CENTER ON TWO-WAY ROAD; STRADDLING	e	REAR	REAR-ENI
90	IMP-OVER	IMPROPER OVERTAKING	4	SS-M	SIDESWI
0.7	TOO-CLOS	FOLLOWED TOO CLOSELY	Ω	25-0	SIDESWI
80	IMP-TURN	MADE IMPROPER TURN	9	TURN	TURNING
60	DRINKING	ALCOHOL OR DRUG INVOLVED	_	PARK	PARKING
1.0	OTHR-IMP	OTHER IMPROPER DRIVING	00	NCOL	NON-COL
11	MECH-DEF	MECHANICAL DEFECT	6	FIX	FIXED OF
12	OTHER	OTHER (NOT IMPROPER DRIVING)			
13	IMP LN C	IMPROPER CHANGE OF TRAFFIC LANES			
14	DIS ICD	DISREGARDED OTHER TRAFFIC CONTROL DEVICE			
15	WRNG WAY	WRONG WAY ON ONE-WAY ROAD; WRONG SIDE DIVIDED RO			
16	FATIGUE	DRIVER DROWSY/FATIGUED/SLEEPY			
17	ILLNESS	PHYSICAL ILLNESS			
18	IN RDWY	NON-MOTORIST ILLEGALLY IN ROADWAY			
19	NT VISBL	NON-MOTORIST NOT VISIBLE; NON-REFLECTIVE CLOTHIN			
20	IMP PKNG	VEHICLE IMPROPERLY PARKED		DAGO HOD	נסה שחררת שמעיה שמגמת
21	DEF STER	DEFECTIVE STEERING MECHANISM		arr news	
22	DEF BRKE	INADEQUATE OR NO BRAKES	CRASH	SHORT	
24	LOADSHFT	VEHICLE LOST LOAD OR LOAD SHIFTED	TYPE	DESCRIPTION	LONG D
25	TIREFAIL	TIRE FAILURE	·	Marringary	THEFT
26	PHANTOM	PHANTOM / NON-CONTACT VEHICLE	১ ৫	NOW COLI	OVERIO
27	INATTENT	INATTENTION	) r	NON-COLL	OIRER
28	NM INATT	NON-MOTORIST INATTENTION	<b>⊣</b> (	OTH KDWY	MOTOR
29	F AVOID	FAILED TO AVOID VEHICLE AHEAD	7 (	PKKU MV	PAKKED
30	SPEED	DRIVING IN EXCESS OF POSTED SPEED	η,	PED	PEDEST
31	RACING	SPEED RACING (PER PAR)	4 4	TKAIN	KALLWA
32	CARELESS	CARELESS DRIVING (PER PAR)	n 0	BIKE	PEDALC
33	RECKLESS	RECKLESS DRIVING (PER PAR)	~ 0	ANIMAL	ANIMAL
34	AGGRESV	AGGRESSIVE DRIVING (PER PAR)	ω α	FIX OBJ	FIXED
35	RD RAGE	ROAD RAGE (PER PAR)	י ת	OTH OBJ	OTHER
40	VIEW OBS	VIEW OBSCURED	⊲C (	ANGL-STP	ENTERL
5.0	TISED MDN	IMPROPER USE OF MEDIAN OR SHOULDER	ŋ	ANGL-OTH	ENTERL
5, 12	FAII, I'N	FATTED TO MAINTAIN LANE	U	S-STRGHT	FROM S
) L	OFF DD	DAN ORF DOAD	Д	S-1TURN	FROM S
7	OF F. P.D.	NAN OFF NORD	口	S-1STOP	FROM S
			ш	S-OTHER	FROM S
			ტ	O-STRGHT	FROM C
			H	O-1 L-TURN	FROM C
			П	O-1STOP	FROM C
			p	O-OTHER	FROM C

## COLLISION TYPE CODE TRANSLATION LIST

			COULTSION LIFE	CORDISION LIFE CODE INTROPRILON HIST
TION	LONG DESCRIPTION	COLL	SHORT DESCRIPTION	LONG DESCRIPTION
	NO CAUSE ASSOCIATED AT THIS LEVEL	ઝ	OTH	MISCELLANEOUS
E	r)	ı	BACK	BACKING
9	- <	0	PED	PEDESTRIAN
)P	PASSED STOP SIGN OR RED FLASHER	T	ANGL	ANGLE
r.D.	DISREGARDED TRAFFIC SIGNAL	2	HEAD	HEAD-ON
Ä.	DROVE LEFT OF CENTER ON TWO-WAY ROAD; STRADDLING	m	REAR	REAR-END
E.		4	SS-M	SIDESWIPE - MEETING
53	FOLLOWED TOO CLOSELY	Ŋ	SS-0	SIDESWIPE - OVERTAKING
2 2	MADE TMPROPER TITRN	9	TURN	TURNING MOVEMENT
1 5	A COLOT OF PRICE THROUGH	7	PARK	PARKTNG MANEITVER
Ď É	ALCOHOL ON DANG INVOLVED	- 00	NCOL	NON-ICOLITE TO STORM
됩	OLHER IMPROPER DRIVING	0 0	20 1	TOTAL OF THE OF
Eq.	MECHANICAL DEFECT	v	r i A	FIARD OBOECT ON CIREN OBOECT
	OTHER (NOT IMPROPER DRIVING)			
S	IMPROPER CHANGE OF TRAFFIC LANES			
_	DISREGARDED OTHER TRAFFIC CONTROL DEVICE			
Ϋ́	WRONG WAY ON ONE-WAY ROAD; WRONG SIDE DIVIDED RO.			
Fu <sup>2</sup>	DRIVER DROWSY/FATIGUED/SLEEPY			
7.0	PHYSICAL ILLNESS			
	NON-MOTORIST TILEGALLY IN ROADWAY			
	-			
3 5	NON MOTORITY NOT VISIBLE, NON MET BECTIVE CHOIMEN			
ا چ	VERLCEE IMPROPERED		CRASH TYP	CRASH TYPE CODE TRANSLATION LIST
H.	DEFECTIVE STEERING MECHANISM			
E	INADEQUATE OR NO BRAKES	CRASH	SHORT	
I.	VEHICLE LOST LOAD OR LOAD SHIFTED	TYPE	DESCRIPTION	LONG DESCRIPTION
1	TIRE FAILURE		Marrimagra	- danatim-daiso
4	PHANTOM / NON-CONTACT VEHICLE	ð (	OVERTORN	OVERLORNED
	TNATTENTON	0	NON-COLL	OTHER NON-COLLISION
! F	NOTHINGHERN TO THE MANAGEMENT OF THE MANAGEMENT	1	OTH RDWY	MOTOR VEHICLE ON OTHER ROADWAY
1. /	NON-MOLORISI INALIENILON	2	PRKD MV	PARKED MOTOR VEHICLE
_	FAILED TO AVOID VEHICLE AMEAD	m	PED	PEDESTRIAN
	DRIVING IN EXCESS OF POSTED SPEED	4	TRAIN	RAILWAY TRAIN
	SPEED RACING (PER PAR)	9	BIKE	PEDALCYCLIST
SO.	CARELESS DRIVING (PER PAR)	7	ANIMAL	ANIMAL
S	RECKLESS DRIVING (PER PAR)	α	FTY OB.T	TAKED ORTEON
_	AGGRESSIVE DRIVING (PER PAR)	0	OTH OBI	CHERT OF THE PERSON OF THE PER
Fv7	ROAD RAGE (PER PAR)	۱ ر	OTH OTHER	CHILD OBOBOL  ONE THAT ARE ANOTHER COMPANY OF THE CHARLES
38	VIEW OBSCURED	ď í	ANGL-STP	ENTEKING AT ANGLE - ONE VEHICLE STOPPED
NC	TMPROPER USE OF MEDIAN OR SHOULDER	ជ	ANGL-OTH	
		O	S-STRGHT	SAME DIRECTION
-	FALLED TO MAINTAIN LAINE	Д	S-1TURN	FROM SAME DIRECTION - ONE TURN, ONE STRAIGHT
	KAN OFF KOAD	臼	S-1STOP	FROM SAME DIRECTION - ONE STOPPED
		[II.4	S-OTHER	FROM SAME DIRECTION-ALL OTHERS, INCLUDING PARKING
		Ŋ	O-STRGHT	FROM OPPOSITE DIRECTION - BOTH GOING STRAIGHT
		н	O-1 L-TURN	FROM OPPOSITE DIRECTION-ONE LEFT TURN, ONE STRAIGHT
		Ι	O-1STOP	FROM OPPOSITE DIRECTION - ONE STOPPED
		Þ	O-OTHER	FROM OPPOSITE DIRECTION-ALL OTHERS INCL. PARKING

## DRIVER LICENSE CODE TRANSLATION LIST

# DRIVER RESIDENCE CODE TRANSLATION LIST

LIC	SHORT	LONG DESCRIPTION	RES	SHORT	LONG DESCRIPTION
C	NONE	NOT LICENSED (HAD NEVER BEEN LICENSED)	Т	OR<25	OREGON RESIDENT WITHIN 25 MILE OF HOME
·	OR-Y	VALID OREGON TICENSE	2	OR>25	OREGON RESIDENT 25 OR MORE MILES FROM HOME
7	X-HLO	VALID LICENSE, OTHER STATE OR COUNTRY	m	OR-2	OREGON RESIDENT - UNKNOWN DISTANCE FROM HO
m	SUSP	SUSPENDED/REVOKED	4	N-RES	NON-RESIDENT
4	EXP	EXPIRED	O	UNK	UNKNOWN IF OREGON RESIDENT
80	N-VAL	OTHER NON-VALID LICENSE			
đ	IINIK	THIS GO GO THE TA CENSEL SAM CENTED WAS			

### ERROR CODE TRANSLATION LIST

амайара б жж ненякочевки ов соркичай	WIDE TURN CUT CORNER ON TURN CUT CORNER ON TURN EFALLED TO OBEY MANDATORY TRAFFIC TURN SIGNAL, SIGN OR LANE MARKINGS LEFT TURN IN FRONT OF ONCOMING TRAFFIC LEFT TURN WHENE PROHIBITED TURNED FROW WRONG LANE TURNED INTO WRONG LANE O-TURNED ILLEGALLY IMPROPERLY STOPPED IN TRAFFIC LANE
	WATORY TRAFFIC TURN SIGNAL, SIGN OR LANE MARKINGS F OF ONCOMING TRAFFIC LANE LANE LANE LANE LANE LANE LANE LANE
	NDATORY FRAFFIC TURN SIGN OR LANE MARKINGS F OF ONCOMING TRAFFIC LANE LANE LANE LANE LANE LANE LANE LANE
	T OF ONCOMING TRAFFIC ACHIBITED LANE LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE  LANE
	VALIBILED LANE LANE  (
	LANE  IN TRAFFIC LANE
	IN TRAFFIC LANE
	) IN TRAFFIC LANE
	RAILURE TO SIGNAL
	( NOT PARKING)
	IMPROPER START LEAVING PARKED POSITION
	OM STOPPED POSITION
	IMPROPER OR NO LIGHTS (VEHICLE IN TRAFFIC)
	INATTENTION (FAILURE TO DIM LIGHTS PRIOR TO 4/1/97)
	DRIVING UNSAFE VEHICLE (NO OTHER ERROR APPARENT)
	ENTERING/EXITING PARKED POSITION W/ INSUFFICIENT CLEARANCE; OTHER IMPROPER PARKING MANEUVER
	DRIVER'S SIGNAL
	IC SIGNAL
	DISREGARDED STOP SIGN OR FLASHING RED
	DISREGARDED WARNING SIGN, FLARES OR FLASHING AMBER
	DISREGARDED POLICE OFFICER OR FLAGMAN
	DISREGARDED SIREN OR WARNING OF EMERGENCY VEHICLE
	DISREGARDED RR SIGNAL, RR SIGN, OR RR FLAGMAN
	FAILED TO AVOID STOPPED OR PARKED VEHICLE AHEAD OTHER THAN SCHOOL BUS
	DID NOT HAVE RIGHT-OF-WAY OVER PEDALCYCLIST
	P-OF-WAY
	FAILED TO YIELD RIGHT-OF-WAY TO PEDESTRIAN
	ONG SIDE
	PASSING ON STRAIGHT ROAD UNDER UNSAFE CONDITIONS
	PASSED VEHICLE STOPPED AT CROSSWALK FOR PEDESTRIAN
	SCTION
	DF HILL
	SSING" ZONE
	IN FRONT OF ONCOMING TRAFFIC
COLL-IN COLLING IN (IWO LAND	CUTTING IN (TWO LANES - TWO WAY ONLY)
WRNGSIDE DRIVING ON WRONG SII	DRIVING ON WRONG SIDE OF THE ROAD (2-WAY UNDIVIDED ROADWAYS)
THRU MED DRIVING THROUGH SAFI	DRIVING THROUGH SAFETY ZONE OR OVER ISLAND
**	S SCHOOL BUS

### ERROR CODE TRANSLATION LIST

	MOVING VEHICLE	PRICER'S REPORT)			SIDE DIVIDED ROAD	EXCEEDING POSTED SPEED)	ANE						SIGNAL PRESENT	GNAL PRESENT	X.		ON SHOULDER WITH TRAFFIC	SHOULDER FACING TRAFFIC	ON PAVEMENT WITH TRAFFIC	PAVEMENT FACING TRAFFIC		AD OR ON SHOULDER	~		BY NON-MOTORIST								TEHICLE WITH CARGO OR PASSENGERS	SREGARDED TRAFFIC CONTROL DEVICE
FULL DESCRIPTION	FAILED TO DECREASE SPEED FOR SLOWER MOVING VEHICLE	FOLLOWING TOO CLOSELY (MUST BE ON OFFICER'S REPORT)	STRADDLING OR DRIVING ON WRONG LANES	IMPROPER CHANGE OF TRAFFIC LANES	WRONG WAY ON ONE-WAY ROADWAY; WRONG SIDE DIVIDED ROAD	DRIVING TOO FAST FOR CONDITIONS (NOT EXCEEDING POSTED SPEED)	OPENED DOOR INTO ADJACENT TRAFFIC LANE	IMPEDING TRAFFIC	DRIVING IN EXCESS OF POSTED SPEED	RECKLESS DRIVING (PER PAR)	CARELESS DRIVING (PER PAR)	SPEED RACING (PER PAR)	CROSSING AT INTERSECTION, NO TRAFFIC SIGNAL PRESENT	CROSSING AT INTERSECTION, TRAFFIC SIGNAL PRESENT	CROSSING AT INTERSECTION - DIAGONALLY	CROSSING BETWEEN INTERSECTIONS	ETC.,	WALKING, RUNNING, RIDING, ETC., ON S	WALKING, RUNNING, RIDING, ETC., ON F	WALKING, RUNNING, RIDING, ETC., ON PAVEMENT	PLAYING IN STREET OR ROAD	PUSHING OR WORKING ON VEHICLE IN ROAD OR ON SHOULDER	WORKING IN ROADWAY OR ALONG SHOULDER	STANDING OR LYING IN ROADWAY	Ę	ELUDING / ATTEMPT TO ELUDE	FAILED TO NEGOTIATE A CURVE	FAILED TO MAINTAIN LANE	RAN OFF ROAD	DRIVER MISJUDGED CLEARANCE	OVER-CORRECTING	CODE NOT IN USE	OVERLOADING OR IMPROPER LOADING OF VEHICLE WITH CARGO OR PASSENGERS	UNABLE TO DETERMINE WHICH DRIVER DISREGARDED TRAFFIC CONTROL DEVICE
SHORT DESCRIPTION	F/SLO MV	TOO CLOSE	STRDL LN	IMP CHG	WRNG WAY	BASCRULE	OPN DOOR	IMPEDING	SPEED	RECKLESS	CARELESS	RACING	X N/SGNL	X W/SGNL	DIAGONAL	BTWN INT	W/TRAF-S	A/TRAF-S	W/TRAF-P	A/TRAF-P	PLAYINRD	PUSH MV	WORK IN RD	LAY ON RD	NM IMP USE	ELUDING	F NEG CURV	FAIL LN	OFF RD	NO CLEAR	OVRSTEER	NOT USED	OVRLOAD	UNA DIS TC
ERROR	042	043	044	045	046	047	048	049	020	051	052	053	054	055	056	057	059	090	061	0.62	063	064	065	0.40	071	073	079	080	081	082	083	084	085	097

### EVENT CODE TRANSLATION LIST

EVENT	SHORT DESCRIPTION	LONG DESCRIPTION
001 002 003 004	FEL/JUMP INTERFER BUG INTF INDRCT PED	OCCUPANT FELL, JUMPED OR WAS EJECTED FROM MOVING VEHICLE PASSENGER INTERFERED WITH DRIVER ANIMAL OR INSECT IN VEHICLE INTERFERED WITH DRIVER PEDESTRIAN INDIRECTLY INVOLVED (NOT STRUCK) WITH DEPRESTRY IN INTIRED STRUCK)
00000000000000000000000000000000000000	INDRCT BIK HITCHIKR PSNGR TOW ON/OFF V	T STRUCK)  D OR PUSHED ON CONV.  E (OCCUPANTS ONLY;
010 011 012 013	SUB OTRN MV PUSHD MV TOWED FORCED	OVERTURNED AFTER FIRST HARMFUL EVENT VEHICLE BEING PUSHED VEHICLE TOWED OR HAD BEEN TOWING ANOTHER VEHICLE VEHICLE FORCED BY IMPACT INTO ANOTHER VEHICLE, PEDALCYCLIST OR PEDESTRIAN
014 015 017 017	SET MOTN RR ROW LT RL ROW RR HIT V	VEHICLE SET IN MOTION BY NON-DRIVER (CHILD RELEASED BRAKES, ETC.) AT OR ON RAILROAD RIGHT-OF-WAY (NOT LIGHT RAIL) AT OR ON LIGHT-RAIL RIGHT-OF-WAY TRAIN STRUCK VEHICLE VEHICLE STRUCK TRAIN
019	HIT RR CAR JACKNIFE TRL OTRN	VEHICLE STRUCK RAILBOAD CAR ON ROADWAY JACKKNIFE, TRAILER OR TOWED VEHICLE STRUCK TOWING VEHICLE TRAILER OR TOWED VEHICLE OVERTURNED TRAILER CONNECTION PROVE
00000 00000 00000000000000000000000000	DETACH TRL V DOOR OPN WHEELOFF HOOD UP	ER CON HED TE LE DOC CAME FLEW U
028 030 032 032	LOAD SHIFT LIREFAIL PET LVSTOCK HORSE HPCREPTD	LOST LOAD, LOAD MOVED OR SHLFTED TIRE FAILURE PET: CAT, DOG AND SIMILAR STOCK: COW, CALE, BULL, STEER, SHEEP, ETC. HORSE, MULE, OR DONKEY
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	GAME DEER ELK ANML VEH CULVERT ATENUATN PK METER	MIND ANIMAL, GAME (INCLUDES BIRDS; NOT DEER OR ELK) DEER OR ELK, WAPITI ANIMAL-DEANNY VEHICLE CULVERT, OPEN LOW OR HIGH MANHOLE IMPACT ATTENUATOR PARKING MATER
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	CURKE JUGGLE GDRL END GARDRAIL BARRIER WALL BR RAIL BR RAIL BR RECOLMNI	CORB (ALSO NARALES ON BAILDES) JIGGLE BAR OR TRAFFIC SNAKE FOR CHANNELIZATION LEADING EDGE OF GUARDRAIL GUARD RAIL (NOT METAL MEDIAN BARRIER) MEDIAN BARRIER (RAISED OR METAL) RETAINING WALL OR TUNNEL WALL BRIDGE RAILING OR PARAPET (ON BRIDGE OR APPROACH) BRIDGE RAILING OR PARAPET (ON BRIDGE OR APPROACH) BRIDGE PILLAR OR COLUMN
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BR GIRDR ISLAND GORE POLE UNK POLE UTL ST LIGHT TRF SGNL SGN BRDG STOPSIGN OTH SIGN HYDRANT	

### EVENT CODE TRANSLATION LIST

LONG DESCRIPTION	DELINEATOR OR MARKER (REFLECTOR POSTS) MAILBOX TRRE, STUMP OR SHRUBS	TREE BRANCH OR OTHER VEGETATION OVERHEAD, ETC.	TEMPORARY SIGN OR BARRICADE IN ROAD, ETC.	PERMANENT SIGN OR BARRICADE IN/OFF ROAD	FOREIGN OBSTRUCTION/DEBRIS IN ROAD (NOT GRAVEL)	EQUIPMENT WORKING IN/OFF ROAD	LINCEUDES FARRED IRAILER, LOW OR SANDING EQUIPMENT	ROCK, BRICK OR OTHER SOLID WALL	OTHER BUMP (NOT SPEED BUMP), POTHOLE OR PAVEMENT IRREGULARITY (PER PAR) OTHER OVERHEAD OBJECT (HIGHWAY SIGN, SIGNAL HEAD, ETC.); NOT BRIDGE	ROAD CAVE IN	AND ANK SIND ANK SIND ANK	LOW OR HIGH SHOULDER AT PAVEMENT EDGE	CUI SLOPE, OK DITCH EMBANKBATI STRINGS BY BOOK OR OWHER ORLFOW SET IN MOTION BY OFFIER VEHICLE (FNOT. LOGY LODDS)	ER MOVING OR FLYING OBJECT	VEHICLE OBSCURED VIEW	VEGETATION OBSCURED TEM VIEW OBSCURED BY FENCE, SIGN, PHONE BOOTH, ETC.	WIND GUST	VEHICLE IMMERSED IN BODY OF WATER	FENCE ON BUILDING, ETC.	CRASH RELATED TO ANOTHER SEPARATE CRASH	TWO-WAY INSEPTIC ON DIVIDED KOADWAY ALL KOUTED TO ONE SIDE	OTHER (PHANTOM) NON-CONTACT VEHICLE	CELL PHONE (ON PAR OR DRIVER IN USE)	TEBNAGE DRIVER IN VIOLATION OF GRADUATED LICENSE PGM	GOI WIKE BERN (FIRFHEN OR GRAVET, MOTIND)	GRAVEL IN ROADWAY	ABRUPT EDGE	CELL PHONE USE WITHOUS SED BY OTHER PARTICIPANT	FIAD CABOLI, UNIVON LIED NOM-FIXED OBJECT, OTHER OR UNIVON TYPE	TEXTING	WORK ZONE WORKER	FASSENGER KIDING ON VEHICLE EXIEKTOR PASSENGER RIDING ON PEDALCYCLE	PEDESTRIAN IN NON-MOTORIZED WHEELCHAIR	PEDESTRIAN IN MOTORIZED WHEELCHAIR IAM FNEOROFMFNIT / POLICE OFFICER	"SUB-BIKE": PEDALCYCLIST INJURED SUBSEQUENT TO COLLISION, ETC.	NON-MOTORIST STRUCK VEHICLE	STREET CAR/TROLLEY (ON RAILS OR OVERHEAD WIRE SYSTEM) STRUCK VEHICLE VEHICLE STRUCK STREET CAR/TROLLEY (ON RAILS OR OVERHEAD WIRE SYSTEM)	AT OR ON STREET CAR OR TROLLEY RIGHT-OF-WAY	VEHICLE STRUCK RAILROAD EQUIPMENT (NOT TRAIN) ON TRACKS DISTRACTED BY NAVIGATION SYSTEM OR GPS DEVICE	DISTRACTED BY OTHER ELECTRONIC DEVICE RAIL CROSSING DROP-ARM GATE
SHORT	MARKER MAILBOX TREE	VEG OHED	TEMP SGN	PERM SGN	FRGN OBJ	EQP WORK	MAIN EQP	OTHER WALL	IRRGL PVMT OVERHD OBJ	CAVE IN	HI WATER SNO BANK	LO-HI EDGE	DITCH OBJ FRM MV	FLY-OBJ	VEH HID	VEG AID BLDG HID	WIND GUST	IMMERSED FIRF/FYD	FENC/BLD	OTHR CRASH	TO I SIDE	PHANTOM	CELL PHONE	VIOL GDL	GOI WIKE Berm	GRAVEL	ABR EDGE	CELL WINSD	OTHER OBJ	TEXTING	WZ WORKER	ON VEHICLE PEDAL PSGR	MAN WHLCHR	MTR WHICHR	SUB-BIKE	N-MTR	S CAR VS V V VS S CAR	S CAR ROW	RR EQUIP DSTRCT GPS	DSTRCT OTH RR GATE
EVENT	060 061 062	063	065	066	068	069	071	072	07.4	075	0 / 0	0.78	6/.0	081	082	084	085	086	088	680	090	092	093	094	260 960	760	860	000	101	102	103	105	106	107	109	110	111	113	114	116

### EVENT CODE TRANSLATION LIST

EVENT SHORT

CODE	CODE DESCRIPTION	LONG DESCRIPTION
118	EXPNSN JNT	EXPANSION JOINT
119	JERSEY BAR	JERSEY BARKIER
120	WIRE BAR	WIRE OR CABLE MEDIAN BARRIER
121	FENCE	FENCE
123	OBJ IN VEH	LOOSE OBJECT IN VEHICLE STRUCK OCCUPANT
124	SLIPPERY	SLIDING OR SWERVING DUE TO WET, ICY, SLIPPERY OR LOOSE SURFACE (NOT GRAVEL)
125	SHLDR	SHOULDER GAVE WAY
126	BOULDER	ROCK(S), BOULDER (NOT GRAVEL; NOT ROCK SLIDE)
127	LAND SLIDE	ROCK SLIDE OR LAND SLIDE
128	CURVE INV	CURVE PRESENT AT CRASH LOCATION
129	HILL INV	VERTICAL GRADE / HILL PRESENT AT CRASH LOCATION
130	CURVE HID	VIEW OBSCURED BY CURVE
131	HILL HID	VIEW OBSCURED BY VERTICAL GRADE / HILL
132	WINDOW HID	VIEW OBSCURED BY VEHICLE WINDOW CONDITIONS
133	SPRAY HID	VIEW OBSCURED BY WATER SPRAY
134	TORRENTIAL	TORRENTIAL RAIN (EXCEPTIONALLY HEAVY RAIN)

## FUNCTIONAL CLASSIFICATION TRANSLATION LIST

CLASS	DESCRIPTION	
0.1	RURAL PRINCIPAL ARTERIAL - IN	INTERSTATE
02	RURAL PRINCIPAL ARTERIAL - OT	OTHER
90	RURAL MINOR ARTERIAL	
0.7	RURAL MAJOR COLLECTOR	
80	RURAL MINOR COLLECTOR	
60	RURAL LOCAL	
11	URBAN PRINCIPAL ARTERIAL - IN	INTERSTATE
12	URBAN PRINCIPAL ARTERIAL - OT	OTHER FREEWAYS AND EXP
14	URBAN PRINCIPAL ARTERIAL - OT	OTHER
16	URBAN MINOR ARTERIAL	
17	URBAN MAJOR COLLECTOR	
18	URBAN MINOR COLLECTOR	
19	URBAN LOCAL	
78	UNKNOWN RURAL SYSTEM	
79	UNKNOWN RURAL NON-SYSTEM	
86	UNKNOWN URBAN SYSTEM	
0	MUTANTIN MARKING MARKET MARKET	

## INJURY SEVERITY CODE TRANSLATION LIST

		EDING, BROKEN BONES		OF PAIN		AGE.	THE AGE OF 4
LONG DESCRIPTION	FATAL INJURY	INCAPACITATING INJURY - BLEEDING, BROKEN BONES	NON-INCAPACITATING INJURY	POSSIBLE INJURY - COMPLAINT OF PAIN	DIED PRIOR TO CRASH	NO INJURY - 0 TO 4 YEARS OF	PARTICIPANT UNINJURED, OVER THE
SHORT	KILL	INJA	INJB	INJC	PRI	NO<5	NONE
CODE	_	2	m	4	Ŋ	7	0

### MEDIAN TYPE CODE TRANSLATION LIST

	SHORT	
CODE	DESC	LONG DESCRIPTION
0	NONE	NO MEDIAN
$\vdash$	RSDMD	SOLID MEDIAN BARRIER
7	DIVMD	EARTH, GRASS OR PAVED MEDIAN

### HIGHWAY COMPONENT TRANSLATION LIST

CODE DESCRIPTION

0	MAINLINE	STATE	HIGHWAY
Н	COUPLET		
m	FRONTAGE	ROAD	
9	CONNECTION	NC	
ω	HIGHWAY	- OTHER	

## LIGHT CONDITION CODE TRANSLATION LIST

CODE	DESC DESC UNK DAY	LONG DESCRIPTION UNKNOWN DAYLIGHT DAPRINGS - WITH STREET LIGHTS
1 ധ 4 ര	DARK DAWN DUSK	- 55

### MILEAGE TYPE CODE TRANSLATION LIST

LONG DESCRIPTION	REGULAR MILEAGE	TEMPORARY	SPUR	OVERLAPPING
CODE	0	H	X	Z

## MOVEMENT TYPE CODE TRANSLATION LIST SHORT

	LONG DESCRIPTION	UNKNOMN	STRAIGHT AHEAD	TURNING RIGHT	TURNING LEFT	MAKING A U-TURN	BACKING	STOPPED IN TRAFFIC	PARKED - PROPERLY	PARKED - IMPROPERLY	PARKING MANEUVER
SHOW	DESC	UNK	STRGHT	TURN-R	TURN-L	U-TURN	BACK	STOP	PRKD-P	PRKD-I	PARKNG
	CODE	0	$\vdash$	2	m	4	2	9	7	∞	0

# NON-MOTORIST LOCATION CODE TRANSLATION LIST

## ROAD CHARACTER CODE TRANSLATION LIST

	LONG DESCRIPTION	UNKNOMN	INTERSECTION	DRIVEWAY OR ALLEY	STRAIGHT ROADWAY	TRANSITION	CURVE (HORIZONTAL CURVE)	OPEN ACCESS OR TURNOUT	GRADE (VERTICAL CURVE)	BRIDGE STRUCTURE	TUNNET
SHORT	DESC	UNK	INTER	ALLEY	STRGHT	TRANS	CURVE	OPENAC	GRADE	BRIDGE	TUNNEL
	CODE	0	П	2	m	4	ιΩ	9	<u></u>	00	9

## PARTICIPANT TYPE CODE TRANSLATION LIST

LONG DESCRIPTION	UNKNOWN OCCUPANT TYPE	DRIVER	PASSENGER	PEDESTRIAN	PEDESTRIAN USING A PEDESTRIAN CONVEYAL	PEDESTRIAN TOWING OR TRAILERING AN OB	PEDALCYCLIST	PEDALCYCLIST TOWING OR TRAILERING AN	OCCUPANT OF A PARKED MOTOR VEHICLE	UNKNOWN TYPE OF NON-MOTORIST
SHORT	220	DRVR	PSNG	PED	CONV	PTOW	BIKE	BTOW	PRKD	UNK
CODE	0	Ţ	2	e	4	2	9	7	00	0

# TRAFFIC CONTROL DEVICE CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
000	NONE	NO CONTROL
001	TRF SIGNAL	TRAFFIC SIGNALS
002	FLASHBCN-R	FLASHING BEACON - RED (STOP)
003	FLASHBCN-A	FLASHING BEACON - AMBER (SLOW)
004	STOP SIGN	STOP SIGN
002	SLOW SIGN	STOW SIGN
900	REG-SIGN	REGULATORY SIGN
007	YIELD	YIELD SIGN
800	WARNING	WARNING SIGN
600	CURVE	CURVE SIGN
010	SCHL X-ING	SCHOOL CROSSING SIGN OR SPECIAL SIGNAL
011	OFCR/FLAG	POLICE OFFICER, FLAGMAN - SCHOOL PATROL
012	BRDG-GATE	BRIDGE GATE - BARRIER
013	TEMP-BARR	TEMPORARY BARRIER
014	NO-PASS-ZN	NO PASSING ZONE
015	ONE-WAY	ONE-WAY STREET
016	CHANNEL	CHANNELIZATION
017	MEDIAN BAR	MEDIAN BARRIER
018	PILOT CAR	PILOT CAR
019	SP PED SIG	SPECIAL PEDESTRIAN SIGNAL
020	X-BUCK	CROSSBUCK
021	THR-GN-SIG	THROUGH GREEN ARROW OR SIGNAL
022	L-GRN-SIG	LEFT TURN GREEN ARROW, LANE MARKINGS, OR SIGNAL
023	R-GRN-SIG	RIGHT TURN GREEN ARROW, LANE MARKINGS, OR SIGNAL
024	WIGWAG	WIGWAG OR FLASHING LIGHTS W/O DROP-ARM GATE
025	X-BUCK WRN	CROSSBUCK AND ADVANCE WARNING
026	WW W/ GATE	FLASHING LIGHTS WITH DROP-ARM GATES
027	OVRHD SGNL	SUPPLEMENTAL OVERHEAD SIGNAL (RR XING ONLY)
028	SP RR STOP	SPECIAL RR STOP SIGN
029	ILUM GRD X	ILLUMINATED GRADE CROSSING
037	RAMP METER	METERED RAMPS
038	RUMBLE STR	RUMBLE STRIP
060	L-TURN REF	LEFT TURN REFUGE (WHEN REFUGE IS INVOLVED)
091	R-TURN ALL	RIGHT TURN AT ALL TIMES SIGN, ETC.
092	EMR SGN/FL	EMERGENCY SIGNS OR FLARES
093	ACCEL LANE	ACCELERATION OR DECELERATION LANES
094	R-TURN PRO	RIGHT TURN PROHIBITED ON RED AFTER STOPPING
095	BUS STPSGN	BUS STOP SIGN AND RED LIGHTS
660	UNKNOWN	UNKNOWN OR NOT DEFINITE

### VEHICLE TYPE CODE TRANSLATION LIST

CODE	SHORT DESC	LONG DESCRIPTION
00	PDO	NOT COLLECTED FOR PDO CRASHES
01	PSNGR CAR	PASSENGER CAR, PICKUP, LIGHT DELIVERY, ETC.
0.5	BOBTAIL	TRUCK TRACTOR WITH NO TRAILERS (BOBTAIL)
03	FARM TRCTR	FARM TRACTOR OR SELF-PROPELLED FARM EQUIPMENT
04	SEMI TOW	TRUCK TRACTOR WITH TRAILER/MOBILE HOME IN TOW
0.5	TRUCK	TRUCK WITH NON-DETACHABLE BED, PANEL, ETC.
90	MOPED	MOPED, MINIBIKE, SEATED MOTOR SCOOTER, MOTOR BIKE
10	SCHI BUS	SCHOOL BUS (INCLUDES VAN)
80	OTH BUS	OTHER BUS
60	MTRCYCLE	MOTORCYCLE, DIRT BIKE
10	OTHER	OTHER: FORKLIFT, BACKHOE, ETC.
11	MOTRHOME	MOTORHOME
12	TROLLEY	MOTORIZED STREET CAR/TROLLEY (NO RAILS/WIRES)
13	ATV	ATV
14	MTRSCTR	MOTORIZED SCOOTER (STANDING)
15	SNOWMOBILE	SNOWMOBILE
66	UNKNOMN	UNKNOWN VEHICLE TYPE

## WEATHER CONDITION CODE TRANSLATION LIST

CODE SHORT DESC LONG DESCRIPTION	UNKNOMN	CLEAR	CLOUDY	RAIN	SLEET	FOG	SNOW	DUST	SMOKE	ASH
SHORT DESC	UNK	CLR	CID	RAIN	SLT	FOG	SNOW	DUST	SMOK	ASH
CODE	0	1	2	m	4	5	9	7	80	O

### **APPENDIX E**

### TRAFFIC COUNT DATA, ADJUSTMENTS AND GROWTH CALCULATIONS

# Gary's Traffic Data 310 Pitney Lane, Unit 39

### Junction City, OR 97448

Fast, Accurate, High Quality Counts

Weather: Sunny, clear File Name: FLORENCE 35th St. @ Hwy. 101

34 degrees F. AM, 60 PM Site Code: US 101 Collected By: G.Mc. Start Date : 3/19/2020

Page No : 1

**Groups Printed- Unshifted** 

										Printed	a- uns										,
		H	HWY 1	01				35TH	I			H	HWY 1	01				35TH	I		
		Fr	om No	orth			F	rom E	ast			Fr	om So	uth			Fi	rom W	est		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
07:00 AM	1	37	2	0	40	4	3	5	0	12	5	40	1	0	46	2	0	5	0	7	105
07:15 AM	1	37	5	0	43	1	2	1	0	4	6	41	1	0	48	4	0	10	0	14	109
07:30 AM	1	68	9	0	78	1	4	4	0	9	6	43	1	0	50	0	1	10	0	11	148
07:45 AM	2	74	8	0	84	6	4	2	0	12	5	61	3	0	69	6	1	10	0	17	182
Total	5	216	24	0	245	12	13	12	0	37	22	185	6	0	213	12	2	35	0	49	544
											1										ı
08:00 AM	3	56	7	0	66	7	3	1	0	11	3	53	2	0	58	5	0	17	0	22	157
08:15 AM	3	49	6	0	58	3	6	6	0	15	5	53	1	0	59	11	3	7	0	21	153
08:30 AM	2	65	8	0	75	2	3	6	0	11	6	52	0	0	58	10	1	8	0	19	163
08:45 AM	6	71_	7	0	84	6	4	7	0	17	11	66	0	0	77	17	4	25	0	46	224
Total	14	241	28	0	283	18	16	20	0	54	25	224	3	0	252	43	8	57	0	108	697
*** BREAK **	*																				
04:00 PM	2	106	9	0	118	6	2	F	^	14	15	129	10	0	154	16	_	24	0	45	331
04:00 PM	3	95	10	0	108	5	3 5	5 7	0	17	14	88	10	0	108	15	5	10	-	30	263
04:15 PM	9	95 117	11		137	2	5			8	17		6	0	136	15	5 3	16	0	34	
04:30 PM 04:45 PM	8	94	11	0 1	114	10	1	5 5	0	16	15	116 98	3 5	0	118	10	ა 3	19	0	34	315 280
Total	23	412	41	<u> </u>	477	23	10	22	0	55	61	431	24	0	516	56	<u></u>	69	0	<u>3∠</u> 141	1189
Total	20	712	71	'	7//	20	10	22	O	00	01	701	27	U	310	50	10	03	U	171	1105
05:00 PM	4	114	6	0	124	1	2	9	0	12	22	112	14	0	148	21	6	18	1	46	330
05:15 PM	4	106	12	0	122	5	6	9	0	20	17	120	5	0	142	9	7	19	0	35	319
05:30 PM	4	99	5	2	110	3	3	2	0	8	16	98	4	2	120	19	6	14	1	40	278
05:45 PM	4	97	6	0	107	1	2	6	0	9	13	72	1_	0	86	12	7	16	0	35	237
Total	16	416	29	2	463	10	13	26	0	49	68	402	24	2	496	61	26	67	2	156	1164
Grand Total	58	1285	122	3	1468	63	52	80	0	195	176	1242	57	2	1477	172	52	228	2	454	3594
Apprch %	4	87.5	8.3	0.2		32.3	26.7	41	Ö		11.9	84.1	3.9	0.1		37.9	11.5	50.2	0.4		
Total %	1.6	35.8	3.4	0.1	40.8	1.8	1.4	2.2	Ö	5.4	4.9	34.6	1.6	0.1	41.1	4.8	1.4	6.3	0.1	12.6	
	-			- '	1	-			-		,		-			-				-	'

# Gary's Traffic Data 310 Pitney Lane, Unit 39

### Junction City, OR 97448

Fast, Accurate, High Quality Counts

Weather: Sunny, clear 34 degrees F. AM, 60 PM

Collected By: G.Mc.

File Name: FLORENCE 35th St. @ Hwy. 101

Site Code: US 101 Start Date : 3/19/2020

Page No : 4

		H	HWY 1	01				35TH				ŀ	HWY 1	01				35TH	l		
		Fr	om No	orth			F	rom E	ast			Fr	om Sc	uth			Fi	rom W	est		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ar	nalysis	From (	07:00 A	AM to (	)5:45 PM	1 - Pea	k 1 of	1													
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:3	0 PM															
04:30 PM	9	117	11	0	137	2	1	5	0	8	17	116	3	0	136	15	3	16	0	34	315
04:45 PM	8	94	11	1	114	10	1	5	0	16	15	98	5	0	118	10	3	19	0	32	280
05:00 PM	4	114	6	0	124	1	2	9	0	12	22	112	14	0	148	21	6	18	1	46	330
05:15 PM	4	106	12	0	122	5	6	9	0	20	17	120	5	0	142	9	7	19	0	35	319
Total Volume	25	431	40	1	497	18	10	28	0	56	71	446	27	0	544	55	19	72	1	147	1244
% App. Total	5	86.7	8	0.2		32.1	17.9	50	0		13.1	82	5	0		37.4	12.9	49	0.7		
PHF	.694	.921	.833	.250	.907	.450	.417	.778	.000	.700	.807	.929	.482	.000	.919	.655	.679	.947	.250	.799	.942

Peak Hour Analysis From 07:00 AM to 05:45 PM - Peak 1 of 1

Peak Hour for	Each A	pproac	n Begii	ns at:																
	04:30 PM					04:30 PM					04:30 PM	Į				05:00 PM				
+0 mins.	9	117	11	0	137	2	1	5	0	8	17	116	3	0	136	21	6	18	1	46
+15 mins.	8	94	11	1	114	10	1	5	0	16	15	98	5	0	118	9	7	19	0	35
+30 mins.	4	114	6	0	124	1	2	9	0	12	22	112	14	0	148	19	6	14	1	40
+45 mins.	4	106	12	0	122	5	6	9	0	20	17	120	5	0	142	12	7	16	0	35
Total Volume	25	431	40	1	497	18	10	28	0	56	71	446	27	0	544	61	26	67	2	156
% App. Total	5	86.7	8	0.2		32.1	17.9	50	0		13.1	82	5	0		39.1	16.7	42.9	1.3	
PHF	.694	.921	.833	.250	.907	.450	.417	.778	.000	.700	.807	.929	.482	.000	.919	.726	.929	.882	.500	.848

# Florence, Oregon US 101 @ 35<sup>th</sup> Street Manual Truck Counts

#### Thursday 3/19/2020 7:00 to 9:00 AM and 4:00 to 6:00 PM

Truck					
Axles	7:00-7:15	7:15-7:30	7:30-7:45	7:45-8:00	
4	0	2 (2-11)	0	0	
5	1 (1-11)	0	3 (1-3, 2-11)	1 (1-3)	
6	0	0	1 (1-14)	2 (2-11)	
7	0	2 (1-3, 1-11)	0	0	
Totals	1	4	4	3	12
Truck					
Axles	8:00-8:15	8:15-8:30	8:30-8:45	8:45-9:00	
4	2 (1-2, 1-3)	1 (1-3)	0	0	
5	2 (2-3)	1 (1-11)	1 (1-3)	1 (1-11)	
6	0	0	0	1 (1-3)	
7	0	0	0	1 (1-11)	
Totals	4	2	1	3	10
				Total of 22 trucks i	n 2 hrs.

.....

Truck					
Axles	4:00-4:15	4:15-4:30	4:30-4:45	4:45-5:00	
4	0	0	0	1 (1-3)	
5	1 (1-14)	0	0	0	
6	0	0	0	0	
7	1 (1-11)	0	0	0	
Totals	2	0	0	1	3
Truck					
Axles	5:00-5:15	5:15-5:30	5:30-5:45	5:45-6:00	
4	0	0	0	0	
5	0	1 (1-10)	0	0	
6	0	0	2 (2-11)	0	
Totals	0	1	2	0	3
					_

Total of 6 trucks in 2 hrs.

#### **Grand Total of 16 heavy trucks during four peak hours**

#### **LEGEND of MOVEMENTS**

2 = Southbound on 101, turning right onto 35<sup>th</sup> St.

3 = Southbound Through on 101

10 = Northbound on 101, turning right onto 35<sup>th</sup> St.

11 = Northbound Through on 101

14 = Eastbound on 35<sup>th</sup>, turning right onto 101

G.Mc. 3/20/2020

#### Florence, Oregon Hwy. 101 @ 35<sup>th</sup> Street Manual Bicycle Counts

Thursday 3/19/2020 7:00 to 9:00 AM, 4:00 to 6:00 PM

	7:00-7:15	7:15-7:30	7:30-7:45	7:45-8:00	
	0	0	1 (1-3)	0	
Totals	0	0	1	0	1
	8:00-8:15	8:15-8:30	8:30-8:45	8:45-9:00	
	0	0	0	0	
Totals	0	0	0	0	0
	4:00-4:15	4:15-4:30	4:30-4:45	4:45-5:00	
	1 (1-15)	0	0	0	
	1	0	0	0	1
	5:00-5:15	5:15-5:30	5:30-5:45	5:45-6:00	
	3 (1-7, 2-15) 7 was W/W on S/W	2 (2-16) both used LTL	1 (1-3)	0	
Totals	3	2	1	0	6

Grand Total of 8 bicycles observed during two AM and two PM peak hours riding on both sides of Hwy. 101 or 35<sup>th</sup> St. and executing noted movements at the intersection.

#### **LEGEND of MOVEMENTS**

3 = Southbound Through on 101

7 = Westbound Through on 35<sup>th</sup>

15 = Eastbound Through on  $35^{th}$ 

16 = Eastbound on 35<sup>th</sup>, turning left onto 101

S/W = riding on sidewalk
W/W = Wrong way on wrong side of road
LTL = used channelized left turn lane

G.Mc. 3/20/2020

# Gary's Traffic Data 310 Pitney Lane, Unit 39

### Junction City, OR 97448

Fast, Accurate, High Quality Counts

Weather: Clear, sunny File Name: FLRNC 35th @ Redwood

34 degrees F. AM, 60 PM Site Code : Florence Collected By: MH Start Date : 3/19/2020

Page No : 1

<b>Groups Printed- Unshift</b>	ed
--------------------------------	----

		35TH	I CT			REDWOO				35TH	TOT		
		From				From				From	-		
Start Time	1 -44	Thru		nn Tatal	Left			nn Tatal	Th			Ann Tatal	Int Total
07:00 AM	Left	1 nru   8		pp. Total		Right 0		pp. Total 5	Thru	Right		App. Total	Int. Total
	0	8	0	8	5 2	0	0	-	0	1	0	1	14
07:15 AM	2	1	0	3	_	0	0	2	1	1	0	2	12
07:30 AM	0	6	0	6	3	1	0	4	2	1	0	3	13
07:45 AM	3	8	0	11	4	0	0	4	3	3	0	6	21
Total	5	23	0	28	14	1	0	15	6	6	0	12	55
08:00 AM	1	7	0	8	5	0	0	5	4	1	0	5	18
08:15 AM	4	8	2	14	7	1	0	8	0	7	0	7	29
08:30 AM	1	6	0	7	5	4	0	9	3	0	0	3	19
08:45 AM	4	10	0	14	7	1_	0	8	5	4	0	9	31_
Total	10	31	2	43	24	6	0	30	12	12	0	24	97
*** BREAK ***													
04:00 PM	0	7	0	7	10	2	0	12	11	4	0	15	34
04:15 PM	5	9	0	14	7	5	0	12	10	6	0	16	42
04:30 PM	0	6	0	6	4	2	0	6	6	12	0	18	30
04:45 PM	8	9	0	17	5	2	2	9	7	7	0	14	40
Total	13	31	0	44	26	11	2	39	34	29	0	63	146
05:00 PM	6	6	0	12	7	4	0	11	16	10	0	26	49
05:15 PM	5	8 2	0	13	10	3	0	13	9	8	0	17	43
05:30 PM	1	2	0	3	4	3	1	8	6	9	0	15	26
05:45 PM	2	2	0	4	7	1	0	8	5	6	0	11	23
Total	14	18	0	32	28	11	1	40	36	33	0	69	141
Grand Total	42	103	2	147	92	29	3	124	88	80	0	168	439
Apprch %	28.6	70.1	1.4	• • •	74.2	23.4	2.4		52.4	47.6	Ő	.00	100
Total %	9.6	23.5	0.5	33.5	21	6.6	0.7	28.2	20	18.2	0	38.3	

# Gary's Traffic Data 310 Pitney Lane, Unit 39

### Junction City, OR 97448

Fast, Accurate, High Quality Counts

Weather: Clear, sunny

34 degrees F. AM, 60 PM

Collected By: MH

File Name: FLRNC 35th @ Redwood

Site Code : Florence

Start Date : 3/19/2020

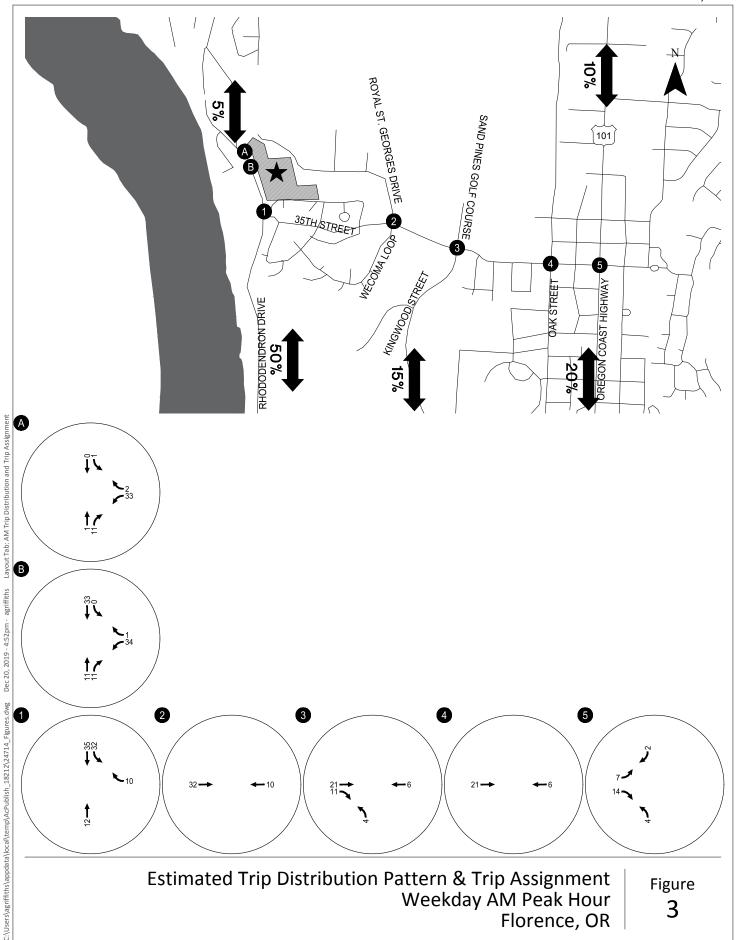
Page No : 4

		35TH	1 ST			REDWO	OD LOO	Р		35TI	H ST		
		From	East			From	South			From	West		
Start Time	Left	Thru	Peds /	App. Total	Left	Right	Peds	App. Total	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Analysis	From 07:0	0 AM to 0	5:45 PM -	Peak 1 of 1		_				-			
Peak Hour for Entire	e Intersecti	on Begins	at 04:30 F	PM									
04:30 PM	0	6	0	6	4	2	0	6	6	12	0	18	30
04:45 PM	8	9	0	17	5	2	2	9	7	7	0	14	40
05:00 PM	6	6	0	12	7	4	0	11	16	10	0	26	49
05:15 PM	5	8	0	13	10	3	0	13	9	8	0	17	43
Total Volume	19	29	0	48	26	11	2	39	38	37	0	75	162
% App. Total	39.6	60.4	0		66.7	28.2	5.1		50.7	49.3	0		
PHF	.594	.806	.000	.706	.650	.688	.250	.750	.594	.771	.000	.721	.827

Peak Hour Analysis From 07:00 AM to 05:45 PM - Peak 1 of 1

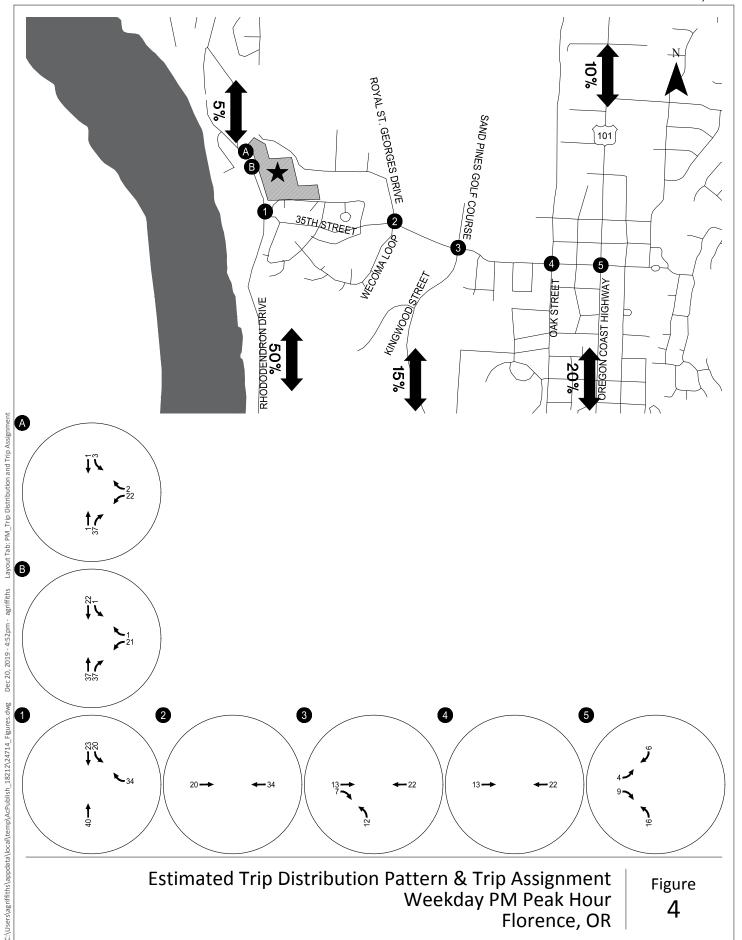
Peak Hour for Each	Approach E	Begins at:										
	04:15 PM				04:45 PM				04:30 PM			
+0 mins.	5	9	0	14	5	2	2	9	6	12	0	18
+15 mins.	0	6	0	6	7	4	0	11	7	7	0	14
+30 mins.	8	9	0	17	10	3	0	13	16	10	0	26
+45 mins.	6	6	0	12	4	3	1_	8	9	8	0	17
Total Volume	19	30	0	49	26	12	3	41	38	37	0	75
% App. Total	38.8	61.2	0		63.4	29.3	7.3		50.7	49.3	0	
PHF	.594	.833	.000	.721	.650	.750	.375	.788	.594	.771	.000	.721

Florence Residential Subdivision January 2019





Florence Residential Subdivision January 2019







								SEA	SONAL TRE	SEASONAL TREND TABLE (Updated: 6/26/19	(Updated	: 6/26/19)												Seasonal Frend
TREND	1-Jan	15-Jan	1-Feb	15-Feb	1-Mar	15-Mar	1-Apr	15-Apr	1-May	15-May	1-Jun	15-Jun	1-Jul	15-Jul 1	1-Aug 1	15-Aug 1	1-Sep 15-	15-Sep 1-	1-Oct 15-Oct	ct 1-Nov	v 15-Nov	lov 1-Dec	c 15-Dec	_
INTERSTATE URBANIZED	1.0419	1.0728	1.0640	1.0552	1.0259	9966.0	9686.0	0.9825	0.9768	0.9711	0.9558	0.9404	0.9561	0.9718 0	0.9804 0.	0 0686	5:0 0986:	0.9830	.9864 0.9897	37 1.0055	55 1.0213	13 1.0436	36 1.0659	0.9404
INTERSTATE NONURBANIZED	1.2583	1.3379	1.2962	1.2545	1.1572	1.0600	1.0383	1.0166	0.9863	0.9561	0.9075	0.8588	0.8422	0.8256 0	0.8325 0.	0.8394 0.	3.0 9088.0	0.9218 0.9	0.9559 0.9900	1.0158	Ľ	1.0416 1.1192	1.1969	0.8256
COMMUTER	1.0577	1.1050	1.0844	1.0638	1.0406	1.0173	0.9975	0.9777	0.9711	0.9645	0.9542	0.9438	0.9544	0.9649 0	0.9592 0.	0.9535 0.	9:0 2:00	9.9738 0.9	0.9737 0.9737	37 0.9976	-	.0215 1.0520	1.0825	0.9438
COASTAL DESTINATION	1.2069	1.2238	1.1889	1.1540	1.1006	1.0472	1.0504	1.0536	1.0125	0.9714	0.9394	0.9074	0.8574	0.8074 0	0.8100 0.	0.8126 0.	0.8635 0.9	0.9145 0.9	0.9648 1.0152	52 1.0683	-	1.1214 1.1636	36 1.2058	0.8074
COASTAL DESTINATION ROUTE	1.3738	1.4039	1.3653	1.3267	1.2268	1.1268	1.1203	1.1138	1.0308	0.9478	0.9031	0.8584	0.7781	0.6978 0	0.7080 0.	0.7182 0.	.7932 0.8	0.8682 0.9	0.9574 1.0466	1	.1248 1.2030	30 1.2836	36 1.3642	8/69.0
AGRICULTURE	1.4390	1.5042	1.4606	1.4171	1.3208	1.2246	1.1445	1.0643	0.9843	0.9043	0.8736	0.8429	0.8259	0   6808:0	0.8114 0.	0.8140 0.	.7847 0.7	0.7554 0.8	0.8267 0.8980	30 0.9879	1.0778	78 1.2559	1.4339	0.7554
RECREATIONAL SUMMER	1.6714	1.6739	1.6571	1.6403	1.4889	1.3375	1.2642	1.1909	1.0325 (	0.8742	0.8177	0.7611	0.7119	0.6626 0	0.6933 0.	0.7239 0.	0.7598 0.7	0.7957 0.8	0.8898 0.9838	38 1.1028	28 1.2218	1.3720	20 1.5221	0.6626
RECREATIONAL SUMMER WINTER	1.0752	0.9963	1.0200	1.0437	1.0500	1.0563	1.1766	1.2970	1.1496	1.0021	0.9514	9006'0	0.8005	0.7005 0	0.7590 0.	0.8176 0.	0.9133 1.0	1.1 1.000.	1.3532	32 1.4605	1.5677	77 1.2312	12 0.8948	0.7005
RECREATIONAL WINTER	0.8178	0.6528	0.7315	0.8102	0.8326	0.8549	1.0558	1.2566	1.1918	1.1270	1.1295	1.1321	1.0004	0.8687 0	0.9344 1.	.0001	.0823 1.1	1.1646   1.2	2984 1.4323	`	1.7685 2.1047	47 1.4326	26 0.7605	0.6528
SUMMER	1.2007	1.2609	1.2367	1.2125	1.1528	1.0932	1.0592	1.0252	0.9810	0.9368	0.9061	0.8753	0.8535	0.8317 0	0.8437 0.	0.8557 0.	0.8872 0.9	0.9188 0.9	0.9502 0.9816	1.0276	1.0737	37   1.1341	1.1945	0.8317
SUMMER < 2500	1.2437	1.3130	1.2858	1.2586	1.1886	1.1186	1.0667	1.0147	0.9592	0.9036	0.8816	0.8595	0.8489	0.8382 0	0.8564 0.	0.8746 0.	0.8721 0.8	5.0   9698.0	0.9094 0.9491	1.0234	34 1.0977	77 1.1930	30 1.2883	0.8382

'Seasonal Trend Table factors are based on previous year ATR data. The table is updated yearly.

\*Grey shading indicates months were seasonal factor is greater than 30%

March 15-31 Trend Seems to be right around 30%!!

March 19 Trend = [(1.0504-1.0472/4/17)+1.0472|= 1.047953 Sessonal Adjustment Farbr = Count Date Sessonal TrendPeak Sesson 3/19/2020 = 1.047953/0.8074 = 1.297953 = 29.80% 1.0472 16 Day Period March 15th Trend April 1 Trend 0.947493

Linear interpolation Calculations: Seas and Trend Values
15-Mar 12-Mar 18-Mar 12-Mar 12-Mar 21-Mar 22-Mar 22-Mar 24-Mar 24-Mar 25-Mar 26-Mar 22-Mar 28-Mar 28-Mar 30-Mar 31-Mar 11-Mar 18-Mar 12-Mar 18-Mar 12-Mar 18-Mar 11-Mar 18-Mar 11-Mar 18-Mar 11-Mar 18-Mar 11-Mar 11-Mar 18-Mar 11-Mar 1



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**ODOT FVT Values:** 

Site ID	НМУ	DIR	Description	2018	2038	RSQ	
1170	6	1	0.03 Mile South Munsel Lake Rd	9500	0096	0.2347	
1171	6	1	0.02 Mile South of 36th Street	12500	12600	0.4298	
1172	6	1	0.02 Mile South of 29th Street	14100	14200	0.805	RSQ OK

Average Annual 20 Year Growth Rate (AAGR) = (2038 ADT - 2018ADT)/(2018ADT \* 20 Years)

0.0004 1.0004 1.002128

0.0355% per year

AAGR =

6 year Growth Factor (2026 DH)

1 year Growth Factor (2021 DH)



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										<b>Missing Data</b>						
	Weekday	Differential	-0.21	-0.23	08'0-	-0.16	-0.25	-0.11	-0.19	N/A	-0.17	-0.18	-0.21	-0.17	-0.17	-0.19
npared to 2019		2019	656,882	262,471	142,360	357,887	141,194	2,763	23,631	N/A	11,234	27,541	17,195	29,600	78,030	1,750,788
State Highway Traffic Trends 2020 compared to 2019		2020	517,960	202,741	668'66	302,183	105,554	2,456	19,199	41,059	6)789	22,703	13,603	24,708	65,127	1,426,481
State Highway Traf		Corridor	1-5	1-205	1-405	I-84	16 SU	US197	US20	US 26	0830	US395	OR18	OR22	US101	<b>Z</b>

\*Source:

Oregon Department of Transportation

Observed Statewide Traffic Volume Patterns:

Related to COVID-19 Monitoring

https://www.oregon.gov/odot/Data/Documents/ODOT TrafficReport April10 2020.pdf

# APPENDIX F ITE TRIP GENERATION

## Land Use: 934 Fast-Food Restaurant with Drive-Through Window

#### **Description**

This category includes fast-food restaurants with drive-through windows. This type of restaurant is characterized by a large drive-through clientele, long hours of service (some are open for breakfast, all are open for lunch and dinner, some are open late at night or 24 hours a day) and high turnover rates for eat-in customers. These limited-service eating establishments do not provide table service. Non-drive-through patrons generally order at a cash register and pay before they eat. Fast casual restaurant (Land Use 930), high-turnover (sit-down) restaurant (Land Use 932), fast-food restaurant without drive-through window (Land Use 933), and fast-food restaurant with drive-through window and no indoor seating (Land Use 935) are related uses.

#### **Additional Data**

Users should exercise caution when applying statistics during the AM peak periods, as the sites contained in the database for this land use may or may not be open for breakfast. In cases where it was confirmed that the sites were not open for breakfast, data for the AM peak hour of the adjacent street traffic were removed from the database.

The outdoor seating area is not included in the overall gross floor area. Therefore, the number of seats may be a more reliable independent variable on which to establish trip generation rates for facilities having significant outdoor seating.

Time-of-day distribution data for this land use for a weekday, Saturday, and Sunday are presented in Appendix A. For the 46 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 11:45 a.m. and 12:45 p.m. and 12:00 and 1:00 p.m., respectively. For the one dense multi-use urban site with data, the same AM and PM peak hours were observed.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alaska, Alberta (CAN), California, Colorado, Florida, Indiana, Kentucky, Maryland, Massachusetts, Minnesota, Montana, New Hampshire, New Jersey, New York, North Carolina, Ohio, Pennsylvania, South Dakota, Texas, Vermont, Virginia, Washington, and Wisconsin.

#### **Source Numbers**

163, 164, 168, 180, 181, 241, 245, 278, 294, 300, 301, 319, 338, 340, 342, 358, 389, 438, 502, 552, 577, 583, 584, 617, 640, 641, 704, 715, 728, 810, 866, 867, 869, 885, 886, 927, 935, 962, 977



### Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

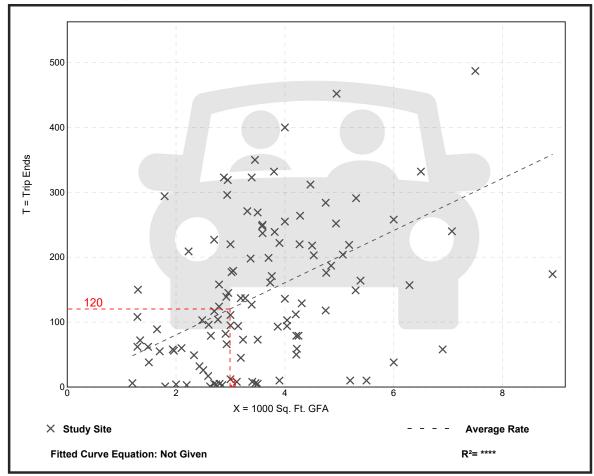
Number of Studies: 111 Avg. 1000 Sq. Ft. GFA: 4

Directional Distribution: 51% entering, 49% exiting

#### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
40.19	0.38 - 164.25	28.78

#### **Data Plot and Equation**



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### Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

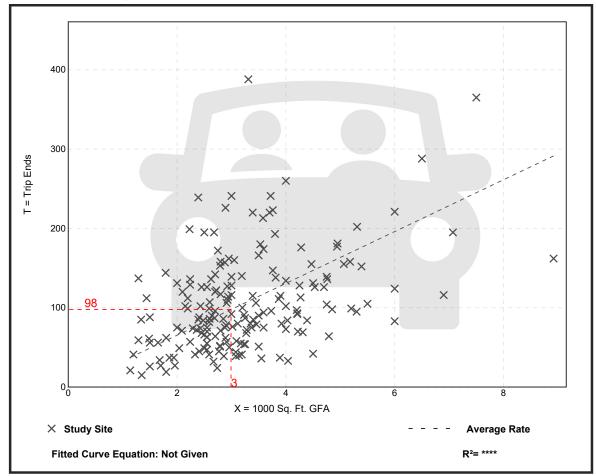
Number of Studies: 185 Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 52% entering, 48% exiting

#### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
32.67	8.17 - 117.22	17.87

#### **Data Plot and Equation**



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### Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban

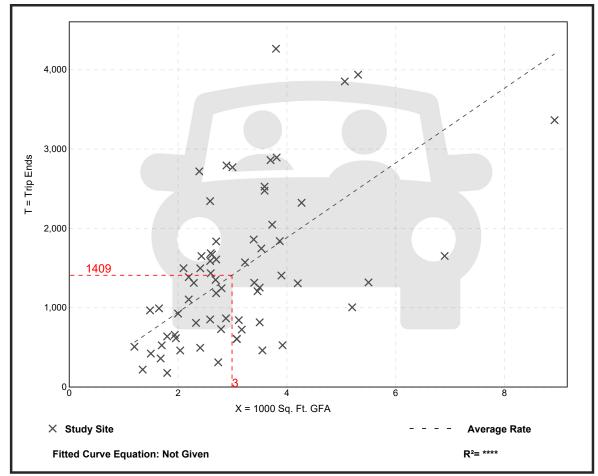
Number of Studies: 67 Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 50% entering, 50% exiting

#### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
470.95	98.89 - 1137.66	244.44

#### **Data Plot and Equation**



Trip Gen Manual, 10th Edition ● Institute of Transportation Engineers

### **APPENDIX G**

Existing and No-Build SYNCHRO Performance Calculations

	۶	<b>→</b>	*	•	<b>←</b>	4	1	†	<i>&gt;</i>	1	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	7>		*	f)		*	<b>†</b>		ň	<b>†</b>	
Traffic Volume (vph)	66	12	88	28	26	31	39	346	5	22	372	43
Future Volume (vph)	66	12	88	28	26	31	39	346	5	22	372	43
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.868			0.918			0.998			0.984	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1506	0	1662	1606	0	1662	3223	0	1662	3208	0
FIt Permitted	0.709			0.675			0.424			0.492		
Satd. Flow (perm)	1241	1506	0	1181	1606	0	742	3223	0	861	3208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		113			40			2			16	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Adj. Flow (vph)	85	15	113	36	33	40	50	444	6	28	477	55
Shared Lane Traffic (%)												
Lane Group Flow (vph)	85	128	0	36	73	0	50	450	0	28	532	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	Cl+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

#### Lanes, Volumes, Timings 1: HWY 101 & 35th Street

06/12/2020

	•	<b>→</b>	•	•	•	•	1	<b>†</b>	~	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	28.0	28.0		28.0	28.0		13.0	22.8		13.0	22.8	
Total Split (s)	34.0	34.0		34.0	34.0		17.0	41.0		15.0	39.0	
Total Split (%)	37.8%	37.8%		37.8%	37.8%		18.9%	45.6%		16.7%	43.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		12.2	36.2		10.2	34.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	17.0	17.0		17.0	17.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	7.4	7.4		7.3	7.3		14.2	14.8		13.3	13.0	
Actuated g/C Ratio	0.24	0.24		0.24	0.24		0.46	0.48		0.43	0.42	
v/c Ratio	0.29	0.29		0.13	0.18		0.10	0.29		0.05	0.39	
Control Delay	14.6	6.4		13.0	8.6		5.7	8.5		5.5	10.8	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.6	6.4		13.0	8.6		5.7	8.5		5.5	10.8	
LOS	В	Α		В	Α		Α	Α		Α	В	
Approach Delay		9.6			10.0			8.2			10.6	
Approach LOS		Α			В			Α			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 30.6

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 9.5 Intersection LOS: A Intersection Capacity Utilization 39.2% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2020 AM DESIGN HOUR 8:00 am 03/19/2020 2020 CONDITIONS DNH

### HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/12/2020

	۶	<b>→</b>	*	1	<b>←</b>	•	1	<b>†</b>	~	1	<b>↓</b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1		7	₽		*	<b>†</b>		7	<b>↑</b> ↑	
Traffic Volume (vph)	66	12	88	28	26	31	39	346	5	22	372	43
Future Volume (vph)	66	12	88	28	26	31	39	346	5	22	372	43
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.87		1.00	0.92		1.00	1.00		1.00	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1505		1662	1606		1662	3223		1662	3209	
Flt Permitted	0.71	1.00		0.67	1.00		0.42	1.00		0.49	1.00	
Satd. Flow (perm)	1242	1505		1181	1606		742	3223		861	3209	
Peak-hour factor, PHF	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Adj. Flow (vph)	85	15	113	36	33	40	50	444	6	28	477	55
RTOR Reduction (vph)	0	93	0	0	33	0	0	1	0	0	10	0
Lane Group Flow (vph)	85	35	0	36	40	0	50	449	0	28	522	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	6.6	6.6		6.6	6.6		15.8	14.0		14.0	13.1	
Effective Green, g (s)	6.1	6.1		6.1	6.1		14.8	13.5		13.0	12.6	
Actuated g/C Ratio	0.17	0.17		0.17	0.17		0.42	0.38		0.37	0.36	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	215	261		205	279		346	1239		328	1151	
v/s Ratio Prot		0.02			0.02		c0.01	0.14		0.00	c0.16	
v/s Ratio Perm	c0.07			0.03			0.06			0.03		
v/c Ratio	0.40	0.13		0.18	0.14		0.14	0.36		0.09	0.45	
Uniform Delay, d1	12.9	12.3		12.4	12.3		6.1	7.7		7.1	8.6	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.2		0.3	0.2		0.1	0.1		0.1	0.2	
Delay (s)	13.7	12.4		12.7	12.5		6.2	7.9		7.2	8.8	
Level of Service	В	В		В	В		Α	Α		Α	Α	
Approach Delay (s)		13.0			12.5			7.7			8.7	
Approach LOS		В			В			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			9.3	H	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capacity ratio			0.41									
Actuated Cycle Length (s)			35.1		um of lost				15.1			
Intersection Capacity Utilization			39.2%	IC	U Level o	of Service	•		Α			
Analysis Period (min)		15										
c Critical Lane Group												

2020 AM DESIGN HOUR 8:00 am 03/19/2020 2020 CONDITIONS DNH

#### Lanes, Volumes, Timings

#### 2: REDWOOD ST/Site & 35th Street

06/12/2020

	۶	<b>→</b>	•	1	•	•	4	<b>†</b>	/	-	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	20	19	15	48	0	37	0	9	0	0	0
Future Volume (vph)	0	20	19	15	48	0	37	0	9	0	0	0
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.934						0.973				
Flt Protected					0.988			0.962				
Satd. Flow (prot)	0	1634	0	0	1729	0	0	1638	0	0	1750	0
Flt Permitted					0.988			0.962				
Satd. Flow (perm)	0	1634	0	0	1729	0	0	1638	0	0	1750	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			493			457			274	
Travel Time (s)		8.6			13.4			12.5			6.2	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Adj. Flow (vph)	0	26	25	19	62	0	48	0	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	51	0	0	81	0	0	60	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized Intersection Capacity Utilization 20.3%

Analysis Period (min) 15

ICU Level of Service A

Intersection												
Int Delay, s/veh	3.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	20	19	15	48	0	37	0	9	0	0	0
Future Vol, veh/h	0	20	19	15	48	0	37	0	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	26	25	19	62	0	48	0	12	0	0	0
Major/Minor N	Major1			Major2		<u> </u>	Minor1		N	/linor2		
Conflicting Flow All	62	0	0	51	0	0	139	139	39	145	151	62
Stage 1	-	-	-	-	-	-	39	39	-	100	100	-
Stage 2	-	-	-	-	-	-	100	100	-	45	51	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1554	-	-	1568	-	-	836	756	1038	828	744	1009
Stage 1	-	-	-	-	-	-	981	866	-	911	816	-
Stage 2	-	-	-	-	-	-	911	816	-	974	856	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1554	-	-	1568	-	-	828	746	1038	811	734	1009
Mov Cap-2 Maneuver	-	-	-	-	-	-	828	746	-	811	734	-
Stage 1	-	-	-	-	-	-	981	866	-	911	805	-
Stage 2	-	-	-	-	-	-	899	805	-	963	856	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.7			9.5			0		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		862	1554			1568	-					
HCM Lane V/C Ratio		0.069	-	_		0.012	_	_	_			
HCM Control Delay (s)		9.5	0	_	-	7.3	0	_	0			
HCM Lane LOS		A	A	-	_	A	Ā	-	A			
HCM 95th %tile Q(veh)		0.2	0	_	-	0	-	-	-			
		- J. <u>-</u>										

### Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/12/2020

	1	•	1	1	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Volume (vph)	0	0	443	0	0	437
Future Volume (vph)	0	0	443	0	0	437
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt						
Flt Protected						
Satd. Flow (prot)	0	1750	3260	0	0	3260
Flt Permitted						
Satd. Flow (perm)	0	1750	3260	0	0	3260
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Adj. Flow (vph)	0	0	540	0	0	533
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	540	0	0	533
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized Intersection Capacity Utilization 16.6% Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	0					
		14/5-5			05:	05-
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>↑</b> ↑			<b>^</b>
Traffic Vol, veh/h	0	0	443	0	0	437
Future Vol, veh/h	0	0	443	0	0	437
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	0	0	540	0	0	533
	linor1		/lajor1		/lajor2	
Conflicting Flow All	-	270	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	_	-	_	-	_	-
Follow-up Hdwy	-	3.3	-	_	-	-
Pot Cap-1 Maneuver	0	734	_	_	0	_
Stage 1	0	-	_	_	0	_
Stage 2	0	_	_	_	0	_
Platoon blocked, %	U		_	_	U	_
Mov Cap-1 Maneuver	_	734	_		_	
	-	7 34		-	-	-
Mov Cap-2 Maneuver	-	-	-	-		
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		•			
110111 200	,,					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		-	-	-	-	
HCM Control Delay (s)		-	-	0	-	
HCM Lane LOS		-	-	Α	-	
HCM 95th %tile Q(veh)		-	-	-	-	
.,						

	۶	<b>→</b>	*	•	<b>←</b>	4	1	†	<i>&gt;</i>	1	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1>		*	f)		*	<b>†</b>		*	<b>↑</b> ↑	
Traffic Volume (vph)	85	31	111	28	15	43	110	689	44	41	666	62
Future Volume (vph)	85	31	111	28	15	43	110	689	44	41	666	62
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.883			0.889			0.991			0.987	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1533	0	1662	1556	0	1662	3291	0	1662	3282	0
FIt Permitted	0.717			0.661			0.253			0.356		
Satd. Flow (perm)	1255	1533	0	1157	1556	0	443	3291	0	623	3282	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		118			46			10			13	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Adj. Flow (vph)	90	33	118	30	16	46	117	733	47	44	709	66
Shared Lane Traffic (%)												
Lane Group Flow (vph)	90	151	0	30	62	0	117	780	0	44	775	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12	, i		12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

#### Lanes, Volumes, Timings 1: HWY 101 & 35th Street

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	•	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	-	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	22.0	22.0		28.0	28.0		13.0	28.8		13.0	22.8	
Total Split (s)	29.0	29.0		29.0	29.0		18.0	47.0		14.0	43.0	
Total Split (%)	32.2%	32.2%		32.2%	32.2%		20.0%	52.2%		15.6%	47.8%	
Maximum Green (s)	25.0	25.0		25.0	25.0		13.2	42.2		9.2	38.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		17.0	17.0			17.0			11.0	
Pedestrian Calls (#/hr)	1	1		0	0			0			1	
Act Effct Green (s)	8.8	8.8		8.8	8.8		24.2	21.7		20.6	16.3	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.54	0.49		0.46	0.37	
v/c Ratio	0.37	0.38		0.13	0.18		0.29	0.48		0.11	0.64	
Control Delay	21.0	9.2		17.4	9.3		7.1	11.2		6.0	16.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	21.0	9.2		17.4	9.3		7.1	11.2		6.0	16.3	
LOS	С	Α		В	Α		Α	В		Α	В	
Approach Delay		13.6			12.0			10.6			15.7	
Approach LOS		В			В			В			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 44.5

Natural Cycle: 70

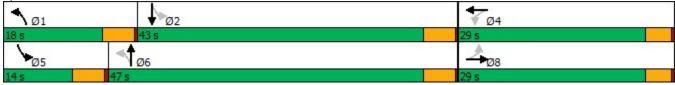
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 13.1 Intersection LOS: B
Intersection Capacity Utilization 57.6% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2020 PM DESIGN HOUR 4:30 pm 03/19/2020 EXISTING DNH

### HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/15/2020

	۶	<b>→</b>	*	1	<b>←</b>	•	1	<b>†</b>	~	<b>/</b>	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1		×	1		7	<b>↑</b> ↑		7	<b>†</b>	
Traffic Volume (vph)	85	31	111	28	15	43	110	689	44	41	666	62
Future Volume (vph)	85	31	111	28	15	43	110	689	44	41	666	62
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.88		1.00	0.89		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1533		1662	1555		1662	3291		1662	3283	
FIt Permitted	0.72	1.00		0.66	1.00		0.25	1.00		0.36	1.00	
Satd. Flow (perm)	1254	1533		1157	1555		444	3291		624	3283	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	90	33	118	30	16	46	117	733	47	44	709	66
RTOR Reduction (vph)	0	96	0	0	37	0	0	5	0	0	8	0
Lane Group Flow (vph)	90	55	0	30	25	0	117	775	0	44	767	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	9.3	9.3		9.3	9.3		27.6	22.2		21.2	19.0	
Effective Green, g (s)	8.8	8.8		8.8	8.8		26.6	21.7		20.2	18.5	
Actuated g/C Ratio	0.19	0.19		0.19	0.19		0.56	0.46		0.43	0.39	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	233	285		215	289		375	1509		303	1284	
v/s Ratio Prot		0.04			0.02		c0.03	c0.24		0.01	c0.23	
v/s Ratio Perm	c0.07			0.03			0.14			0.06		
v/c Ratio	0.39	0.19		0.14	0.08		0.31	0.51		0.15	0.60	
Uniform Delay, d1	16.9	16.3		16.1	15.9		5.4	9.1		8.0	11.4	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.8	0.2		0.2	0.1		0.3	0.2		0.2	0.6	
Delay (s)	17.7	16.5		16.3	16.0		5.7	9.3		8.1	12.1	
Level of Service	В	В		В	В		Α	Α		Α	В	
Approach Delay (s)		16.9			16.1			8.8			11.9	
Approach LOS		В			В			Α			В	
Intersection Summary												
HCM 2000 Control Delay			11.3	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ty ratio		0.50									
Actuated Cycle Length (s)			47.3	Sı	um of lost	time (s)			15.1			
Intersection Capacity Utilizati	on		57.6%		U Level o		•		В			
Analysis Period (min)			15									
c Critical Lane Group												

2020 PM DESIGN HOUR 4:30 pm 03/19/2020 EXISTING DNH

#### 2020 PM DESIGN HOUR CONDITIONS

Lanes, Volumes, Timings

2.	<b>REDWOOD</b>	ST/Site &	35th	Street
۷.	ILLDVVOOD		JJUII	Succi

06/15/2020

	٠	<b>→</b>	*	•	•	•	4	<b>†</b>	/	/	<b>↓</b>	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	59	57	29	46	0	40	0	17	0	0	0
Future Volume (vph)	0	59	57	29	46	0	40	0	17	0	0	0
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.933						0.959				
Flt Protected					0.981			0.966				
Satd. Flow (prot)	0	1633	0	0	1717	0	0	1621	0	0	1750	0
Flt Permitted					0.981			0.966				
Satd. Flow (perm)	0	1633	0	0	1717	0	0	1621	0	0	1750	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			225			457			251	
Travel Time (s)		8.6			6.1			12.5			5.7	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Adj. Flow (vph)	0	72	70	35	56	0	49	0	21	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	142	0	0	91	0	0	70	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized Intersection Capacity Utilization 21.2%

Analysis Period (min) 15

ICU Level of Service A

06/15/2020

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	59	57	29	46	0	40	0	17	0	0	0
Future Vol, veh/h	0	59	57	29	46	0	40	0	17	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	72	70	35	56	0	49	0	21	0	0	0
Major/Minor N	Major1		N	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	56	0	0	142	0	0	233	233	107	244	268	56
Stage 1	-	-	-	-	-	-	107	107	-	126	126	-
Stage 2	-	-	-	-	-	-	126	126	-	118	142	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1562	-	-	1453	-	-	726	671	953	714	641	1016
Stage 1	-	-	-	-	-	-	903	811	-	883	796	-
Stage 2	-	-	-	-	-	-	883	796	-	891	783	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1562	-	-	1453	-	-	712	654	953	685	625	1016
Mov Cap-2 Maneuver	-	-	-	-	-	-	712	654	-	685	625	-
Stage 1	-	-	-	-	-	-	903	811	-	883	776	-
Stage 2	-	-	-	-	-	-	861	776	-	872	783	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.9			10.1			0		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		770	1562	_		1453	_	-	-			
HCM Lane V/C Ratio		0.09	-	_		0.024	-	-	-			
HCM Control Delay (s)		10.1	0	-	-	7.5	0	-	0			
HCM Lane LOS		В	A	-	-	Α	A	-	A			
HCM 95th %tile Q(veh)		0.3	0	-	-	0.1	-	-	-			

### Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/15/2020

	1	•	<b>†</b>	-	1	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Volume (vph)	0	0	817	0	0	769
Future Volume (vph)	0	0	817	0	0	769
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt						
Flt Protected						
Satd. Flow (prot)	0	1750	3325	0	0	3325
Flt Permitted						
Satd. Flow (perm)	0	1750	3325	0	0	3325
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	0	869	0	0	818
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	869	0	0	818
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Cummery						

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 27.9%

Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	0					
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	۲Þ			<b>^</b>
Traffic Vol, veh/h	0	0	817	0	0	769
Future Vol, veh/h	0	0	817	0	0	769
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	869	0	0	818
		_				
	inor1		Major1		/lajor2	
Conflicting Flow All	-	435	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	575	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	575	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	_	-	-
Stage 1	_	_	_	_	_	_
Stage 2	_	_	_	_	_	_
Oldgo 2						
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
		NIDT	NIDDI	VBLn1	SBT	
Minor Lano/Major Mymt		NID I			ODI	
Minor Lane/Major Mvmt		NBT	NDIN	102		
Capacity (veh/h)		-	-	-	-	
Capacity (veh/h) HCM Lane V/C Ratio		-	-	-	-	
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		- - -	- - -	- - 0	- - -	
Capacity (veh/h) HCM Lane V/C Ratio		-	-	-	-	

### Lanes, Volumes, Timings 1: HWY 101 & 35th Street

06/15/2020

	۶	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	~	1	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	<b>1</b>		7	13		7	<b>†</b>		7	<b>†</b>	
Traffic Volume (vph)	73	12	102	28	26	31	43	346	5	22	372	45
Future Volume (vph)	73	12	102	28	26	31	43	346	5	22	372	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.865			0.918			0.998			0.984	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1500	0	1662	1606	0	1662	3223	0	1662	3208	0
Flt Permitted	0.709		•	0.664			0.419	00		0.492	0200	
Satd. Flow (perm)	1241	1500	0	1162	1606	0	733	3223	0	861	3208	0
Right Turn on Red		1000	Yes	02	1000	Yes		0220	Yes	00.	0200	Yes
Satd. Flow (RTOR)		131	. 00		40	. 00		2	. 00		17	100
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Adj. Flow (vph)	94	15	131	36	33	40	55	444	6	28	477	58
Shared Lane Traffic (%)	0.	10	101		00		00				.,,,	00
Lane Group Flow (vph)	94	146	0	36	73	0	55	450	0	28	535	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St			Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

#### Lanes, Volumes, Timings 1: HWY 101 & 35th Street

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	۶	<b>→</b>	•	•	<b>←</b>	•	1	†	~	/	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	28.0	28.0		28.0	28.0		13.0	22.8		13.0	22.8	
Total Split (s)	34.0	34.0		34.0	34.0		17.0	41.0		15.0	39.0	
Total Split (%)	37.8%	37.8%		37.8%	37.8%		18.9%	45.6%		16.7%	43.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		12.2	36.2		10.2	34.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	17.0	17.0		17.0	17.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	7.7	7.7		7.6	7.6		14.3	14.8		13.3	13.0	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.46	0.48		0.43	0.42	
v/c Ratio	0.30	0.31		0.13	0.17		0.11	0.29		0.06	0.39	
Control Delay	14.8	6.2		13.0	8.6		5.9	8.6		5.6	11.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.8	6.2		13.0	8.6		5.9	8.6		5.6	11.1	
LOS	В	Α		В	Α		Α	Α		Α	В	
Approach Delay		9.5			10.0			8.3			10.8	
Approach LOS		Α			В			Α			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 31

Natural Cycle: 65

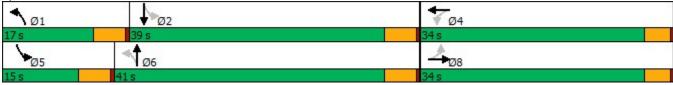
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 9.6 Intersection LOS: A Intersection Capacity Utilization 39.7% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2021 AM DESIGN HOUR  $\,8:00$  am  $\,03/19/2020$  2021 AM No-Build Conditions DNH

#### 2021 AM DESIGN HOUR NO-BUILD CONDITIONS

### HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/15/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ĵ.		×	1		1	<b>†</b>		7	<b>†</b> 1>	
Traffic Volume (vph)	73	12	102	28	26	31	43	346	5	22	372	45
Future Volume (vph)	73	12	102	28	26	31	43	346	5	22	372	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.87		1.00	0.92		1.00	1.00		1.00	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1501		1662	1606		1662	3223		1662	3207	
FIt Permitted	0.71	1.00		0.66	1.00		0.42	1.00		0.49	1.00	
Satd. Flow (perm)	1242	1501		1162	1606		734	3223		861	3207	
Peak-hour factor, PHF	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Adj. Flow (vph)	94	15	131	36	33	40	55	444	6	28	477	58
RTOR Reduction (vph)	0	107	0	0	33	0	0	1	0	0	11	0
Lane Group Flow (vph)	94	39	0	36	40	0	55	449	0	28	524	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	6.9	6.9		6.9	6.9		15.9	14.0		13.9	13.0	
Effective Green, g (s)	6.4	6.4		6.4	6.4		14.9	13.5		12.9	12.5	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.42	0.38		0.36	0.35	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	224	271		210	290		345	1229		322	1132	_
v/s Ratio Prot		0.03			0.03		c0.01	0.14		0.00	c0.16	
v/s Ratio Perm	c0.08			0.03			0.06			0.03		
v/c Ratio	0.42	0.14		0.17	0.14		0.16	0.37		0.09	0.46	
Uniform Delay, d1	12.9	12.2		12.3	12.2		6.2	7.9		7.3	8.9	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.2		0.3	0.2		0.2	0.1		0.1	0.2	
Delay (s)	13.8	12.4		12.5	12.3		6.3	8.0		7.4	9.1	
Level of Service	В	В		В	В		Α	Α		Α	Α	
Approach Delay (s)		12.9			12.4			7.8			9.0	
Approach LOS		В			В			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			9.5	Н	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capaci	ity ratio		0.43									
Actuated Cycle Length (s)			35.4	Sı	um of lost	time (s)			15.1			
Intersection Capacity Utilizati	ion		39.7%		U Level o		)		Α			
Analysis Period (min)			15									
c Critical Lane Group												

#### 2021 AM DESIGN HOUR NO-BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/15/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	20	19	15	48	0	37	0	9	0	0	0
Future Volume (vph)	0	20	19	15	48	0	37	0	9	0	0	0
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.934						0.973				
Flt Protected					0.988			0.962				
Satd. Flow (prot)	0	1634	0	0	1729	0	0	1638	0	0	1750	0
Flt Permitted					0.988			0.962				
Satd. Flow (perm)	0	1634	0	0	1729	0	0	1638	0	0	1750	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			493			457			274	
Travel Time (s)		8.6			13.4			12.5			6.2	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Adj. Flow (vph)	0	26	25	19	62	0	48	0	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	51	0	0	81	0	0	60	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 20.3%

Analysis Period (min) 15

ICU Level of Service A

#### HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

06/15/2020

Intersection												
Int Delay, s/veh	3.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	20	19	15	48	0	37	0	9	0	0	0
Future Vol, veh/h	0	20	19	15	48	0	37	0	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	26	25	19	62	0	48	0	12	0	0	0
Major/Minor N	//ajor1		N	Major2		I	Minor1		N	/linor2		
Conflicting Flow All	62	0	0	51	0	0	139	139	39	145	151	62
Stage 1	-	-	-	-	-	-	39	39	-	100	100	-
Stage 2	-	-	-	-	-	-	100	100	-	45	51	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1554	-	-	1568	-	-	836	756	1038	828	744	1009
Stage 1	-	-	-	-	-	-	981	866	-	911	816	-
Stage 2	-	-	-	-	-	-	911	816	-	974	856	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1554	-	-	1568	-	-	828	746	1038	811	734	1009
Mov Cap-2 Maneuver	-	-	-	-	-	-	828	746	-	811	734	-
Stage 1	-	-	-	-	-	-	981	866	-	911	805	-
Stage 2	-	-	-	-	-	-	899	805	-	963	856	-
J												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.7			9.5			0		
HCM LOS							Α			A		
Minor Lane/Major Mvm	t 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		862	1554	-	-	1568	-	-	-			
HCM Lane V/C Ratio		0.069	-	-	-	0.012	-	-	-			
HCM Control Delay (s)		9.5	0	-	-	7.3	0	-	0			
HCM Lane LOS		Α	Α	-	-	Α	Α	-	Α			
HCM 95th %tile Q(veh)		0.2	0	-	-	0	-	-	-			
,												

### 2021 AM DESIGN HOUR NO-BUILD CONDITIONS

# Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/15/2020

	1	•	<b>†</b>	-	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b> \$			<b>^</b>
Traffic Volume (vph)	0	0	450	0	0	439
Future Volume (vph)	0	0	450	0	0	439
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt						
Flt Protected						
Satd. Flow (prot)	0	1750	3260	0	0	3260
Flt Permitted						
Satd. Flow (perm)	0	1750	3260	0	0	3260
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Adj. Flow (vph)	0	0	549	0	0	535
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	549	0	0	535
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
O . ( . I.T II						

ICU Level of Service A

Control Type: Unsignalized Intersection Capacity Utilization 16.8% Analysis Period (min) 15

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>†</b> †
Traffic Vol, veh/h	0	0	450	0	0	439
Future Vol, veh/h	0	0	450	0	0	439
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	_	0	_	-	_	-
Veh in Median Storage	e,# 0	-	0	_	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	0	0	549	0	0	535
IVIVIIIL I IOW	U	U	J <del>4</del> 3	U	U	333
Major/Minor	Minor1	N	Major1	Λ	/lajor2	
Conflicting Flow All	-	275	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	_	-	-	-
Pot Cap-1 Maneuver	0	729	-	-	0	-
Stage 1	0	-	_	_	0	_
Stage 2	0	_	_	_	0	_
Platoon blocked, %	•		_	_		_
Mov Cap-1 Maneuver	_	729	_	_	_	_
Mov Cap 1 Maneuver	_	-	_	_	_	_
Stage 1		_		_		_
Stage 2	_	-	-	-	-	-
Staye 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Long /Mais a M	-1	NDT	MDDV	MDI 4	CDT	
Minor Lane/Major Mvn	nt	NBT	NRKA	VBLn1	SBT	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		-	-	-	-	
HCM Control Delay (s)		-	-	0	-	
HCM Lane LOS		-	-	Α	-	
HCM 95th %tile Q(veh		-	-	-	-	

	۶	<b>→</b>	*	•	+	4	1	†	<i>&gt;</i>	1	Ţ	<b>√</b>
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	7>		*	f)		*	<b>†</b>		*	<b>†</b>	
Traffic Volume (vph)	89	31	120	28	15	43	126	689	44	41	666	68
Future Volume (vph)	89	31	120	28	15	43	126	689	44	41	666	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.881			0.889			0.991			0.986	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1530	0	1662	1556	0	1662	3291	0	1662	3278	0
FIt Permitted	0.717			0.655			0.249			0.356		
Satd. Flow (perm)	1255	1530	0	1146	1556	0	436	3291	0	623	3278	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		128			46			10			15	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Adj. Flow (vph)	95	33	128	30	16	46	134	733	47	44	709	72
Shared Lane Traffic (%)												
Lane Group Flow (vph)	95	161	0	30	62	0	134	780	0	44	781	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12	Ţ.		12	Ţ.		12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

06/15/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	22.0	22.0		28.0	28.0		13.0	28.8		13.0	22.8	
Total Split (s)	29.0	29.0		29.0	29.0		18.0	47.0		14.0	43.0	
Total Split (%)	32.2%	32.2%		32.2%	32.2%		20.0%	52.2%		15.6%	47.8%	
Maximum Green (s)	25.0	25.0		25.0	25.0		13.2	42.2		9.2	38.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		17.0	17.0			17.0			11.0	
Pedestrian Calls (#/hr)	1	1		0	0			0			1	
Act Effct Green (s)	8.9	8.9		8.9	8.9		24.9	22.3		20.9	16.6	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.55	0.49		0.46	0.37	
v/c Ratio	0.38	0.40		0.13	0.18		0.32	0.48		0.11	0.64	
Control Delay	21.8	9.3		17.8	9.5		7.4	11.1		6.1	16.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	21.8	9.3		17.8	9.5		7.4	11.1		6.1	16.4	
LOS	С	Α		В	Α		Α	В		Α	В	
Approach Delay		13.9			12.2			10.5			15.9	
Approach LOS		В			В			В			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 45.2

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 13.1 Intersection LOS: B
Intersection Capacity Utilization 59.4% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2021 PM DESIGN HOUR 4:30 pm 03/19/2020 2021 PM Background Conditions DNH

## 2021 PM DESIGN HOUR NO-BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1		7	1→		7	<b>†</b>		7	<b>↑</b> ↑	
Traffic Volume (vph)	89	31	120	28	15	43	126	689	44	41	666	68
Future Volume (vph)	89	31	120	28	15	43	126	689	44	41	666	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.88		1.00	0.89		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1529		1662	1555		1662	3291		1662	3279	
FIt Permitted	0.72	1.00		0.66	1.00		0.25	1.00		0.36	1.00	
Satd. Flow (perm)	1254	1529		1146	1555		436	3291		624	3279	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	95	33	128	30	16	46	134	733	47	44	709	72
RTOR Reduction (vph)	0	104	0	0	37	0	0	5	0	0	9	0
Lane Group Flow (vph)	95	57	0	30	25	0	134	775	0	44	772	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	_	8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	9.4	9.4		9.4	9.4		28.5	22.9		21.7	19.5	
Effective Green, g (s)	8.9	8.9		8.9	8.9		27.5	22.4		20.7	19.0	
Actuated g/C Ratio	0.19	0.19		0.19	0.19		0.57	0.47		0.43	0.40	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	232	282		212	287		379	1532		305	1295	
v/s Ratio Prot		0.04			0.02		c0.04	c0.24		0.01	c0.24	
v/s Ratio Perm	c0.08			0.03			0.16			0.06		
v/c Ratio	0.41	0.20		0.14	0.09		0.35	0.51		0.14	0.60	
Uniform Delay, d1	17.3	16.6		16.4	16.2		5.4	9.0		8.0	11.5	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.3		0.2	0.1		0.4	0.2		0.2	0.6	
Delay (s)	18.1	16.8		16.6	16.3		5.8	9.2		8.2	12.1	
Level of Service	В	B		В	В		Α	A		Α	B	
Approach Delay (s)		17.3			16.4			8.7			11.9	
Approach LOS		В			В			Α			В	
Intersection Summary			44.4	1.1.	014 0000		<u> </u>					
HCM 2000 Control Delay			11.4	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.52	_		· ( )			45.4			
Actuated Cycle Length (s)	C		48.1		um of lost				15.1			
Intersection Capacity Utilizat	tion		59.4%	IC	U Level o	of Service	)		В			
Analysis Period (min)			15									
c Critical Lane Group												

### 2021 PM DESIGN HOUR NO-BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/15/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	59	57	29	46	0	40	0	17	0	0	0
Future Volume (vph)	0	59	57	29	46	0	40	0	17	0	0	0
ldeal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.933						0.959				
Flt Protected					0.981			0.966				
Satd. Flow (prot)	0	1633	0	0	1717	0	0	1621	0	0	1750	0
FIt Permitted					0.981			0.966				
Satd. Flow (perm)	0	1633	0	0	1717	0	0	1621	0	0	1750	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			225			457			251	
Travel Time (s)		8.6			6.1			12.5			5.7	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Adj. Flow (vph)	0	72	70	35	56	0	49	0	21	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	142	0	0	91	0	0	70	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	
1.1												

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 21.2%

Analysis Period (min) 15

### HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	59	57	29	46	0	40	0	17	0	0	0
Future Vol, veh/h	0	59	57	29	46	0	40	0	17	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	72	70	35	56	0	49	0	21	0	0	0
Major/Minor N	/lajor1		ľ	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	56	0	0	142	0	0	233	233	107	244	268	56
Stage 1	_	-	_	-	_	_	107	107	-	126	126	_
Stage 2	_	-	-	-	-	-	126	126	_	118	142	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1562	-	-	1453	-	-	726	671	953	714	641	1016
Stage 1	-	-	-	-	-	-	903	811	-	883	796	-
Stage 2	-	-	-	-	-	-	883	796	-	891	783	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1562	-	-	1453	-	-	712	654	953	685	625	1016
Mov Cap-2 Maneuver	-	-	-	-	-	-	712	654	-	685	625	-
Stage 1	-	-	-	-	-	-	903	811	-	883	776	-
Stage 2	-	-	-	-	-	-	861	776	-	872	783	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.9			10.1			0		
HCM LOS	•						В			A		
										, ,		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SRI n1			
	. 1	770	1562			1453		WDR	וושטנ			
Capacity (veh/h) HCM Lane V/C Ratio				-		0.024	-	-	-			
		0.09	-	-			-	<del>-</del>	0			
HCM Lang LOS		10.1	0	-	-	7.5	0	-				
HCM Lane LOS HCM 95th %tile Q(veh)		0.3	A 0	-	-	0.1	Α	-	Α			
HOW SOUL WILL CALLED		0.5	U	-	-	U. I	-	-	-			

### 2021 PM DESIGN HOUR NO-BUILD CONDITIONS

# Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/15/2020

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Volume (vph)	0	0	821	0	0	775
Future Volume (vph)	0	0	821	0	0	775
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt						
Flt Protected						
Satd. Flow (prot)	0	1750	3325	0	0	3325
Flt Permitted						
Satd. Flow (perm)	0	1750	3325	0	0	3325
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	0	873	0	0	824
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	873	0	0	824
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 28.0%

Analysis Period (min) 15

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Vol, veh/h	0	0	821	0	0	775
Future Vol, veh/h	0	0	821	0	0	775
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	_	0	-	-	_	-
Veh in Median Storage,	# 0	_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	0	0	873	0	0	824
IVIVIIILI IOW	U	U	0/3	U	U	024
Major/Minor N	/linor1	N	/lajor1	١	/lajor2	
Conflicting Flow All	-	437	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	_	6.9	_	-	_	-
Critical Hdwy Stg 1	-	-	-	_	-	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.3	_	_	_	_
Pot Cap-1 Maneuver	0	573	_	_	0	_
Stage 1	0	-	_	_	0	_
Stage 2	0	_	_	_	0	_
Platoon blocked, %	U	-	-	_	U	_
		E72				
Mov Cap-1 Maneuver	-	573	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		U		U	
1 TOWN LOO						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		_	-	_	-	
HCM Control Delay (s)		-	-	0	-	
HCM Lane LOS		-	-	A	-	
HCM 95th %tile Q(veh)		-	-	-	-	

06/15/2020

	۶	<b>→</b>	*	•	<b>←</b>	4	4	†	<i>&gt;</i>	1	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1>		*	f)		*	<b>†</b>		7	<b>↑</b> ↑	
Traffic Volume (vph)	73	12	102	28	26	31	43	347	5	22	373	45
Future Volume (vph)	73	12	102	28	26	31	43	347	5	22	373	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.865			0.918			0.998			0.984	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1500	0	1662	1606	0	1662	3223	0	1662	3208	0
FIt Permitted	0.709			0.664			0.419			0.491		
Satd. Flow (perm)	1241	1500	0	1162	1606	0	733	3223	0	859	3208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		131			40			2			17	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Adj. Flow (vph)	94	15	131	36	33	40	55	445	6	28	478	58
Shared Lane Traffic (%)												
Lane Group Flow (vph)	94	146	0	36	73	0	55	451	0	28	536	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

2026 AM DESIGN HOUR  $\,8:\!00$  am 03/19/2020 2026 AM No-Build Conditions DNH

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	28.0	28.0		28.0	28.0		13.0	22.8		13.0	22.8	
Total Split (s)	34.0	34.0		34.0	34.0		17.0	41.0		15.0	39.0	
Total Split (%)	37.8%	37.8%		37.8%	37.8%		18.9%	45.6%		16.7%	43.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		12.2	36.2		10.2	34.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	17.0	17.0		17.0	17.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	7.7	7.7		7.6	7.6		14.3	14.8		13.3	13.0	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.46	0.48		0.43	0.42	
v/c Ratio	0.30	0.31		0.13	0.17		0.11	0.29		0.06	0.39	
Control Delay	14.8	6.2		13.0	8.6		5.9	8.6		5.6	11.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.8	6.2		13.0	8.6		5.9	8.6		5.6	11.1	
LOS	В	Α		В	Α		Α	Α		Α	В	
Approach Delay		9.6			10.0			8.3			10.8	
Approach LOS		Α			В			Α			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 31

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 9.6 Intersection LOS: A Intersection Capacity Utilization 39.7% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2026 AM DESIGN HOUR  $\,8:00$  am  $\,03/19/2020$  2026 AM No-Build Conditions DNH

## 2026 AM DESIGN HOUR NO-BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

	۶	-	•	1	•	•	4	†	~	/	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1→		7	1		1	<b>↑</b> ↑		7	<b>†</b>	
Traffic Volume (vph)	73	12	102	28	26	31	43	347	5	22	373	45
Future Volume (vph)	73	12	102	28	26	31	43	347	5	22	373	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.87		1.00	0.92		1.00	1.00		1.00	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1501		1662	1606		1662	3223		1662	3207	
Flt Permitted	0.71	1.00		0.66	1.00		0.42	1.00		0.49	1.00	
Satd. Flow (perm)	1242	1501		1162	1606		733	3223		860	3207	
Peak-hour factor, PHF	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Adj. Flow (vph)	94	15	131	36	33	40	55	445	6	28	478	58
RTOR Reduction (vph)	0	107	0	0	33	0	0	1	0	0	11	0
Lane Group Flow (vph)	94	39	0	36	40	0	55	450	0	28	525	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	6.9	6.9		6.9	6.9		15.9	14.0		13.9	13.0	
Effective Green, g (s)	6.4	6.4		6.4	6.4		14.9	13.5		12.9	12.5	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.42	0.38		0.36	0.35	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	224	271		210	290		345	1229		322	1132	
v/s Ratio Prot		0.03			0.03		c0.01	0.14		0.00	c0.16	
v/s Ratio Perm	c0.08			0.03			0.06			0.03		
v/c Ratio	0.42	0.14		0.17	0.14		0.16	0.37		0.09	0.46	
Uniform Delay, d1	12.9	12.2		12.3	12.2		6.2	7.9		7.3	8.9	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.2		0.3	0.2		0.2	0.1		0.1	0.2	
Delay (s)	13.8	12.4		12.5	12.3		6.3	8.0		7.4	9.1	
Level of Service	В	В		В	В		Α	Α		Α	Α	
Approach Delay (s)		12.9			12.4			7.8			9.0	
Approach LOS		В			В			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			9.5	H	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capaci	ity ratio		0.43									
Actuated Cycle Length (s)			35.4	Sı	um of lost	time (s)			15.1			
Intersection Capacity Utilizati	on		39.7%		U Level o		•		Α			
Analysis Period (min)			15									
c Critical Lane Group												

### 2026 AM DESIGN HOUR NO-BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/15/2020

	۶	<b>→</b>	•	•	•	•	1	<b>†</b>	-	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	20	19	15	48	0	37	0	9	0	0	0
Future Volume (vph)	0	20	19	15	48	0	37	0	9	0	0	0
ldeal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.934						0.973				
Flt Protected					0.988			0.962				
Satd. Flow (prot)	0	1634	0	0	1729	0	0	1638	0	0	1750	0
Flt Permitted					0.988			0.962				
Satd. Flow (perm)	0	1634	0	0	1729	0	0	1638	0	0	1750	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			493			457			274	
Travel Time (s)		8.6			13.4			12.5			6.2	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Adj. Flow (vph)	0	26	25	19	62	0	48	0	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	51	0	0	81	0	0	60	0	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 20.3%

Analysis Period (min) 15

### HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

Intersection												
Int Delay, s/veh	3.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	20	19	15	48	0	37	0	9	0	0	0
Future Vol, veh/h	0	20	19	15	48	0	37	0	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	26	25	19	62	0	48	0	12	0	0	0
Major/Minor N	Major1		N	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	62	0	0	51	0	0	139	139	39	145	151	62
Stage 1	-	-	-	-	-	-	39	39	-	100	100	-
Stage 2	-	-	-	-	-	-	100	100	-	45	51	-
Critical Hdwy	4.1	_	-	4.1	_	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	_	_	-	-	_	-	6.1	5.5	-	6.1	5.5	_
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1554	-	-	1568	-	-	836	756	1038	828	744	1009
Stage 1	-	-	-	-	-	-	981	866	-	911	816	-
Stage 2	-	-	-	-	-	-	911	816	-	974	856	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1554	-	-	1568	-	-	828	746	1038	811	734	1009
Mov Cap-2 Maneuver	-	-	-	-	-	-	828	746	-	811	734	-
Stage 1	-	-	-	-	-	-	981	866	-	911	805	-
Stage 2	-	-	-	-	-	-	899	805	-	963	856	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.7			9.5			0		
HCM LOS							A			A		
							,					
Minor Lane/Major Mvm	† 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBI n1			
Capacity (veh/h)		862	1554	-		1568	-	-110111	_			
HCM Lane V/C Ratio		0.069	1004	-		0.012	_	_	_			
HCM Control Delay (s)		9.5	0	_	_	7.3	0	-	0			
HCM Lane LOS		9.5 A	A	-	_	7.3 A	A	-	A			
HCM 95th %tile Q(veh)		0.2	0	_	-	0	-	<u>-</u>	-			
HOW JOHN JOHN Q(VEII)		0.2	U			U						

### 2026 AM DESIGN HOUR NO-BUILD CONDITIONS

### Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/15/2020

	1	•	<b>†</b>	1	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b> 1>			<b>^</b>
Traffic Volume (vph)	0	0	451	0	0	440
Future Volume (vph)	0	0	451	0	0	440
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt						
Flt Protected						
Satd. Flow (prot)	0	1750	3260	0	0	3260
Flt Permitted						
Satd. Flow (perm)	0	1750	3260	0	0	3260
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Adj. Flow (vph)	0	0	550	0	0	537
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	550	0	0	537
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					

Control Type: Unsignalized Intersection Capacity Utilization 16.9% Analysis Period (min) 15

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

Novement   WBL   WBR   NBT   NBR   SBL   SBT	Intersection						
The Configurations affic Vol, veh/h	Int Delay, s/veh	0					
The Configurations affic Vol, veh/h	Movement	WRI	WRR	NRT	NRR	SRI	SRT
affic Vol, veh/h		VIDE			NOIL	ODL	
ture Vol, veh/h onflicting Peds, #/hr onflicting Peds, #/hr on 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		Λ		<b>T →</b>	0	0	
Inflicting Peds, #/hr         0         0         0         0         0         0           Inflicting Peds, #/hr         Stop         Stop         Free         D         D         D							
Stop   Stop   Free   Free							
Channelized							
orage Length		Stop		Free		Free	
# in Median Storage, # 0	RT Channelized	-		-	None	-	None
ade, % 0 - 0 - 0 - 0 0 ak Hour Factor 82 82 82 82 82 82 82 82 82 82 82 82 82	Storage Length		0		-	-	
Back Hour Factor         82         83         82	Veh in Median Storage,	# 0	-	0	-	-	0
Paray Vehicles, % 0 0 2 0 0 2  Param Flow 0 0 550 0 0 537  Param Flow 0 0 550 0 0 0 537  Param Flow 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Grade, %	0	-	0	-	-	0
Amit Flow   O   O   550   O   O   537	Peak Hour Factor	82	82	82	82	82	82
Amit Flow   O   O   550   O   O   537							
Ajor/Minor   Minor1   Major1   Major2   Major2	Mvmt Flow						
Stage 1	WWITCHIOW	U	U	000	U	U	001
Stage 1							
Stage 1       - </td <td>Major/Minor Mi</td> <td>inor1</td> <td>N</td> <td>Major1</td> <td>N</td> <td>/lajor2</td> <td></td>	Major/Minor Mi	inor1	N	Major1	N	/lajor2	
Stage 1       -        -       -       -       -       -       -       -       -       -       -       -       -       -       -       -        -	Conflicting Flow All	-	275	0	0	-	-
Stage 2		-	-	-	_	-	-
itical Hdwy Stg 1		-	-	-	-	-	-
itical Hdwy Stg 1		_	6.9	_	_	_	_
itical Hdwy Stg 2		_		_	_	_	_
Stage 1			_	_			
Stage 1			2 2	-			
Stage 1       0       -       -       0       -         Stage 2       0       -       -       0       -         atoon blocked, %       -       -       -       -       -         by Cap-1 Maneuver       -       729       -       -       -       -         by Cap-2 Maneuver       -							
Stage 2       0       -       -       0       -         atoon blocked, %       -       -       -       -       -         by Cap-1 Maneuver       -       729       -       -       -       -         by Cap-2 Maneuver       - <td< td=""><td>•</td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	•						
Action blocked, %			-	-	-		-
Dov Cap-1 Maneuver		0	-	-	-	0	-
Stage 1	Platoon blocked, %			-	-		-
Stage 1         - </td <td>Mov Cap-1 Maneuver</td> <td>-</td> <td>729</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td>	Mov Cap-1 Maneuver	-	729	-	-	-	-
Stage 1         - </td <td>Mov Cap-2 Maneuver</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td>	Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 2		-	_	-	-	-	-
NB		_	_	_	_	_	_
CM Control Delay, s 0 0 0  CM LOS A   nor Lane/Major Mvmt NBT NBRWBLn1 SBT  apacity (veh/h)  CM Lane V/C Ratio  CM Control Delay (s) - 0 -  CM Lane LOS - A	Olago Z						
CM Control Delay, s 0 0 0  CM LOS A   nor Lane/Major Mvmt NBT NBRWBLn1 SBT  apacity (veh/h)  CM Lane V/C Ratio  CM Control Delay (s) - 0 -  CM Lane LOS - A							
nor Lane/Major Mvmt  Apacity (veh/h)  CM Lane V/C Ratio  CM Control Delay (s)  CM Lane LOS  A  NBT NBRWBLn1 SBT	Approach	WB		NB		SB	
nor Lane/Major Mvmt NBT NBRWBLn1 SBT  apacity (veh/h)  CM Lane V/C Ratio  CM Control Delay (s) - 0 -  CM Lane LOS - A	HCM Control Delay, s	0		0		0	
apacity (veh/h)	HCM LOS	Α					
apacity (veh/h)							
apacity (veh/h)	10.1		NET	MES	MDI 4	05.7	
CM Lane V/C Ratio       -       -       -       -         CM Control Delay (s)       -       -       0       -         CM Lane LOS       -       -       A       -			NBT	NBRV	VBLn1	SBT	
CM Control Delay (s) 0 CM Lane LOS A -	Capacity (veh/h)		-	-	-	-	
CM Lane LOS A -	HCM Lane V/C Ratio		-	-	-	-	
CM Lane LOS A -	HCM Control Delay (s)		-	-	0	-	
	HCM Lane LOS		-	-	Α	-	
. ( - /	HCM 95th %tile Q(veh)		-	-		-	
	(· 2.1)						

06/15/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ň	1>		ħ	f)		*	<b>†</b>		*	<b>†</b>	
Traffic Volume (vph)	89	31	120	28	15	43	126	690	44	41	667	68
Future Volume (vph)	89	31	120	28	15	43	126	690	44	41	667	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.881			0.889			0.991			0.986	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1530	0	1662	1556	0	1662	3291	0	1662	3278	0
Flt Permitted	0.717			0.655			0.249			0.356		
Satd. Flow (perm)	1255	1530	0	1146	1556	0	436	3291	0	623	3278	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		128			46			10			15	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Adj. Flow (vph)	95	33	128	30	16	46	134	734	47	44	710	72
Shared Lane Traffic (%)												
Lane Group Flow (vph)	95	161	0	30	62	0	134	781	0	44	782	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	Ū
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	22.0	22.0		28.0	28.0		13.0	28.8		13.0	22.8	
Total Split (s)	29.0	29.0		29.0	29.0		18.0	47.0		14.0	43.0	
Total Split (%)	32.2%	32.2%		32.2%	32.2%		20.0%	52.2%		15.6%	47.8%	
Maximum Green (s)	25.0	25.0		25.0	25.0		13.2	42.2		9.2	38.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		17.0	17.0			17.0			11.0	
Pedestrian Calls (#/hr)	1	1		0	0			0			1	
Act Effct Green (s)	8.9	8.9		8.9	8.9		24.9	22.4		21.0	16.7	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.55	0.49		0.46	0.37	
v/c Ratio	0.38	0.40		0.13	0.18		0.32	0.48		0.11	0.64	
Control Delay	21.8	9.3		17.8	9.5		7.4	11.1		6.1	16.4	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	21.8	9.3		17.8	9.5		7.4	11.1		6.1	16.4	
LOS	С	Α		В	Α		Α	В		Α	В	
Approach Delay		13.9			12.2			10.5			15.9	
Approach LOS		В			В			В			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 45.3

Natural Cycle: 70

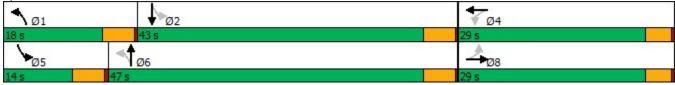
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 13.1 Intersection LOS: B
Intersection Capacity Utilization 59.4% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



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## 2026 PM DESIGN HOUR NO-BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1		7	1		7	<b>†</b>		7	<b>↑</b> ↑	
Traffic Volume (vph)	89	31	120	28	15	43	126	690	44	41	667	68
Future Volume (vph)	89	31	120	28	15	43	126	690	44	41	667	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.88		1.00	0.89		1.00	0.99		1.00	0.99	
FIt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1529		1662	1555		1662	3291		1662	3279	
FIt Permitted	0.72	1.00		0.66	1.00		0.25	1.00		0.36	1.00	
Satd. Flow (perm)	1254	1529		1146	1555		435	3291		623	3279	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	95	33	128	30	16	46	134	734	47	44	710	72
RTOR Reduction (vph)	0	104	0	0	37	0	0	5	0	0	9	0
Lane Group Flow (vph)	95	57	0	30	25	0	134	776	0	44	773	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	_	8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	9.4	9.4		9.4	9.4		28.5	22.9		21.7	19.5	
Effective Green, g (s)	8.9	8.9		8.9	8.9		27.5	22.4		20.7	19.0	
Actuated g/C Ratio	0.19	0.19		0.19	0.19		0.57	0.47		0.43	0.40	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	232	282		212	287		378	1532		304	1295	
v/s Ratio Prot	0.00	0.04		0.00	0.02		c0.04	c0.24		0.01	c0.24	
v/s Ratio Perm	c0.08	0.00		0.03	0.00		0.16	0.54		0.06	0.00	
v/c Ratio	0.41	0.20		0.14	0.09		0.35	0.51		0.14	0.60	
Uniform Delay, d1	17.3	16.6		16.4	16.2		5.4	9.0		8.0	11.5	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.3		0.2	0.1		0.4	0.2		0.2	0.6	
Delay (s)	18.1	16.8		16.6	16.3		5.8	9.2		8.2	12.1	
Level of Service	В	B		В	B		Α	Α		Α	B	
Approach Delay (s) Approach LOS		17.3 B			16.4 B			8.7 A			11.9 B	
• •		D			D			A			D	
Intersection Summary HCM 2000 Control Delay			11.4	Li	CM 2000	Lovelef	Contino		В			
HCM 2000 Control Delay	oity ratio		0.52	11'	CIVI ZUUU	Level of	Service		Ь			
Actuated Cycle Length (s)	City ratio		48.1	Ç,	um of loct	timo (c)			15.1			
Intersection Capacity Utiliza	tion		59.4%		um of lost U Level o				15.1 B			
Analysis Period (min)	uon		15	iC	O Level (	OF VICE	7		D			
c Critical Lane Group			10									
c Chilical Lane Group												

### 2026 PM DESIGN HOUR NO-BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/15/2020

	٠	<b>→</b>	*	•	<b>←</b>	*	1	<b>†</b>	-	/	<b>↓</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	59	57	29	46	0	40	0	17	0	0	0
Future Volume (vph)	0	59	57	29	46	0	40	0	17	0	0	0
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.933						0.959				
Flt Protected					0.981			0.966				
Satd. Flow (prot)	0	1633	0	0	1717	0	0	1621	0	0	1750	0
Flt Permitted					0.981			0.966				
Satd. Flow (perm)	0	1633	0	0	1717	0	0	1621	0	0	1750	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			225			457			251	
Travel Time (s)		8.6			6.1			12.5			5.7	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Adj. Flow (vph)	0	72	70	35	56	0	49	0	21	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	142	0	0	91	0	0	70	0	0	0	0
Enter Blocked Intersection	No	No	No									
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	
1. ( (												

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 21.2%

Analysis Period (min) 15

### HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	59	57	29	46	0	40	0	17	0	0	0
Future Vol, veh/h	0	59	57	29	46	0	40	0	17	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	72	70	35	56	0	49	0	21	0	0	0
Major/Minor N	/lajor1		I	Major2		ı	Minor1		N	/linor2		
Conflicting Flow All	56	0	0	142	0	0	233	233	107	244	268	56
Stage 1	-	-	-	-	-	-	107	107	-	126	126	-
Stage 2	-	-	-	-	-	-	126	126	-	118	142	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1562	-	-	1453	-	-	726	671	953	714	641	1016
Stage 1	-	-	-	-	-	-	903	811	-	883	796	-
Stage 2	-	-	-	-	-	-	883	796	-	891	783	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1562	-	-	1453	-	-	712	654	953	685	625	1016
Mov Cap-2 Maneuver	-	-	-	-	-	-	712	654	-	685	625	-
Stage 1	-	-	-	-	-	-	903	811	-	883	776	-
Stage 2	-	-	-	-	-	-	861	776	-	872	783	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.9			10.1			0		
HCM LOS							В			A		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		770	1562			1453	-					
HCM Lane V/C Ratio		0.09	-	_		0.024	_	_	_			
HCM Control Delay (s)		10.1	0	_	_	7.5	0	_	0			
HCM Lane LOS		В	A	_	_	Α.	A	_	A			
HCM 95th %tile Q(veh)		0.3	0	_	_	0.1	-	_	-			
110111 00til 70tilo 3(VOII)		0.0				0.1						

### 2026 PM DESIGN HOUR NO-BUILD CONDITIONS

### Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/15/2020

	1		<b>†</b>	-	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b> \$			<b>^</b>
Traffic Volume (vph)	0	0	822	0	0	776
Future Volume (vph)	0	0	822	0	0	776
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt						
Flt Protected						
Satd. Flow (prot)	0	1750	3325	0	0	3325
Flt Permitted						
Satd. Flow (perm)	0	1750	3325	0	0	3325
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	0	874	0	0	826
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	874	0	0	826
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 28.0%

Analysis Period (min) 15

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>†</b> †
Traffic Vol, veh/h	0	0	822	0	0	776
Future Vol, veh/h	0	0	822	0	0	776
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	_	0	_	-	_	-
Veh in Median Storage,	# 0	_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	0	0	874	0	0	826
IVIVIIIL I IOW	U	U	074	U	U	020
Major/Minor M	1inor1	N	/lajor1	Λ	/lajor2	
Conflicting Flow All	-	437	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	_	-	-	-	_
Follow-up Hdwy	_	3.3	_	-	_	-
Pot Cap-1 Maneuver	0	573	-	-	0	_
Stage 1	0	-	_	_	0	_
Stage 2	0	_	_	_	0	_
Platoon blocked, %	U		_	_	U	_
Mov Cap-1 Maneuver	_	573	_	_	_	_
Mov Cap-1 Maneuver	_	5/ 5	_	_	_	
Stage 1	_	_	_			_
_	-	-	-	-	-	-
Stage 2	-	_	-	-	-	_
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		*		•	
	, ,					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		-	-	-	-	
HCM Control Delay (s)		-	-	0	-	
HCM Lane LOS		-	-	Α	-	
HCM 95th %tile Q(veh)		-	-	-	-	

# APPENDIX H

Build SYNCHRO Performance Calculations

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1>		7	₽		7	<b>†</b> \$		*	<b>†</b> 1>	
Traffic Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Future Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100		-	55		-	66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.872			0.925			0.989			0.983	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1513	0	1662	1619	0	1662	3199	0	1662	3204	0
FIt Permitted	0.705			0.660			0.461			0.475		
Satd. Flow (perm)	1234	1513	0	1155	1619	0	807	3199	0	831	3204	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		131			40			10			17	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Adj. Flow (vph)	94	22	131	72	40	40	55	436	33	62	459	58
Shared Lane Traffic (%)												
Lane Group Flow (vph)	94	153	0	72	80	0	55	469	0	62	517	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12	, ,		12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

06/18/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	28.0	28.0		28.0	28.0		13.0	22.8		13.0	22.8	
Total Split (s)	34.0	34.0		34.0	34.0		17.0	41.0		15.0	39.0	
Total Split (%)	37.8%	37.8%		37.8%	37.8%		18.9%	45.6%		16.7%	43.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		12.2	36.2		10.2	34.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	17.0	17.0		17.0	17.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	7.8	7.8		7.7	7.7		13.3	12.9		13.6	13.1	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.43	0.42		0.44	0.42	
v/c Ratio	0.30	0.32		0.25	0.19		0.11	0.35		0.12	0.38	
Control Delay	14.7	6.5		14.2	8.7		6.1	11.2		6.0	11.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.7	6.5		14.2	8.7		6.1	11.2		6.0	11.0	
LOS	В	Α		В	Α		Α	В		Α	В	
Approach Delay		9.6			11.3			10.7			10.5	
Approach LOS		Α			В			В			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 31

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.38

Intersection Signal Delay: 10.5 Intersection LOS: B
Intersection Capacity Utilization 39.3% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2021 AM DESIGN HOUR 8:00 am 03/19/2020 2021 AM Build Conditions DNH

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### 2021 AM DESIGN HOUR BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/18/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1		7	1		7	<b>†</b>		7	<b>†</b>	
Traffic Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Future Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.87		1.00	0.93		1.00	0.99		1.00	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1512		1662	1619		1662	3201		1662	3205	
FIt Permitted	0.70	1.00		0.66	1.00		0.46	1.00		0.47	1.00	
Satd. Flow (perm)	1234	1512		1155	1619		807	3201		831	3205	
Peak-hour factor, PHF	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Adj. Flow (vph)	94	22	131	72	40	40	55	436	33	62	459	58
RTOR Reduction (vph)	0	107	0	0	33	0	0	7	0	0	11	0
Lane Group Flow (vph)	94	46	0	72	47	0	55	462	0	62	506	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	6.9	6.9		6.9	6.9		14.0	12.1		14.4	12.3	
Effective Green, g (s)	6.4	6.4		6.4	6.4		13.0	11.6		13.4	11.8	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.37	0.33		0.39	0.34	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	227	278		213	298		336	1070		359	1089	_
v/s Ratio Prot		0.03			0.03		0.01	0.14		c0.01	c0.16	
v/s Ratio Perm	c0.08			0.06			0.05			0.06		
v/c Ratio	0.41	0.17		0.34	0.16		0.16	0.43		0.17	0.46	
Uniform Delay, d1	12.5	11.9		12.3	11.9		7.0	9.0		6.8	9.0	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.2		0.7	0.2		0.2	0.2		0.2	0.2	
Delay (s)	13.4	12.1		13.0	12.1		7.2	9.2		7.0	9.2	
Level of Service	В	В		В	В		Α	Α		Α	Α	
Approach Delay (s)		12.6			12.5			9.0			9.0	
Approach LOS		В			В			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			9.9	H	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capaci	ty ratio		0.43									
Actuated Cycle Length (s)			34.7	Sı	um of lost	time (s)			15.1			
Intersection Capacity Utilizati	on		39.3%		U Level o	٠,	9		Α			
Analysis Period (min)			15									
c Critical Lane Group												

2021 AM DESIGN HOUR 8:00 am 03/19/2020 2021 AM Build Conditions DNH

### 2021 AM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/18/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	52	20	19	15	48	0	37	2	9	0	2	33
Future Volume (vph)	52	20	19	15	48	0	37	2	9	0	2	33
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.972						0.974			0.874	
Flt Protected		0.972			0.988			0.963				
Satd. Flow (prot)	0	1653	0	0	1729	0	0	1641	0	0	1530	0
Flt Permitted		0.972			0.988			0.963				
Satd. Flow (perm)	0	1653	0	0	1729	0	0	1641	0	0	1530	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			493			457			274	
Travel Time (s)		8.6			13.4			12.5			6.2	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Adj. Flow (vph)	68	26	25	19	62	0	48	3	12	0	3	43
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	119	0	0	81	0	0	63	0	0	46	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 28.0%

Analysis Period (min) 15

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	52	20	19	15	48	0	37	2	9	0	2	33
Future Vol, veh/h	52	20	19	15	48	0	37	2	9	0	2	33
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	68	26	25	19	62	0	48	3	12	0	3	43
Major/Minor N	//ajor1		1	Major2		1	Minor1		N	/linor2		
Conflicting Flow All	62	0	0	51	0	0	298	275	39	282	287	62
Stage 1	-	-	-	-	-	-	175	175	-	100	100	-
Stage 2	-	-	-	-	-	-	123	100	-	182	187	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1554	-	-	1568	-	-	658	636	1038	674	626	1009
Stage 1	-	-	-	-	-	-	832	758	-	911	816	-
Stage 2	-	-	-	-	-	-	886	816	-	824	749	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1554	-	-	1568	-	-	600	600	1038	635	590	1009
Mov Cap-2 Maneuver	-	-	-	-	-	-	600	600	-	635	590	-
Stage 1	-	-	-	-	-	-	795	724	-	870	805	-
Stage 2	-	-	-	-	-	-	835	805	-	775	715	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.2			1.7			11.1			8.9		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)			1554	_		1568	-	-	970			
HCM Lane V/C Ratio			0.043	_		0.012	-	_	0.047			
HCM Control Delay (s)		11.1	7.4	0	-	7.3	0	_	8.9			
HCM Lane LOS		В	Α	A	-	A	A	-	A			
HCM 95th %tile Q(veh)		0.3	0.1	-	-	0	_	-	0.1			

# Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

	•	•	<b>†</b>	-	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>↑</b> ↑			<b>^</b>
Traffic Volume (vph)	0	24	437	7	0	451
Future Volume (vph)	0	24	437	7	0	451
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.865	0.998			
Flt Protected						
Satd. Flow (prot)	0	1514	3254	0	0	3260
Flt Permitted						
Satd. Flow (perm)	0	1514	3254	0	0	3260
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Adj. Flow (vph)	0	29	533	9	0	550
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	29	542	0	0	550
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

ICU Level of Service A

Intersection Capacity Utilization 23.4% Analysis Period (min) 15

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	VVDL	VVDK	<b>↑</b>	אטוז	ODL	<u>↑</u>
Traffic Vol, veh/h	0	24	437	7	0	<b>TT</b> 451
Future Vol, veh/h	0	24	437	7	0	451
Conflicting Peds, #/hr	0	0	437	0	0	451
Sign Control			Free	Free	Free	Free
RT Channelized	Stop -	Stop None	riee -		riee -	None
Storage Length	-	None 0	-	None -		None
		-	0		-	0
Veh in Median Storage,				-	-	
Grade, %	0	- 00	0	- 00	- 00	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	0	29	533	9	0	550
Major/Minor M	/linor1	N	Major1	N	/lajor2	
Conflicting Flow All	_	271	0	0		_
Stage 1	_		-	-	_	_
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	6.9	_	_	_	_
Critical Hdwy Stg 1	_	-	_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.3	_	_	_	_
Pot Cap-1 Maneuver	0	733	_	_	0	_
Stage 1	0	-	_	_	0	_
Stage 1	0	-	_	-	0	-
Platoon blocked, %	U	-			U	
		722	-	-		-
Mov Cap-1 Maneuver	-	733	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.1		0		0	
HCM LOS	В				•	
	_					
					05-	
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	733	-	
HCM Lane V/C Ratio		-	-	0.04	-	
HCM Control Delay (s)		-	-	10.1	-	
HCM Lane LOS		-	-	В	-	
HCM 95th %tile Q(veh)		-	-	0.1	-	

	۶	<b>→</b>	*	•	+	4	1	†	<i>&gt;</i>	1	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	7>		*	f)		*	<b>†</b>		7	<b>†</b>	
Traffic Volume (vph)	89	35	120	50	19	43	126	683	62	62	654	68
Future Volume (vph)	89	35	120	50	19	43	126	683	62	62	654	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.884			0.895			0.988			0.986	
FIt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1535	0	1662	1566	0	1662	3280	0	1662	3278	0
FIt Permitted	0.714			0.653			0.267			0.331		
Satd. Flow (perm)	1250	1535	0	1143	1566	0	467	3280	0	579	3278	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		128			46			14			15	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Adj. Flow (vph)	95	37	128	53	20	46	134	727	66	66	696	72
Shared Lane Traffic (%)		<u> </u>										
Lane Group Flow (vph)	95	165	0	53	66	0	134	793	0	66	768	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ŭ		12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

06/18/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	22.0	22.0		28.0	28.0		13.0	28.8		13.0	22.8	
Total Split (s)	29.0	29.0		29.0	29.0		18.0	47.0		14.0	43.0	
Total Split (%)	32.2%	32.2%		32.2%	32.2%		20.0%	52.2%		15.6%	47.8%	
Maximum Green (s)	25.0	25.0		25.0	25.0		13.2	42.2		9.2	38.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		17.0	17.0			17.0			11.0	
Pedestrian Calls (#/hr)	1	1		0	0			0			1	
Act Effct Green (s)	8.9	8.9		8.9	8.9		23.3	19.4		20.9	16.3	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.52	0.43		0.47	0.36	
v/c Ratio	0.38	0.41		0.23	0.19		0.32	0.56		0.16	0.64	
Control Delay	21.6	9.4		19.1	9.8		7.6	13.6		6.4	16.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	21.6	9.4		19.1	9.8		7.6	13.6		6.4	16.5	
LOS	С	Α		В	Α		Α	В		Α	В	
Approach Delay		13.9			13.9			12.7			15.7	
Approach LOS		В			В			В			В	

#### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 44.9

Natural Cycle: 70

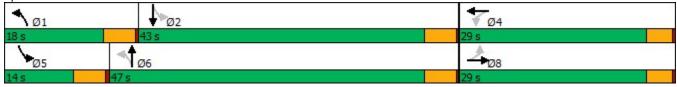
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 14.1 Intersection LOS: B
Intersection Capacity Utilization 59.2% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2021 PM DESIGN HOUR 4:30 pm 03/19/2020 2021 PM Build Conditions DNH

Synchro 7 - Report Page 2

### 2021 PM DESIGN HOUR BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/18/2020

	۶	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	~	-	<b></b>	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	7		7	₽		7	<b>↑</b> ↑		7	<b>↑</b> ↑	
Traffic Volume (vph)	89	35	120	50	19	43	126	683	62	62	654	68
Future Volume (vph)	89	35	120	50	19	43	126	683	62	62	654	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.88		1.00	0.90		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1534		1662	1567		1662	3278		1662	3278	
Flt Permitted	0.71	1.00		0.65	1.00		0.27	1.00		0.33	1.00	
Satd. Flow (perm)	1249	1534		1142	1567		467	3278		580	3278	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	95	37	128	53	20	46	134	727	66	66	696	72
RTOR Reduction (vph)	0	104	0	0	37	0	0	8	0	0	9	0
Lane Group Flow (vph)	95	61	0	53	29	0	134	785	0	66	759	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	9.4	9.4		9.4	9.4		25.5	19.9		21.7	18.0	
Effective Green, g (s)	8.9	8.9		8.9	8.9		24.5	19.4		20.7	17.5	
Actuated g/C Ratio	0.19	0.19		0.19	0.19		0.53	0.42		0.44	0.38	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	238	292		218	299		376	1364		331	1231	
v/s Ratio Prot		0.04			0.02		c0.04	c0.24		0.01	0.23	
v/s Ratio Perm	c0.08			0.05			0.15			0.07		
v/c Ratio	0.40	0.21		0.24	0.10		0.36	0.58		0.20	0.62	
Uniform Delay, d1	16.5	15.9		16.0	15.5		6.0	10.4		7.5	11.8	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.8	0.3		0.4	0.1		0.4	0.5		0.2	0.8	
Delay (s)	17.3	16.2		16.4	15.6		6.5	10.9		7.7	12.6	
Level of Service	В	В		В	В		Α	В		Α	В	
Approach Delay (s)		16.6			16.0			10.3			12.2	
Approach LOS		В			В			В			В	
Intersection Summary												
HCM 2000 Control Delay			12.1	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capa	city ratio		0.52									
Actuated Cycle Length (s)			46.6		um of lost				15.1			
Intersection Capacity Utiliza	tion		59.2%	IC	U Level o	of Service	9		В			
Analysis Period (min)			15									
c Critical Lane Group												

2021 PM DESIGN HOUR 4:30 pm 03/19/2020 2021 PM Build Conditions DNH

### 2021 PM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/18/2020

	٠	<b>→</b>	•	•	•	*	1	<b>†</b>	/	-	<b>↓</b>	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	43	59	57	29	46	0	40	2	17	0	1	26
Future Volume (vph)	43	59	57	29	46	0	40	2	17	0	1	26
ldeal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.951						0.961			0.869	
Flt Protected		0.987			0.981			0.967				
Satd. Flow (prot)	0	1643	0	0	1717	0	0	1626	0	0	1521	0
FIt Permitted		0.987			0.981			0.967				
Satd. Flow (perm)	0	1643	0	0	1717	0	0	1626	0	0	1521	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			225			457			251	
Travel Time (s)		8.6			6.1			12.5			5.7	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Adj. Flow (vph)	52	72	70	35	56	0	49	2	21	0	1	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	194	0	0	91	0	0	72	0	0	33	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 28.3%

ICU Level of Service A

Analysis Period (min) 15

### HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

06/18/2020

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	43	59	57	29	46	0	40	2	17	0	1	26
Future Vol, veh/h	43	59	57	29	46	0	40	2	17	0	1	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	52	72	70	35	56	0	49	2	21	0	1	32
Major/Minor N	Major1		ı	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	56	0	0	142	0	0	354	337	107	349	372	56
Stage 1	-	-	-	-	-	-	211	211	-	126	126	-
Stage 2	-	-	-	-	-	-	143	126	-	223	246	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1562	-	-	1453	-	-	605	587	953	609	561	1016
Stage 1	-	-	-	-	-	-	796	731	-	883	796	-
Stage 2	-	-	-	-	-	-	865	796	-	784	706	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1562	-	-	1453	-	-	558	552	953	566	527	1016
Mov Cap-2 Maneuver	-	-	-	-	-	-	558	552	-	566	527	-
Stage 1	-	-	-	-	-	-	767	705	-	851	776	-
Stage 2	-	-	-	-	-	-	816	776	-	737	681	-
Ü												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2			2.9			11.4			8.8		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		633	1562	-		1453	-	-	982			
HCM Lane V/C Ratio		0.114		-		0.024	-	-	0.034			
HCM Control Delay (s)		11.4	7.4	0	_	7.5	0	_	8.8			
HCM Lane LOS		В	Α	A	-	A	A	-	A			
HCM 95th %tile Q(veh)		0.4	0.1	-	_	0.1	-	_	0.1			

# Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

	1	*	<b>†</b>	-	1	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Volume (vph)	0	20	809	6	0	784
Future Volume (vph)	0	20	809	6	0	784
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.865	0.999			
Flt Protected						
Satd. Flow (prot)	0	1514	3322	0	0	3325
Flt Permitted						
Satd. Flow (perm)	0	1514	3322	0	0	3325
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	21	861	6	0	834
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	21	867	0	0	834
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free

### Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 34.5%

Analysis Period (min) 15

ICU Level of Service A

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	1.00	7	<b>†</b>	11511	UDL	<b>^</b>
Traffic Vol, veh/h	0	20	809	6	0	784
Future Vol, veh/h	0	20	809	6	0	784
Conflicting Peds, #/hr	0	0	009	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	None
Storage Length	-	0	_	NOHE -	_	-
Veh in Median Storage						
•	-	-	0	-	-	0
Grade, %	0	- 04	0	-	- 04	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	21	861	6	0	834
Major/Minor I	Minor1	N	/lajor1	٨	/lajor2	
Conflicting Flow All	_	434	0	0	-	
Stage 1	_	-	-	-	_	_
Stage 2	_	_	_	_	_	
Critical Hdwy		6.9		_	_	
	-	0.9	_	-	-	-
Critical Hdwy Stg 1		-		-		-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	576	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	576	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annragah	WB		ND		SB	
Approach			NB			
HCM Control Delay, s	11.5		0		0	
HCM LOS	В					
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-		-	
HCM Lane V/C Ratio				0.037	_	
HCM Control Delay (s)		-	<u>-</u>	11.5	_	
HCM Lane LOS		_	_	11.5 B	-	
HCM 95th %tile Q(veh)		-	-	0.1	-	
HOW SOUT WILL W(VEII)		-	-	0.1	-	

	٠		`	_	+	•	4	†	<b>/</b>	1	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LDL		EDI	YVDL	₩ <u>₽</u>	WDR	NDL	<b>↑</b>	NDI	SDL	<b>↑</b> ↑	SBN
Traffic Volume (vph)	73	<b>1</b> →	102	56	31	31	43	340	26	48	358	45
Future Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110	1730	0	141	1730	0	146	1730	0	91	1750	0
Storage Lanes	1		0	1		0	140		0	1		0
Taper Length (ft)	100		U	55		U	66		U	54		U
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt	1.00	0.872	1.00	1.00	0.925	1.00	1.00	0.989	0.55	1.00	0.983	0.55
FIt Protected	0.950	0.012		0.950	0.323		0.950	0.505		0.950	0.505	
Satd. Flow (prot)	1662	1513	0	1662	1619	0	1662	3199	0	1662	3204	0
Flt Permitted	0.705	1010	U	0.660	1013	· ·	0.461	0100	U	0.475	0204	U
Satd. Flow (perm)	1234	1513	0	1155	1619	0	807	3199	0	831	3204	0
Right Turn on Red	1204	1010	Yes	1100	1013	Yes	001	0100	Yes	001	0204	Yes
Satd. Flow (RTOR)		131	103		40	103		10	103		17	103
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Heavy Vehicles (%)	0.70	0.76	1%	0.70	0.70	0.70	0.70	3%	0.70	0.70	2%	2%
Adj. Flow (vph)	94	22	131	72	40	40	55	436	33	62	459	58
Shared Lane Traffic (%)	J <del>-1</del>	22	131	12	70	70	55	430	33	02	400	30
Lane Group Flow (vph)	94	153	0	72	80	0	55	469	0	62	517	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Loit	12	rtigitt	LOIL	12	rtigit	LOIL	12	rtigit	LOIL	12	rtigitt
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes			12			Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel	OI - EX	OI LA		O. L.	OI EX		OI LX	O. LA		O. LX	OI EX	
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel	OI. LX	OI LX		OI. LX	OI. LX		OI. LX	OI LX		OI LA	OI LX	
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	. 01111	8		. 01111	4		1	6		5	2	
Permitted Phases	8	-		4	<b>-</b>		6	-		2		
- orinition i ridoco	0			7			U					

# Lanes, Volumes, Timings 1: HWY 101 & 35th Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	28.0	28.0		28.0	28.0		13.0	22.8		13.0	22.8	
Total Split (s)	34.0	34.0		34.0	34.0		17.0	41.0		15.0	39.0	
Total Split (%)	37.8%	37.8%		37.8%	37.8%		18.9%	45.6%		16.7%	43.3%	
Maximum Green (s)	30.0	30.0		30.0	30.0		12.2	36.2		10.2	34.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	17.0	17.0		17.0	17.0			11.0			11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0			0	
Act Effct Green (s)	7.8	7.8		7.7	7.7		13.3	12.9		13.6	13.1	
Actuated g/C Ratio	0.25	0.25		0.25	0.25		0.43	0.42		0.44	0.42	
v/c Ratio	0.30	0.32		0.25	0.19		0.11	0.35		0.12	0.38	
Control Delay	14.7	6.5		14.2	8.7		6.1	11.2		6.0	11.0	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	14.7	6.5		14.2	8.7		6.1	11.2		6.0	11.0	
LOS	В	Α		В	Α		Α	В		Α	В	
Approach Delay		9.6			11.3			10.7			10.5	
Approach LOS		Α			В			В			В	

### Intersection Summary

Area Type: Other

Cycle Length: 90 Actuated Cycle Length: 31

Natural Cycle: 65

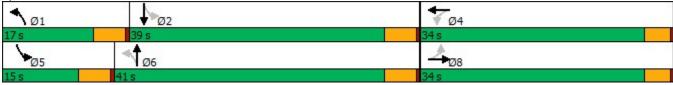
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.38

Intersection Signal Delay: 10.5 Intersection LOS: B
Intersection Capacity Utilization 39.3% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2026 AM DESIGN HOUR  $\,8:00$  am  $\,03/19/2020$  2026 AM Build Conditions DNH

Synchro 7 - Report Page 2

# 2026 AM DESIGN HOUR BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/18/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1		7	1		7	<b>†</b>		7	<b>†</b>	
Traffic Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Future Volume (vph)	73	17	102	56	31	31	43	340	26	48	358	45
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.87		1.00	0.93		1.00	0.99		1.00	0.98	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1512		1662	1619		1662	3201		1662	3205	
FIt Permitted	0.70	1.00		0.66	1.00		0.46	1.00		0.47	1.00	
Satd. Flow (perm)	1234	1512		1155	1619		807	3201		831	3205	
Peak-hour factor, PHF	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78
Adj. Flow (vph)	94	22	131	72	40	40	55	436	33	62	459	58
RTOR Reduction (vph)	0	107	0	0	33	0	0	7	0	0	11	0
Lane Group Flow (vph)	94	46	0	72	47	0	55	462	0	62	506	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	3%	0%	0%	2%	2%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	6.9	6.9		6.9	6.9		14.0	12.1		14.4	12.3	
Effective Green, g (s)	6.4	6.4		6.4	6.4		13.0	11.6		13.4	11.8	
Actuated g/C Ratio	0.18	0.18		0.18	0.18		0.37	0.33		0.39	0.34	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	227	278		213	298		336	1070		359	1089	_
v/s Ratio Prot		0.03			0.03		0.01	0.14		c0.01	c0.16	
v/s Ratio Perm	c0.08			0.06			0.05			0.06		
v/c Ratio	0.41	0.17		0.34	0.16		0.16	0.43		0.17	0.46	
Uniform Delay, d1	12.5	11.9		12.3	11.9		7.0	9.0		6.8	9.0	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.9	0.2		0.7	0.2		0.2	0.2		0.2	0.2	
Delay (s)	13.4	12.1		13.0	12.1		7.2	9.2		7.0	9.2	
Level of Service	В	В		В	В		Α	Α		Α	Α	
Approach Delay (s)		12.6			12.5			9.0			9.0	
Approach LOS		В			В			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			9.9	H	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capaci	ty ratio		0.43									
Actuated Cycle Length (s)			34.7	Sı	um of lost	time (s)			15.1			
Intersection Capacity Utilizati	on		39.3%		U Level o	٠,	9		Α			
Analysis Period (min)			15									
c Critical Lane Group												

2026 AM DESIGN HOUR 8:00 am 03/19/2020 2026 AM Build Conditions DNH

### 2026 AM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOD ST/Site & 35th Street

06/18/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	52	20	19	15	48	0	37	2	9	0	2	33
Future Volume (vph)	52	20	19	15	48	0	37	2	9	0	2	33
ldeal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.972						0.974			0.874	
Flt Protected		0.972			0.988			0.963				
Satd. Flow (prot)	0	1653	0	0	1729	0	0	1641	0	0	1530	0
Flt Permitted		0.972			0.988			0.963				
Satd. Flow (perm)	0	1653	0	0	1729	0	0	1641	0	0	1530	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			493			457			274	
Travel Time (s)		8.6			13.4			12.5			6.2	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Adj. Flow (vph)	68	26	25	19	62	0	48	3	12	0	3	43
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	119	0	0	81	0	0	63	0	0	46	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	
Intersection Cummens												

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 28.0%

Analysis Period (min) 15

ICU Level of Service A

# HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

06/18/2020

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	52	20	19	15	48	0	37	2	9	0	2	33
Future Vol, veh/h	52	20	19	15	48	0	37	2	9	0	2	33
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	68	26	25	19	62	0	48	3	12	0	3	43
Major/Minor N	/lajor1			Major2		1	Minor1		N	/linor2		
Conflicting Flow All	62	0	0	51	0	0	298	275	39	282	287	62
Stage 1	-	-	-	-	-	-	175	175	-	100	100	-
Stage 2	-	-	-	-	-	-	123	100	-	182	187	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	_	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	_	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1554	-	-	1568	-	-	658	636	1038	674	626	1009
Stage 1	_	-	-	_	-	-	832	758	-	911	816	-
Stage 2	-	-	-	-	-	-	886	816	-	824	749	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1554	-	-	1568	-	-	600	600	1038	635	590	1009
Mov Cap-2 Maneuver	-	-	-	-	-	-	600	600	-	635	590	-
Stage 1	-	-	-	-	-	-	795	724	-	870	805	-
Stage 2	-	-	-	-	-	-	835	805	-	775	715	-
Ü												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.2			1.7			11.1			8.9		
HCM LOS							В			Α		
							_			-		
Minor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		652	1554	_	_		_	_	970			
HCM Lane V/C Ratio		0.096		_		0.012	_		0.047			
HCM Control Delay (s)		11.1	7.4	0	-	7.3	0	-	8.9			
HCM Lane LOS		В	A	A	_	A	A	_	A			
HCM 95th %tile Q(veh)		0.3	0.1	-	-	0	-	-	0.1			
		0.0	<b>V</b> .,						<b>J</b> .,			

# 2026 AM DESIGN HOUR BUILD CONDITIONS

# Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b> 1>			<b>^</b>
Traffic Volume (vph)	0	24	437	7	0	451
Future Volume (vph)	0	24	437	7	0	451
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.865	0.998			
Flt Protected						
Satd. Flow (prot)	0	1514	3254	0	0	3260
Flt Permitted						
Satd. Flow (perm)	0	1514	3254	0	0	3260
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	0%	0%	2%	0%	0%	2%
Adj. Flow (vph)	0	29	533	9	0	550
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	29	542	0	0	550
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

ICU Level of Service A

Intersection Capacity Utilization 23.4%
Analysis Period (min) 15

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	1100	7	<b>†</b>	HDIT	ODL	<b>^</b>
Traffic Vol, veh/h	0	24	437	7	0	451
Future Vol, veh/h	0	24	437	7	0	451
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	
Storage Length	_	0	_	-	-	110116
Veh in Median Storage		-	0			0
Grade, %	, # 0 0	-	0	_	-	0
Peak Hour Factor	82	82	82	82	82	82
	02	02	2		02	2
Heavy Vehicles, %				0		
Mvmt Flow	0	29	533	9	0	550
Major/Minor I	Minor1	N	Major1		/lajor2	
Conflicting Flow All	-	271	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	_	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	733	_	-	0	_
Stage 1	0	-	_	_	0	_
Stage 2	0	-	_	-	0	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	-	733	_	-	_	_
Mov Cap-2 Maneuver	_	-	_	_	_	_
Stage 1	_	_	_	_	_	_
Stage 2	_		_	_		_
Olaye Z				_		
Approach	WB		NB		SB	
HCM Control Delay, s	10.1		0		0	
HCM LOS	В					
Minor Long /Mailer Ed		NDT	NDDV	MDI 4	ODT	
	IT	NRI				
		-	-		-	
		-	-		-	
		-	-		-	
		-	-		-	
HCM 95th %tile Q(veh)			-		_	
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS		-	- - -	0.04	- - -	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	7>		*	f)		*	<b>†</b>		7	<b>↑</b> ↑	
Traffic Volume (vph)	89	35	120	50	19	43	126	684	62	62	655	68
Future Volume (vph)	89	35	120	50	19	43	126	684	62	62	655	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	110		0	141		0	146		0	91		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			55			66			54		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Frt		0.884			0.895			0.988			0.986	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1662	1535	0	1662	1566	0	1662	3280	0	1662	3278	0
Flt Permitted	0.714			0.653			0.267			0.331		
Satd. Flow (perm)	1250	1535	0	1143	1566	0	467	3280	0	579	3278	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		128			46			14			15	
Link Speed (mph)		25			25			40			40	
Link Distance (ft)		499			317			925			716	
Travel Time (s)		13.6			8.6			15.8			12.2	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Adj. Flow (vph)	95	37	128	53	20	46	134	728	66	66	697	72
Shared Lane Traffic (%)												
Lane Group Flow (vph)	95	165	0	53	66	0	134	794	0	66	769	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12	- J		12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane		Yes						Yes			Yes	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	2		2	2		2	2		2	2	
Detector Template	Side St	Side St		Side St	Side St		Left			Left		
Leading Detector (ft)	78	78		78	78		78	223		78	223	
Trailing Detector (ft)	2	2		2	2		2	157		2	157	
Detector 1 Position(ft)	2	2		2	2		2	157		2	157	
Detector 1 Size(ft)	16	16		16	16		16	6		16	6	
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	72		72	72		72	217		72	217	
Detector 2 Size(ft)	6	6		6	6		6	6		6	6	
Detector 2 Type	CI+Ex	Cl+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		

# Lanes, Volumes, Timings 1: HWY 101 & 35th Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	8	8		4	4		1	6		5	2	
Switch Phase												
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	22.0	22.0		28.0	28.0		13.0	28.8		13.0	22.8	
Total Split (s)	29.0	29.0		29.0	29.0		18.0	47.0		14.0	43.0	
Total Split (%)	32.2%	32.2%		32.2%	32.2%		20.0%	52.2%		15.6%	47.8%	
Maximum Green (s)	25.0	25.0		25.0	25.0		13.2	42.2		9.2	38.2	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.3	4.3		4.3	4.3	
All-Red Time (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Lost Time Adjust (s)	0.5	0.5		0.5	0.5		0.5	0.5		0.5	0.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Minimum Gap (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Time Before Reduce (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Time To Reduce (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Recall Mode	None	None		None	None		None	Min		None	Min	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0			7.0	
Flash Dont Walk (s)	11.0	11.0		17.0	17.0			17.0			11.0	
Pedestrian Calls (#/hr)	1	1		0	0			0			1	
Act Effct Green (s)	8.9	8.9		8.9	8.9		23.3	19.4		20.9	16.3	
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.52	0.43		0.47	0.36	
v/c Ratio	0.38	0.41		0.23	0.19		0.32	0.56		0.16	0.64	
Control Delay	21.6	9.4		19.1	9.8		7.6	13.6		6.4	16.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	21.6	9.4		19.1	9.8		7.6	13.6		6.4	16.5	
LOS	С	Α		В	Α		Α	В		Α	В	
Approach Delay		13.9			13.9			12.7			15.7	
Approach LOS		В			В			В			В	

### Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 44.9

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 14.1 Intersection LOS: B
Intersection Capacity Utilization 59.3% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: HWY 101 & 35th Street



2026 PM DESIGN HOUR 4:30 pm 03/19/2020 2021 PM Build Conditions DNH

Synchro 7 - Report Page 2

# 2026 PM DESIGN HOUR BUILD CONDITIONS

# HCM Signalized Intersection Capacity Analysis 1: HWY 101 & 35th Street

06/18/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1→		7	1		*	<b>↑</b> ↑		7	<b>^</b>	
Traffic Volume (vph)	89	35	120	50	19	43	126	684	62	62	655	68
Future Volume (vph)	89	35	120	50	19	43	126	684	62	62	655	68
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		5.3	5.3		5.3	5.3	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	0.95		1.00	0.95	
Frt	1.00	0.88		1.00	0.90		1.00	0.99		1.00	0.99	
FIt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1534		1662	1567		1662	3278		1662	3278	
FIt Permitted	0.71	1.00		0.65	1.00		0.27	1.00		0.33	1.00	
Satd. Flow (perm)	1249	1534		1142	1567		467	3278		579	3278	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	95	37	128	53	20	46	134	728	66	66	697	72
RTOR Reduction (vph)	0	104	0	0	37	0	0	8	0	0	9	0
Lane Group Flow (vph)	95	61	0	53	29	0	134	786	0	66	760	0
Heavy Vehicles (%)	0%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		8			4		1	6		5	2	
Permitted Phases	8			4			6			2		
Actuated Green, G (s)	9.4	9.4		9.4	9.4		25.5	19.9		21.7	18.0	
Effective Green, g (s)	8.9	8.9		8.9	8.9		24.5	19.4		20.7	17.5	
Actuated g/C Ratio	0.19	0.19		0.19	0.19		0.53	0.42		0.44	0.38	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.8	4.8		4.8	4.8	
Vehicle Extension (s)	2.5	2.5		2.5	2.5		2.5	2.5		2.5	2.5	
Lane Grp Cap (vph)	238	292		218	299		376	1364		331	1231	
v/s Ratio Prot		0.04			0.02		c0.04	c0.24		0.01	0.23	
v/s Ratio Perm	c0.08			0.05			0.15			0.07		
v/c Ratio	0.40	0.21		0.24	0.10		0.36	0.58		0.20	0.62	
Uniform Delay, d1	16.5	15.9		16.0	15.5		6.0	10.4		7.5	11.8	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	0.8	0.3		0.4	0.1		0.4	0.5		0.2	0.8	
Delay (s)	17.3	16.2		16.4	15.6		6.5	10.9		7.7	12.6	
Level of Service	В	В		В	В		Α	В		Α	В	
Approach Delay (s)		16.6			16.0			10.3			12.2	
Approach LOS		В			В			В			В	
Intersection Summary												
HCM 2000 Control Delay			12.1	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.52									
Actuated Cycle Length (s)			46.6		um of lost				15.1			
Intersection Capacity Utilizat	tion		59.3%	IC	U Level o	of Service	9		В			
Analysis Period (min)			15									
c Critical Lane Group												

2026 PM DESIGN HOUR 4:30 pm 03/19/2020 2021 PM Build Conditions DNH

### 2026 PM DESIGN HOUR BUILD CONDITIONS

Lanes, Volumes, Timings

2: REDWOOI	ST/Site &	35th Street

06/18/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	43	59	57	29	46	0	40	2	17	0	1	26
Future Volume (vph)	43	59	57	29	46	0	40	2	17	0	1	26
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.951						0.961			0.869	
Flt Protected		0.987			0.981			0.967				
Satd. Flow (prot)	0	1643	0	0	1717	0	0	1626	0	0	1521	0
Flt Permitted		0.987			0.981			0.967				
Satd. Flow (perm)	0	1643	0	0	1717	0	0	1626	0	0	1521	0
Link Speed (mph)		25			25			25			30	
Link Distance (ft)		317			225			457			251	
Travel Time (s)		8.6			6.1			12.5			5.7	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Adj. Flow (vph)	52	72	70	35	56	0	49	2	21	0	1	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	194	0	0	91	0	0	72	0	0	33	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			35	
Crosswalk Width(ft)		12			12			12			12	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type: Other

Control Type: Unsignalized Intersection Capacity Utilization 28.3%

Analysis Period (min) 15

ICU Level of Service A

# HCM 2010 TWSC 2: REDWOOD ST/Site & 35th Street

06/18/2020

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	43	59	57	29	46	0	40	2	17	0	1	26
Future Vol, veh/h	43	59	57	29	46	0	40	2	17	0	1	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	52	72	70	35	56	0	49	2	21	0	1	32
Major/Minor N	/lajor1		_	Major2		_	Minor1		N	/linor2		
Conflicting Flow All	56	0	0	142	0	0	354	337	107	349	372	56
Stage 1	-	-	-	-	-	-	211	211	-	126	126	-
Stage 2	_	-	-	_	-	-	143	126	-	223	246	-
Critical Hdwy	4.1	_	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1562	-	-	1453	-	-	605	587	953	609	561	1016
Stage 1	-	-	-	-	-	-	796	731	-	883	796	-
Stage 2	_	-	-	-	-	-	865	796	-	784	706	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1562	-	-	1453	-	-	558	552	953	566	527	1016
Mov Cap-2 Maneuver	-	-	-	-	-	-	558	552	-	566	527	-
Stage 1	-	-	-	-	-	-	767	705	-	851	776	-
Stage 2	-	-	-	-	-	-	816	776	-	737	681	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2			2.9			11.4			8.8		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		633	1562	-		1453	_	-	982			
HCM Lane V/C Ratio		0.114		-		0.024	_		0.034			
HCM Control Delay (s)		11.4	7.4	0	-	7.5	0	-	8.8			
HCM Lane LOS		В	Α	A	-	A	A	-	A			
HCM 95th %tile Q(veh)		0.4	0.1	-	-	0.1	-	-	0.1			
						-						

# Lanes, Volumes, Timings 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

	1	*	<b>†</b>	-	-	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Volume (vph)	0	20	810	6	0	785
Future Volume (vph)	0	20	810	6	0	785
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Frt		0.865	0.999			
Flt Protected						
Satd. Flow (prot)	0	1514	3322	0	0	3325
Flt Permitted						
Satd. Flow (perm)	0	1514	3322	0	0	3325
Link Speed (mph)	30		40			40
Link Distance (ft)	243		716			923
Travel Time (s)	5.5		12.2			15.7
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	0	21	862	6	0	835
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	21	868	0	0	835
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		12	_		12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	12		12			12
Two way Left Turn Lane			Yes			Yes
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11
Sign Control	Stop		Free			Free
Intersection Summary						

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Intersection Capacity Utilization 34.5%

Analysis Period (min) 15

ICU Level of Service A

# HCM 2010 TWSC 3: HWY 101 & RIRO SITE ACCESS

06/18/2020

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b>			<b>^</b>
Traffic Vol, veh/h	0	20	810	6	0	785
Future Vol, veh/h	0	20	810	6	0	785
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	_	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	21	862	6	0	835
	•			_	•	
Major/Minor	Minar1		laior1	Λ.	/aiar0	
	Minor1		/lajor1		/lajor2	
Conflicting Flow All	-	434	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	576	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	576	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.5		0		0	
HCM LOS	11.3 B		U		U	
TICIVI LOS	D					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	576	-	
HCM Lane V/C Ratio		-	-	0.037	-	
HCM Control Delay (s)		-	-	11.5	-	
HCM Lane LOS		-	-	В	-	
HCM 95th %tile Q(veh)		-	-	0.1	-	

# **APPENDIX I**

Existing and No-Build SimTraffic Queue Length Calculations

<sup>©</sup> Branch Engineering, Inc.

06/12/2020

Intersection: 1: HWY 101 & 35th Street, Interval #1
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Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	66	66	49	50	54	122	77	53	119	142	
Average Queue (ft)	40	44	24	31	26	71	33	21	74	76	
95th Queue (ft)	73	67	57	58	58	121	81	62	125	143	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)						0			4		
Queuing Penalty (veh)						0			1		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	79	64	40	64	39	107	59	44	101	75	
Average Queue (ft)	32	35	18	27	16	48	20	9	48	32	
95th Queue (ft)	66	60	46	57	43	89	50	34	83	67	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0					0			0		
Queuing Penalty (veh)	0					0			0		

### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	84	66	54	64	54	127	82	63	124	142	
Average Queue (ft)	34	37	19	28	18	53	23	12	54	42	
95th Queue (ft)	68	62	49	58	48	100	59	43	98	97	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0					0			1		
Queuing Penalty (veh)	0					0			0		

# Queuing and Blocking Report 2020 CONDITIONS

06/12/2020

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	16	41
Average Queue (ft)	2	29
95th Queue (ft)	19	56
Link Distance (ft)	468	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	NB
Directions Served	LTR
Maximum Queue (ft)	40
Average Queue (ft)	28
95th Queue (ft)	52
Link Distance (ft)	427
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	16	46
Average Queue (ft)	1	28
95th Queue (ft)	9	53
Link Distance (ft)	468	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Queuing and Blocking Report 2020 CONDITIONS

06/12/2020

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### **Network Summary**

Network wide Queuing Penalty, Interval #1: 1

Network wide Queuing Penalty, Interval #2: 0

Network wide Queuing Penalty, All Intervals: 0

# Queuing and Blocking Report

EXISTING 06/15/2020

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	74	100	49	53	84	153	132	78	150	124	
Average Queue (ft)	43	59	25	27	50	89	73	35	97	82	
95th Queue (ft)	78	105	59	59	87	152	134	84	160	134	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)		1				1			10		
Queuing Penalty (veh)		1				1			4		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	91	117	69	69	92	138	126	70	175	158	
Average Queue (ft)	44	53	19	31	45	77	60	24	95	82	
95th Queue (ft)	81	95	53	58	78	130	104	58	154	145	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0				0			8		
Queuing Penalty (veh)	0	0				0			3		

### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	96	126	70	69	102	161	150	108	178	158	
Average Queue (ft)	44	54	21	30	46	80	63	27	96	82	
95th Queue (ft)	80	97	55	59	80	136	113	65	155	142	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0				0			9		
Queuing Penalty (veh)	0	0				0			4		

# Queuing and Blocking Report

EXISTING 06/15/2020

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	26	55
Average Queue (ft)	4	37
95th Queue (ft)	28	57
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	21	52
Average Queue (ft)	2	34
95th Queue (ft)	12	55
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	37	61
Average Queue (ft)	2	35
95th Queue (ft)	17	56
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Queuing and Blocking Report

EXISTING 06/15/2020

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### **Network Summary**

Network wide Queuing Penalty, Interval #1: 6

Network wide Queuing Penalty, Interval #2: 4

Network wide Queuing Penalty, All Intervals: 4

2020 PM DESIGN HOUR

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SimTraffic Report

Page 3

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06/15/2020

Intersection:	1· HWY 101	& 35th Street,	Interval #1
11110130011011.	1. 1 1 7 7 1 1 0 1	a John Ohlock.	m = m + m = m

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	101	76	35	54	54	110	105	49	121	119	
Average Queue (ft)	53	42	20	35	35	73	49	18	80	62	
95th Queue (ft)	102	71	48	56	58	119	105	51	132	124	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	1					0			5		
Queuing Penalty (veh)	1					0			1		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	81	64	45	59	54	84	66	39	119	117	
Average Queue (ft)	37	37	14	26	20	45	19	12	48	35	
95th Queue (ft)	72	61	44	56	50	82	54	38	91	84	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0								1		
Queuing Penalty (veh)	0								0		

### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	117	76	45	65	65	110	105	54	143	137	
Average Queue (ft)	41	38	16	28	24	52	27	13	56	42	
95th Queue (ft)	82	64	45	57	54	96	73	42	106	97	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0					0			2		
Queuing Penalty (veh)	0					0			0		

# Queuing and Blocking Report 2021 AM No-Build Conditions

06/15/2020

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	NB
Directions Served	LTR
Maximum Queue (ft)	56
Average Queue (ft)	36
95th Queue (ft)	57
Link Distance (ft)	427
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	10	46
Average Queue (ft)	0	25
95th Queue (ft)	6	52
Link Distance (ft)	468	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	10	56
Average Queue (ft)	0	27
95th Queue (ft)	5	55
Link Distance (ft)	468	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Queuing and Blocking Report 2021 AM No-Build Conditions

06/15/2020

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)

Queuing Penalty (veh) Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

#### Movement

Directions Served

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### **Network Summary**

Network wide Queuing Penalty, Interval #1: 2

Network wide Queuing Penalty, Interval #2: 0

Network wide Queuing Penalty, All Intervals: 1

# Queuing and Blocking Report 2021 PM Background Conditions

06/15/2020

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	76	106	54	63	102	167	151	66	173	157	
Average Queue (ft)	41	60	22	32	63	94	74	29	108	92	
95th Queue (ft)	81	116	56	68	102	160	149	73	175	163	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	1				1			11		
Queuing Penalty (veh)	0	1				1			5		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	99	109	44	71	86	165	129	65	173	183	
Average Queue (ft)	47	49	16	29	47	77	59	24	88	81	
95th Queue (ft)	79	90	46	62	84	136	112	56	146	149	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0		0		1		0	7		
Queuing Penalty (veh)	0	0		0		1		0	3		

### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	105	126	54	75	104	195	168	81	182	185	
Average Queue (ft)	46	52	18	29	51	81	63	25	93	84	
95th Queue (ft)	80	97	48	64	90	143	123	60	155	152	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0		0		1		0	8		
Queuing Penalty (veh)	0	0		0		1		0	3		

# Queuing and Blocking Report 2021 PM Background Conditions

06/15/2020

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	36	55
Average Queue (ft)	8	34
95th Queue (ft)	34	64
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	16	52
Average Queue (ft)	1	28
95th Queue (ft)	10	54
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	36	65
Average Queue (ft)	3	30
95th Queue (ft)	18	57
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Queuing and Blocking Report 2021 PM Background Conditions

06/15/2020

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

#### Movement

Directions Served

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### **Network Summary**

Network wide Queuing Penalty, Interval #1: 7

Network wide Queuing Penalty, Interval #2: 4

Network wide Queuing Penalty, All Intervals: 5

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	68	64	35	54	52	134	74	34	100	105	
Average Queue (ft)	42	43	21	30	27	63	37	16	67	54	
95th Queue (ft)	75	73	48	65	59	124	87	43	108	103	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)						1			3		
Queuing Penalty (veh)						0			1		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	70	64	45	55	50	105	66	39	99	81	
Average Queue (ft)	32	35	15	28	21	49	23	15	50	35	
95th Queue (ft)	65	63	44	53	50	91	55	41	85	73	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)									1		
Queuing Penalty (veh)									0		

### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	77	74	45	60	53	136	84	39	105	110	
Average Queue (ft)	35	37	17	28	22	53	26	15	54	39	
95th Queue (ft)	69	66	46	56	52	101	65	42	93	83	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)						0			1		
Queuing Penalty (veh)						0			0		

# Queuing and Blocking Report 2026 AM No-Build Conditions

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# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	NB
Directions Served	LTR
Maximum Queue (ft)	35
Average Queue (ft)	27
95th Queue (ft)	51
Link Distance (ft)	427
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	10	61
Average Queue (ft)	1	30
95th Queue (ft)	8	56
Link Distance (ft)	468	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	10	61
Average Queue (ft)	1	29
95th Queue (ft)	7	55
Link Distance (ft)	468	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Queuing and Blocking Report 2026 AM No-Build Conditions

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### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (%)
Queuing Penalty (veh)
Storage Bay Dist (ft)

Storage Blk Time (%)
Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

#### Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### **Network Summary**

Network wide Queuing Penalty, Interval #1: 1

Network wide Queuing Penalty, Interval #2: 0

Network wide Queuing Penalty, All Intervals: 0

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Intersection:	1. HWY 101	& 35th Street	Interval #1
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Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	81	84	53	58	89	133	154	88	171	148	
Average Queue (ft)	42	51	20	32	52	88	66	36	110	100	
95th Queue (ft)	86	83	55	65	93	153	140	88	179	163	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0				1			10		
Queuing Penalty (veh)	1	0				1			4		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	94	100	54	68	92	154	131	76	182	166	
Average Queue (ft)	47	50	21	30	46	77	60	24	94	83	
95th Queue (ft)	82	86	52	60	77	133	108	61	159	148	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0				0		0	8		
Queuing Penalty (veh)	0	0				0		0	3		

### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	98	105	64	78	99	156	160	106	200	178	
Average Queue (ft)	45	50	21	30	47	80	61	27	97	87	
95th Queue (ft)	83	85	53	61	81	139	117	69	165	153	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0				0		0	8		
Queuing Penalty (veh)	0	0				1		0	3		

# Queuing and Blocking Report 2026 PM No-Build Conditions

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# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	25	56
Average Queue (ft)	6	35
95th Queue (ft)	26	59
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	26	57
Average Queue (ft)	3	32
95th Queue (ft)	17	54
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	WB	NB
Directions Served	LTR	LTR
Maximum Queue (ft)	36	62
Average Queue (ft)	3	32
95th Queue (ft)	19	56
Link Distance (ft)	201	427
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Queuing and Blocking Report 2026 PM No-Build Conditions

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### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

# Movement Directions Served Maximum Queue (ft) Average Queue (ft) 95th Queue (ft) Link Distance (ft) Upstream Blk Time (%) Queuing Penalty (veh) Storage Bay Dist (ft)

Storage Blk Time (%)
Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

# Movement

**Directions Served** 

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

#### Movement

Directions Served

Maximum Queue (ft)

Average Queue (ft)

95th Queue (ft)

Link Distance (ft)

Upstream Blk Time (%)

Queuing Penalty (veh)

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

### **Network Summary**

Network wide Queuing Penalty, Interval #1: 6

Network wide Queuing Penalty, Interval #2: 4

Network wide Queuing Penalty, All Intervals: 5

# **APPENDIX J**

Build SimTraffic Queue Length Calculations

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mitoroccioni. Il rivi i io i a comi on cot, mitorian m	Intersection: 1: HWY	101 &	35th Street,	Interval #1
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Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	83	55	64	60	59	122	93	60	94	105	
Average Queue (ft)	41	41	34	34	32	77	46	36	66	62	
95th Queue (ft)	81	60	70	67	64	129	100	68	105	113	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0					0		0	2		
Queuing Penalty (veh)	0					0		0	1		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	Т	TR	
Maximum Queue (ft)	65	69	60	54	54	125	96	60	96	89	
Average Queue (ft)	33	37	32	26	23	52	32	23	51	37	
95th Queue (ft)	66	64	59	55	53	100	75	55	89	78	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)						0			1		
Queuing Penalty (veh)						0			0		

#### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	85	69	65	65	60	143	120	65	105	106	
Average Queue (ft)	35	38	32	28	26	58	35	26	55	43	
95th Queue (ft)	70	63	62	58	56	110	82	60	94	90	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0					0		0	1		
Queuing Penalty (veh)	0					0		0	0		

# Queuing and Blocking Report 2021 AM Build Conditions

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# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	9	5	41	34
Average Queue (ft)	1	1	28	26
95th Queue (ft)	10	8	53	48
Link Distance (ft)	212	468	427	240
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	17	10	46	45
Average Queue (ft)	1	0	29	21
95th Queue (ft)	11	6	55	50
Link Distance (ft)	212	468	427	240
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

#### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	22	15	52	45
Average Queue (ft)	1	1	29	23
95th Queue (ft)	11	6	54	50
Link Distance (ft)	212	468	427	240
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Queuing and Blocking Report 2021 AM Build Conditions

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#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement	WB
Directions Served	R
Maximum Queue (ft)	43
Average Queue (ft)	23
95th Queue (ft)	52
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

Movement	WB
Directions Served	R
Maximum Queue (ft)	40
Average Queue (ft)	17
95th Queue (ft)	46
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

Movement	WB
Directions Served	R
Maximum Queue (ft)	43
Average Queue (ft)	19
95th Queue (ft)	48
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### **Network Summary**

Network wide Queuing Penalty, Interval #1: 2
Network wide Queuing Penalty, Interval #2: 0
Network wide Queuing Penalty, All Intervals: 1

# Queuing and Blocking Report 2021 PM Build Conditions

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Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	93	80	53	54	83	161	161	81	172	167	
Average Queue (ft)	54	55	27	31	52	88	70	35	102	87	
95th Queue (ft)	92	84	60	55	87	147	135	85	176	169	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	1					1		0	12		
Queuing Penalty (veh)	1					2		1	8		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	Т	TR	
Maximum Queue (ft)	81	111	64	63	81	149	140	102	172	167	
Average Queue (ft)	44	53	33	30	46	82	68	37	93	85	
95th Queue (ft)	74	90	63	62	75	134	124	78	151	150	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)		0				0		0	7		
Queuing Penalty (veh)		0				0		0	5		

#### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	94	112	64	64	95	170	167	119	199	190	
Average Queue (ft)	46	53	31	31	47	84	69	36	95	86	
95th Queue (ft)	79	89	62	60	78	138	127	80	158	155	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	0				0		0	9		
Queuing Penalty (veh)	0	0				1		0	5		

#### Queuing and Blocking Report 2021 PM Build Conditions

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#### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	2	15	65	35
Average Queue (ft)	0	4	37	21
95th Queue (ft)	2	18	67	48
Link Distance (ft)	211	201	427	216
Upstream Blk Time (%)				
Queuing Penalty (veh)				
O( D D' (/())				

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

#### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	16	27	52	35
Average Queue (ft)	1	3	32	20
95th Queue (ft)	8	20	54	47
Link Distance (ft)	211	201	427	216
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				

Storage Blk Time (%)

Queuing Penalty (veh)

#### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	16	27	71	35
Average Queue (ft)	1	3	33	20
95th Queue (ft)	7	20	58	48
Link Distance (ft)	211	201	427	216
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

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# Queuing and Blocking Report 2021 PM Build Conditions

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# Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement	WB
Directions Served	R
Maximum Queue (ft)	35
Average Queue (ft)	22
95th Queue (ft)	48
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

Movement	WB
Directions Served	R
Maximum Queue (ft)	45
Average Queue (ft)	21
95th Queue (ft)	49
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

Movement	WB
Directions Served	R
Maximum Queue (ft)	45
Average Queue (ft)	21
95th Queue (ft)	49
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### **Network Summary**

Network wide Queuing Penalty, Interval #1: 11
Network wide Queuing Penalty, Interval #2: 5
Network wide Queuing Penalty, All Intervals: 7

# Queuing and Blocking Report 2026 AM Build Conditions

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Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	74	93	64	53	54	131	90	44	101	109	
Average Queue (ft)	47	48	38	30	29	66	46	26	66	58	
95th Queue (ft)	78	94	69	64	57	127	93	52	105	103	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)		0				0			2		
Queuing Penalty (veh)		0				0			1		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	65	62	62	65	63	123	96	86	107	113	
Average Queue (ft)	32	35	30	25	23	54	26	25	49	36	
95th Queue (ft)	57	59	58	55	54	104	71	63	86	76	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)						0		0	1		
Queuing Penalty (veh)						0		0	0		

#### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	79	93	66	73	64	136	101	86	125	127	
Average Queue (ft)	36	38	32	26	24	57	31	25	53	41	
95th Queue (ft)	64	70	61	58	55	110	79	61	93	85	
Link Distance (ft)		456		212		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)		0				0		0	1		
Queuing Penalty (veh)		0				0		0	0		

# Queuing and Blocking Report 2026 AM Build Conditions

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# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	5	15	46	34
Average Queue (ft)	1	2	31	28
95th Queue (ft)	5	14	61	48
Link Distance (ft)	212	468	427	240
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	EB	NB	SB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	8	50	39
Average Queue (ft)	1	24	23
95th Queue (ft)	5	52	49
Link Distance (ft)	212	427	240
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

#### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	10	15	52	39
Average Queue (ft)	1	1	26	24
95th Queue (ft)	5	6	55	49
Link Distance (ft)	212	468	427	240
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Queuing and Blocking Report 2026 AM Build Conditions

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#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement	WB
Directions Served	R
Maximum Queue (ft)	39
Average Queue (ft)	21
95th Queue (ft)	50
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

Movement	WB
Directions Served	R
Maximum Queue (ft)	48
Average Queue (ft)	19
95th Queue (ft)	49
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

Movement	WB
Directions Served	R
Maximum Queue (ft)	48
Average Queue (ft)	20
95th Queue (ft)	49
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### **Network Summary**

Network wide Queuing Penalty, Interval #1: 1
Network wide Queuing Penalty, Interval #2: 1
Network wide Queuing Penalty, All Intervals: 1

Intersection:	1· HWY 101	& 35th Street	Interval #1
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Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	T	TR	L	T	TR	
Maximum Queue (ft)	64	107	74	59	97	143	122	64	196	192	
Average Queue (ft)	42	59	44	30	60	94	74	34	124	116	
95th Queue (ft)	79	104	80	63	102	159	130	64	202	201	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)		1				1		0	15		
Queuing Penalty (veh)		1				1		0	10		

# Intersection: 1: HWY 101 & 35th Street, Interval #2

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	Т	TR	
Maximum Queue (ft)	110	115	79	74	129	177	189	113	191	199	
Average Queue (ft)	46	54	32	30	48	88	73	37	98	85	
95th Queue (ft)	89	100	67	65	93	152	144	78	162	150	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	1			0	1		0	10		
Queuing Penalty (veh)	1	1			0	1		0	6		

#### Intersection: 1: HWY 101 & 35th Street, All Intervals

Movement	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	
Directions Served	L	TR	L	TR	L	Т	TR	L	T	TR	
Maximum Queue (ft)	110	136	89	79	130	184	189	113	225	218	
Average Queue (ft)	45	55	35	30	51	90	73	36	104	92	
95th Queue (ft)	87	101	71	65	96	154	141	75	174	166	
Link Distance (ft)		456		211		894	894		661	661	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)	110		141		146			91			
Storage Blk Time (%)	0	1			0	1		0	11		
Queuing Penalty (veh)	0	1			0	1		0	7		

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# Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #1

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	2	15	57	40
Average Queue (ft)	0	4	37	27
95th Queue (ft)	0	20	61	52
Link Distance (ft)	211	201	427	216
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Pay Diet (ft)				

Storage Bay Dist (ft)

Storage Blk Time (%)

Queuing Penalty (veh)

#### Intersection: 2: REDWOOD ST/Site & 35th Street, Interval #2

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	4	25	59	41
Average Queue (ft)	0	3	36	20
95th Queue (ft)	2	16	61	48
Link Distance (ft)	211	201	427	216
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

#### Intersection: 2: REDWOOD ST/Site & 35th Street, All Intervals

Movement	EB	WB	NB	SB
Directions Served	LTR	LTR	LTR	LTR
Maximum Queue (ft)	5	25	76	46
Average Queue (ft)	0	3	36	21
95th Queue (ft)	2	17	61	49
Link Distance (ft)	211	201	427	216
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

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# Queuing and Blocking Report 2021 PM Build Conditions

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#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #1

Movement	WB
Directions Served	R
Maximum Queue (ft)	35
Average Queue (ft)	21
95th Queue (ft)	49
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, Interval #2

Movement	WB
Directions Served	R
Maximum Queue (ft)	50
Average Queue (ft)	17
95th Queue (ft)	48
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Intersection: 3: HWY 101 & RIRO SITE ACCESS, All Intervals

Movement	WB
Directions Served	R
Maximum Queue (ft)	50
Average Queue (ft)	18
95th Queue (ft)	48
Link Distance (ft)	201
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### **Network Summary**

Network wide Queuing Penalty, Interval #1: 12
Network wide Queuing Penalty, Interval #2: 8
Network wide Queuing Penalty, All Intervals: 9